

Sociedad Colombiana de Geotecnia
XII CONGRESO COLOMBIANO DE GEOTECNIA

Bogotá 3 - 4 Septiembre 2008



**Taludes de corte estabilizados
en Venezuela mediante pantallas
de concreto proyectado ancladas**

Gianfranco Perri











2 m



2 m



Technical Data

Bar Diameter mm	Steel Grade	Ultimate Strength MPa (ksi)	Yield Strength MPa (ksi)	Pre-stressing Stress	
				Min (%)	EN (C ₁)
16	S500/600	195	159		
20	S600/800	345	283		
26.5	S600/800	575	523		
32	S600/800	844	764		
36	S600/800	1059	967		
40	S600/800	1320	1194		
47	S600/800	1822	1645		
Clear-Steel					
16	S500/600	121	100		
20	S500/600	188	157		
25	S500/600	295	245		
28	S500/600	370	308		
32	S500/600	482	402		
40	S500/600	755	630		
50	S500/600	1176	980		
63.5	S550/700	2217	1758		

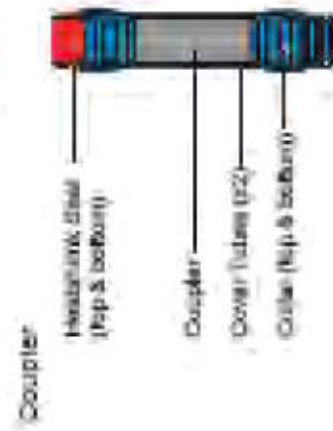
Corrosion Protection for Permanent Bar Anchors

Permanent Anchors (lifespans in excess of 2 years) require sufficient corrosion protection to ensure durability throughout the working life of each anchor.

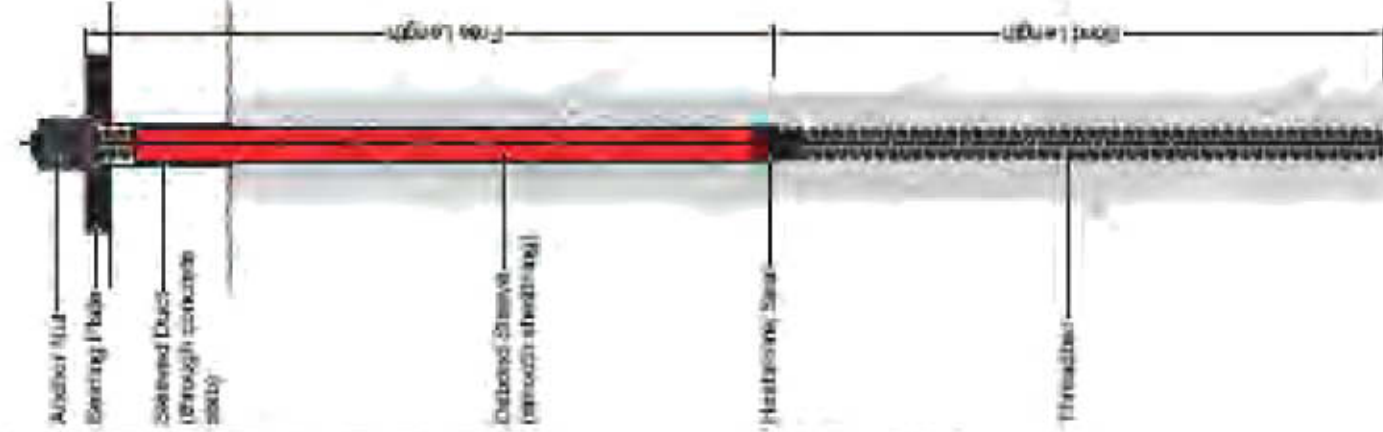
Double Corrosion Protection, featuring factory pregrouted encapsulation of the bar within a corrugated plastic sheath, ensures comprehensive protection to all parts of the anchor. Protection at the anchor head is provided by a fully enclosed assembly.

Borehole Grout is not recognised by current anchor standards (BS6081 or EN1537) as a corrosion protection barrier.

Double Corrosion Protection Detail:



Temporary Bar Anchor (Lifespan: up to 2 years)



Lantern Spacer - plain bar (soil or rock)



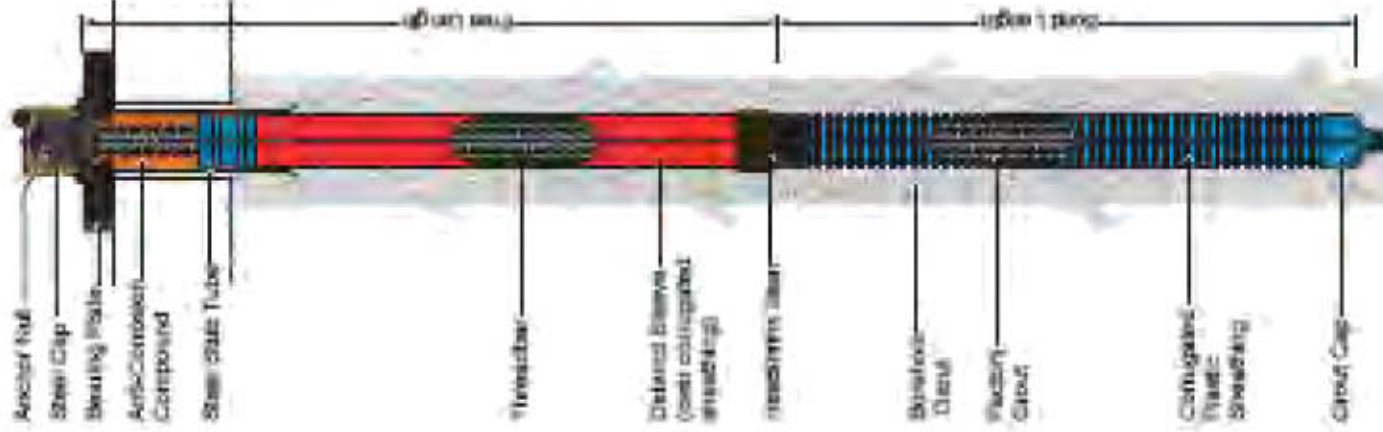
Lantern Spacer (cased boreholes, soil)



Wrap-around Spacer (stable boreholes, rock)



Permanent Bar Anchor with Double Corrosion Protection (Lifespan: up to 120 years)









45 m





ESTADO DE GUATEMALA
TUESO A VIENNA
CALLE DE LA GUATEMALA
CALLE DE LA GUATEMALA
CALLE DE LA GUATEMALA





WIS COLNAT
DE TORONTO



URB. COLINAS
DE TURUMO





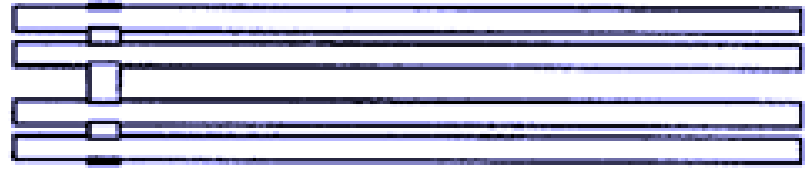

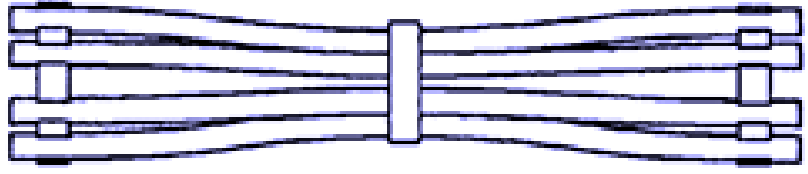



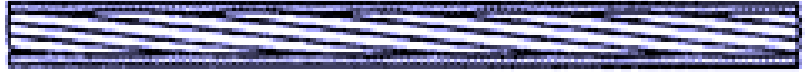

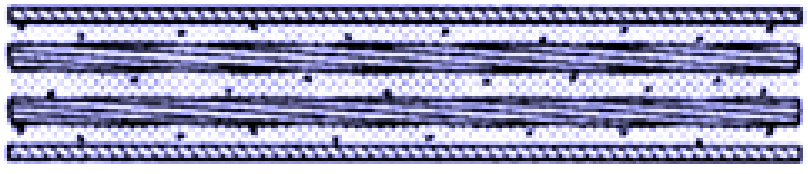
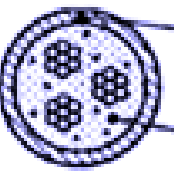


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TORONES O GUAYAS PARA ANCLAJES

TYPE	LONGITUDINAL SECTION	CROSS SECTION
Multi-wire tendon (Clifford, 1974)		
Birdcaged multi-wire tendon (Jirovec, 1978)		 Antinode Node
Single strand (Hunt & Askew, 1977)		 Normal Indented Drawn
Coated single strand (Hunt & Askew, 1977)		 Sheathed Coated Encapsulated
High capacity shear dowel (Mathews et al, 1986)		 Steel tube Concrete

Technical Data

Strand Anchors are constructed from compacted strand, grade 100K / 1520 N/mm²

Number of strands	Ultimate Strength (kN (k))	0.1% Proof (kN (k) @ length)
1	900	295
2	800	510
3	900	785
4	1300	1020
5	1500	1275
6	1800	1530
7	2100	1785
8	2400	2040
9	2700	2295
10	3000	2550
11	3300	2805
12	3600	3060

Higher capacity anchors are available through the addition of further strands.

Corrosion Protection for Permanent Strand Anchors

The same principles of corrosion protection for Permanent Bar Anchors apply for Permanent Strand Anchors, to ensure durability throughout the lifespan.

For Permanent Strand Anchors, Double

Corrosion Protection is achieved by two

independent layers of plastic sheathing

as follows:

Bond Length

Two concentric corrugated plastic sheaths (typically factory pregrouted).

Free Length

Individual plastic sheaths (smooth) for each strand, all enclosed within a common external corrugated plastic sheath.

Protection at the anchor head is provided

by a fully enclosed assembly.

Anchors are factory assembled and coated (except for pregrouted bond length), prior to delivery.

Grout is not recognised by current anchor standards (BS5081 or EN1537) as a corrosion protection barrier.

Double Corrosion Protection Detail:

Bond Length

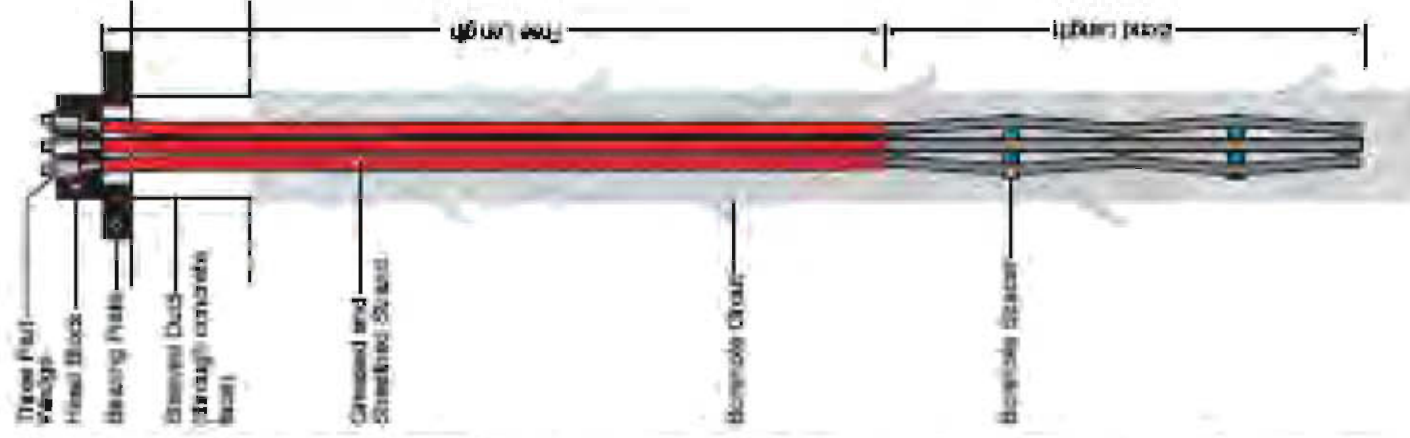


Free Length



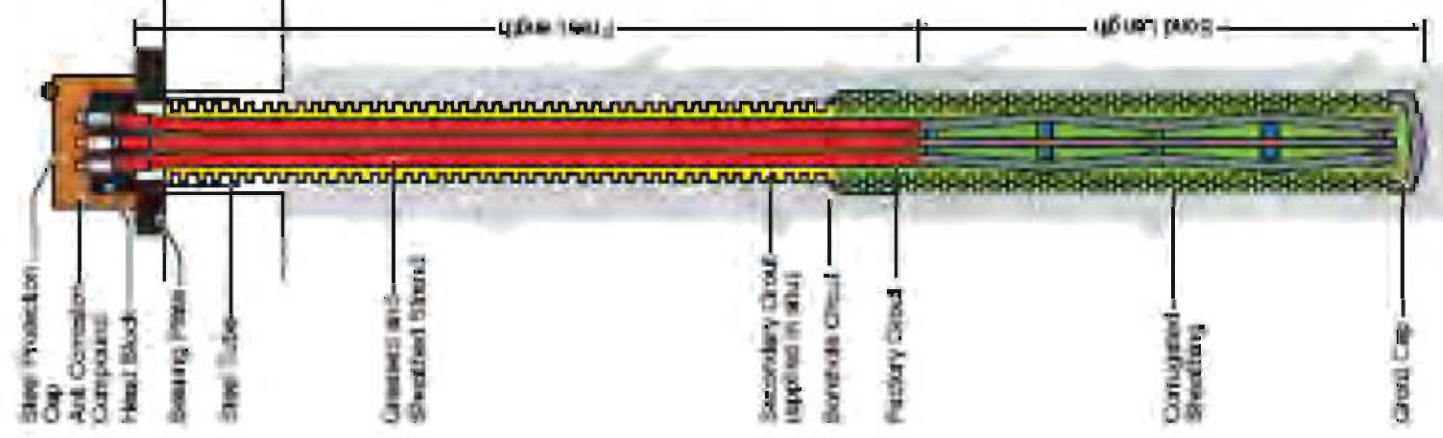
Temporary Strand Anchor

(Lifetime: up to 2 years)



Permanent Strand Anchor with Double Corrosion Protection

(Lifetime: up to 120 years)



Permanent Strand Anchors, Head Assemblies

Strandless (non-retrievable)



Thimble Headblock (non-retrievable)



Long End Cap (non-retrievable)

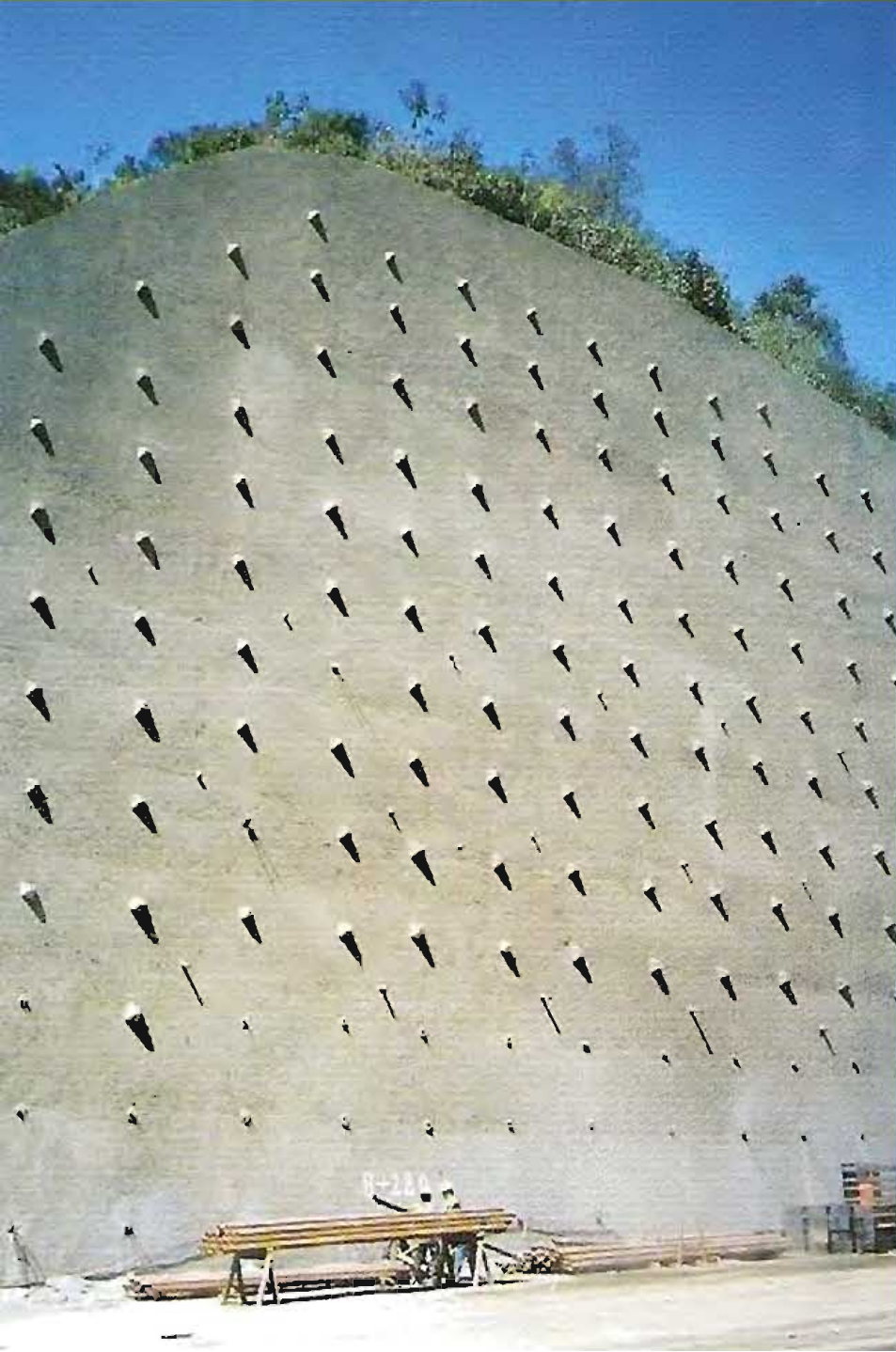




24 m



26 m





...PUESTOS

PRIMERA FERIA DE
MATERIA CONCRETA
CALLE 40 N. 100
BOGOTÁ

BOGOTÁ

BOGOTÁ

WHITE CAR



Pantallas de Concreto Proyectado en Seco Ancladas y Claveteadas

Concreto Proyectado Armado:

10 a 40 cm de espesor

250 a 350 Kg/cm² de Resistencia a 28 días

350 a 450 Kg/m³ de Cemento Portland Tipo I

Pernos de Alta Resistencia:

0.5" a 1" de Diámetro

6 a 18 m de Longitud

20 a 50 t de Capacidad

Anclajes de Torones:

7 y 9 Hilos de 2 a 4 mm de Diámetro

1/2" y 5/8" de Diámetro por Torón

6 a 60 m de Longitud

Clavos Metálicos:

1" y 1+3/8" de Diámetro

3 a 12 m de Longitud

20 a 30 t de Capacidad

10 y 15 t de Capacidad por Torón

2 a 6 Torones por Anclaje

3 a 12 m Longitud Bulbo

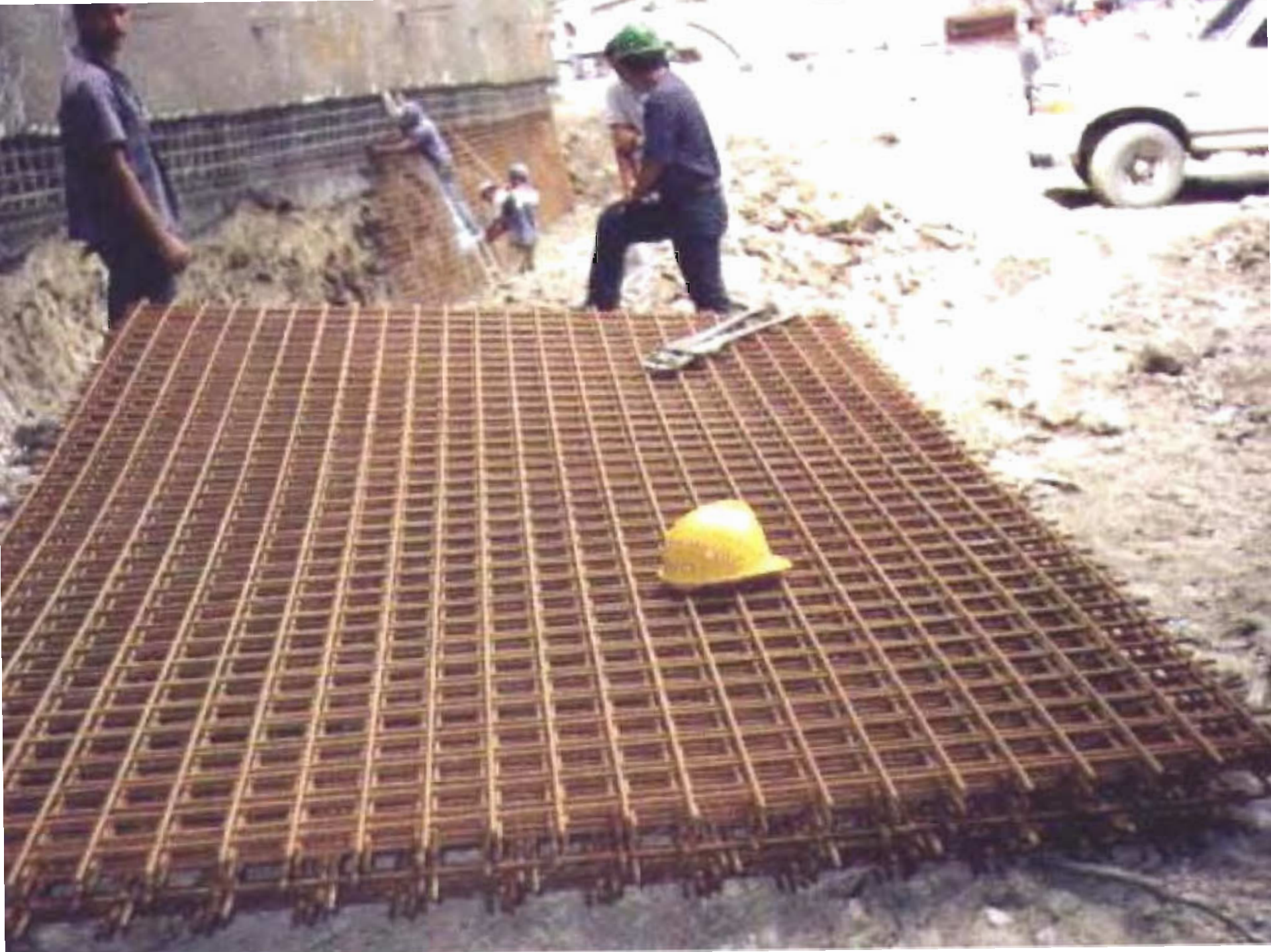
20 a 90 t de Capacidad por Anclaje

Mortero de Inyección:

0.4 a 0.8 Agua/Cemento en peso

2 a 6 Kg/cm² de Presión













Máquina Projectadora de Rotor





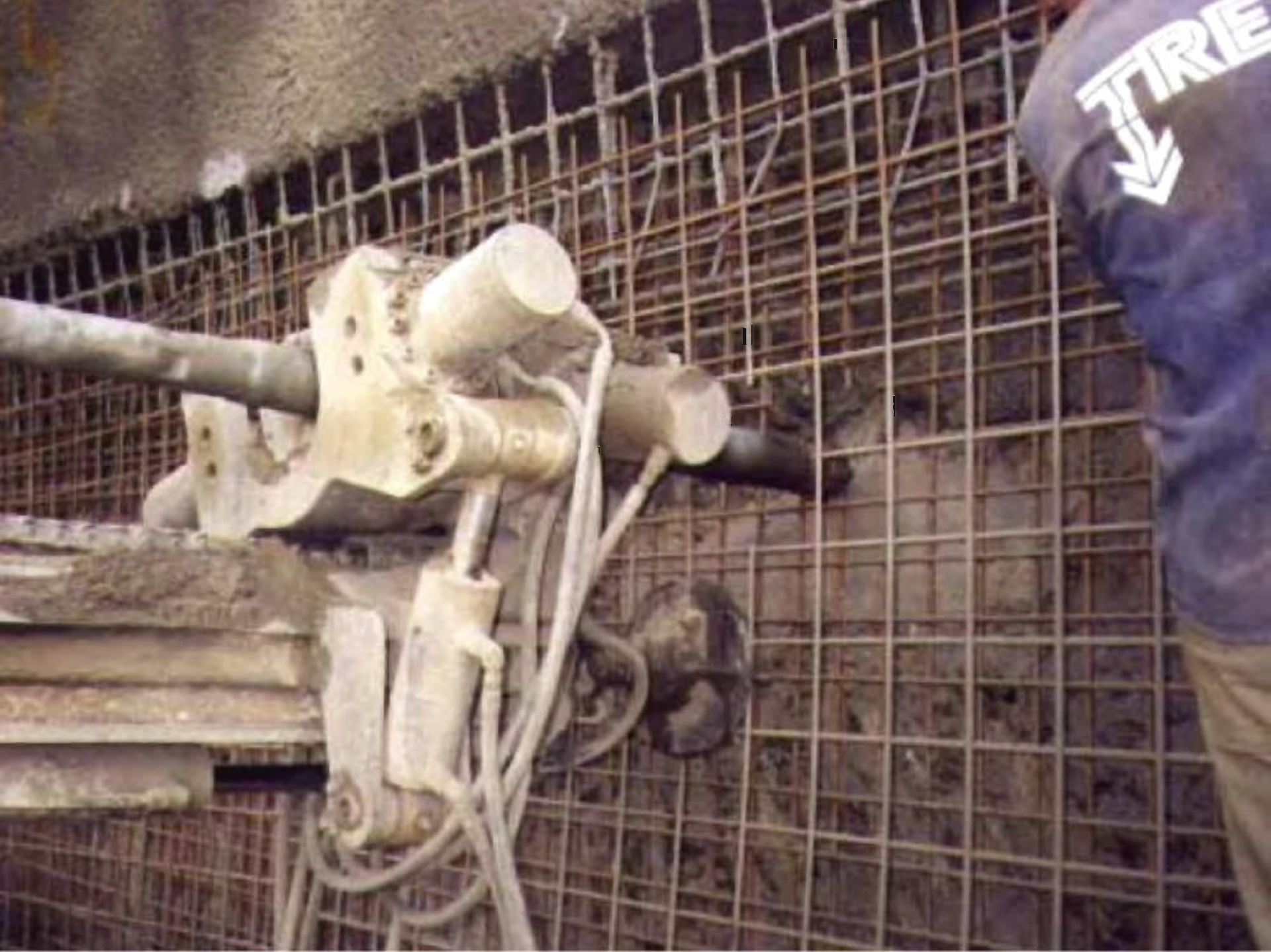






















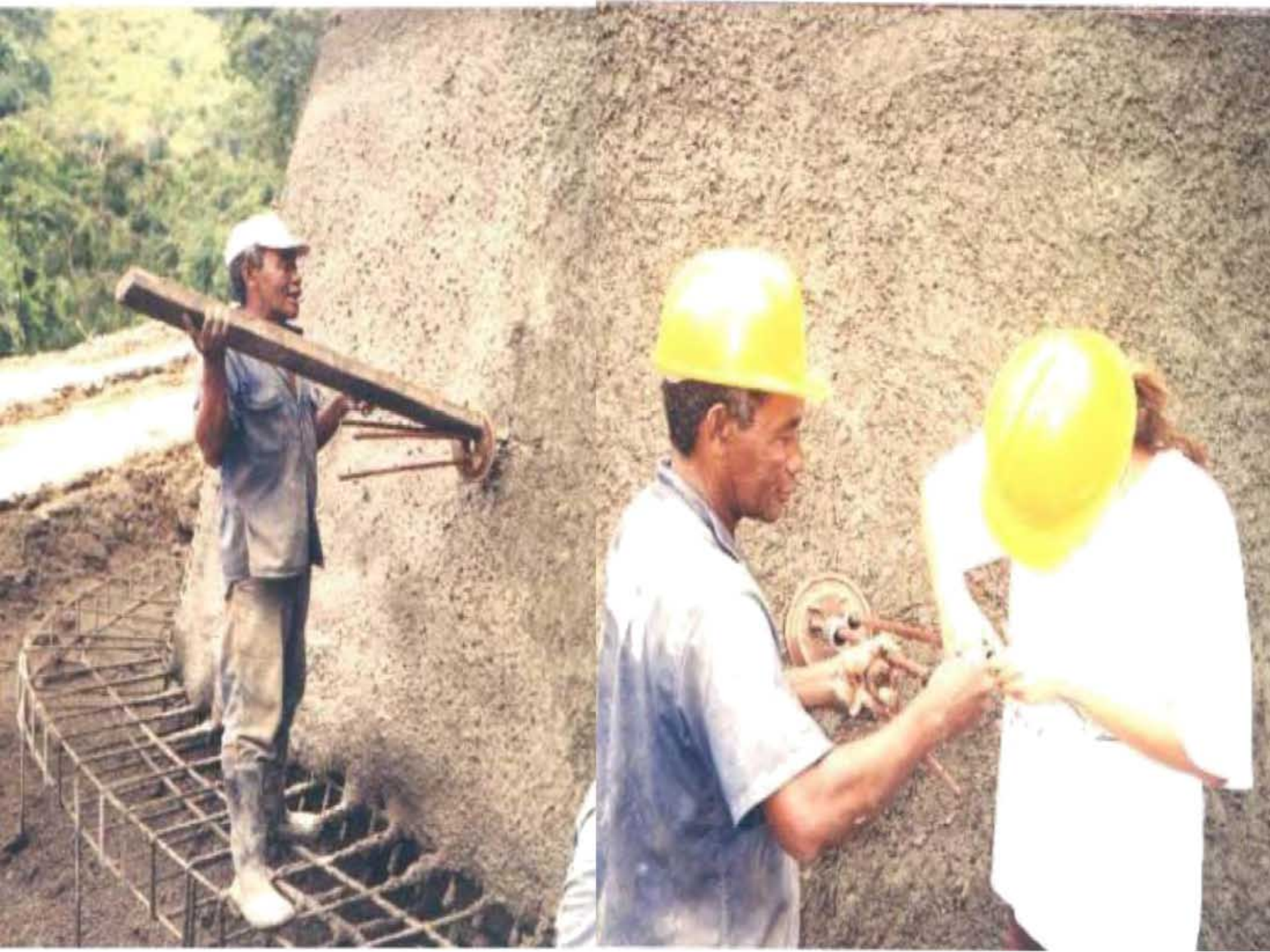








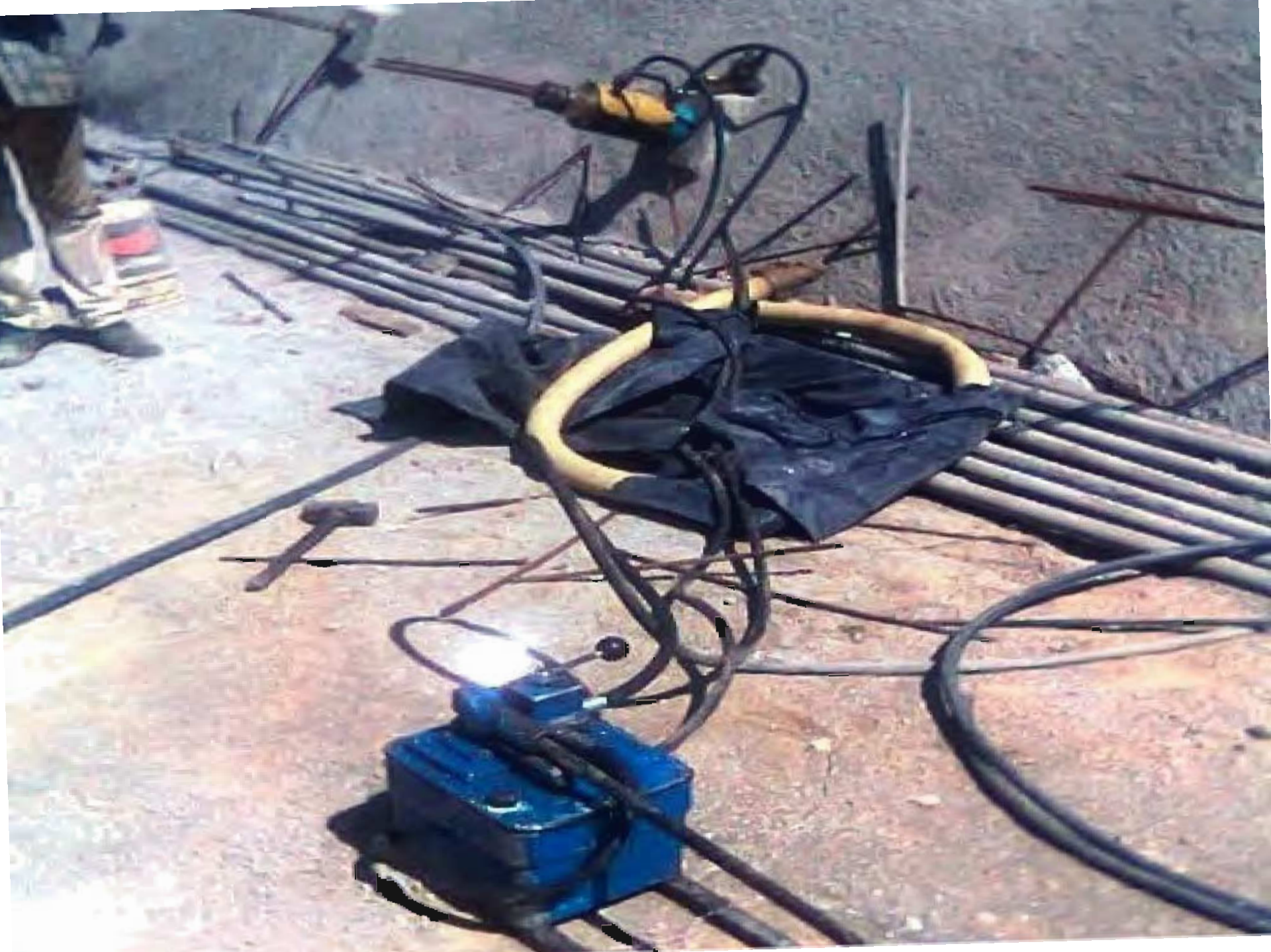
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Leche
Lactosa
de Vaca

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Tiempo (min)	Presión (bares)	Tensión por Toron (ton)	Tensión por Anclaje (ton)	Elongación (mm)		
				Gato 1	Gato 2	Gato 3
1	60	3	9	50	45	55
1	130	6	18	65	65	70
1	190	9	27	80	80	90
1	260	12	36	100	95	105
1	320	15	45	115	110	120
1	390	18	54	135	130	140
2	390	18	54	135	130	140
4	390	18	54	135	130	140
8	390	18	54	135	130	140
12	390	18	54	135	130	140
16	390	18	54	135	130	140
1	60	3	9	100	110	125
1	320	15	45	120	110	130

Tensado del Anclaje G-2 de la pantalla Palmarito: tres torones de 15 ton cada uno





Rigid micro-slotted drainage pipe

Physical characteristics

Material		HDPE (PP on request)
Outer diameter	mm	62
Inner diameter	mm	50
Outer surface		corrugated
Slot width	mm	0.5 – 0.7
Slot layout		4 arranged at 90°
Bar length	m	1.5 – 3.00 – 6.00

Filtering geo-fabric characteristics (TNT)

Material		Polypropylene	
Mass in air	g/m ²	120	150
Traction resistance	kN/m	7.5	9
Perforation resistance	N	1300	1600
Filtering diameter	micron	130	130
Permeability	m/s	1.18x10 ⁻³	1.15x10 ⁻³



PRODUCT PATENTED BY ELAS GEOTECNICA s.r.l.

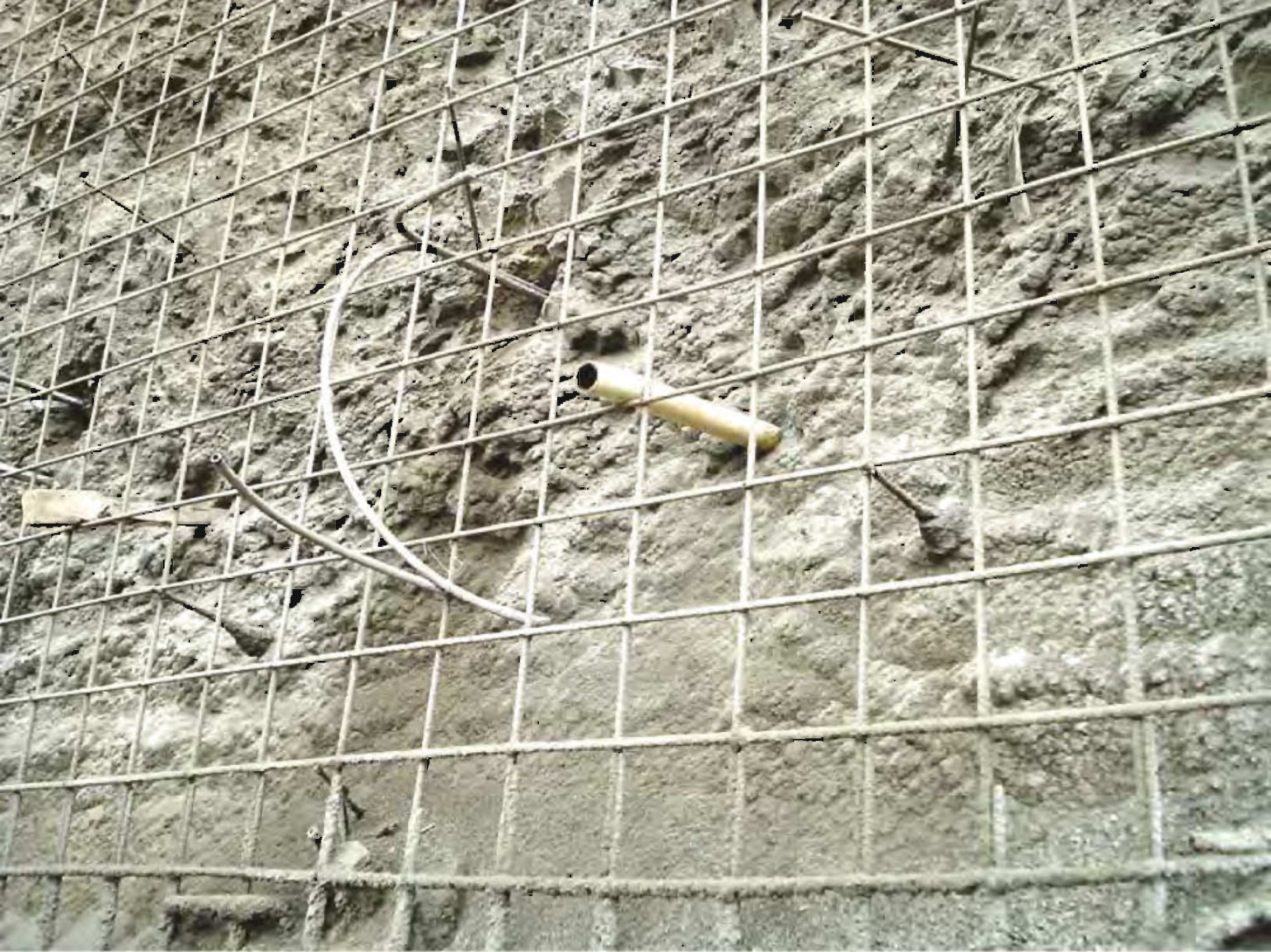




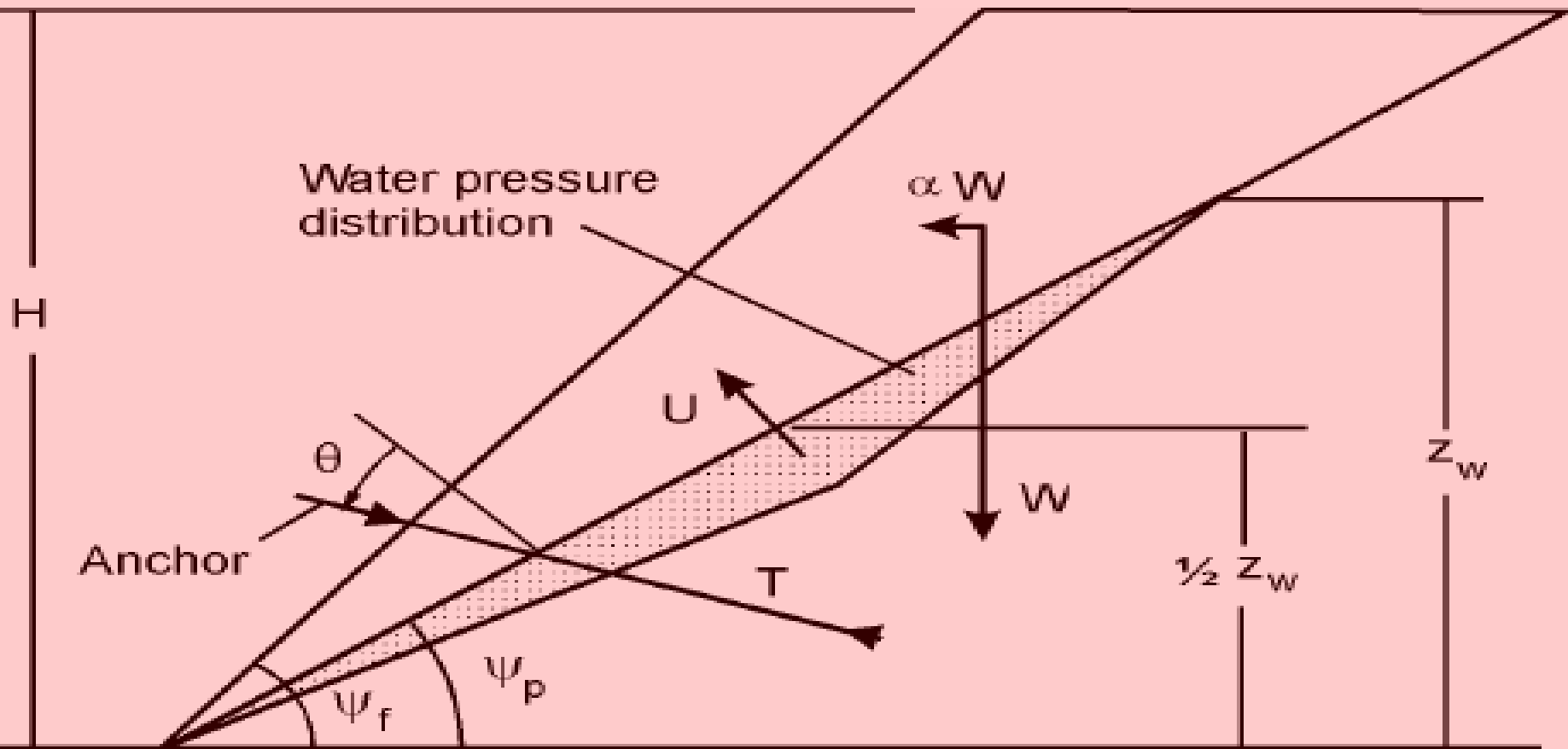












Fs vs. Pf

$$F = \frac{cA + (W (\cos \psi_p - \alpha \sin \psi_p) - U + T \cos \theta) \tan \phi}{W (\sin \psi_p + \alpha \cos \psi_p) - T \sin \theta}$$

=C/D

Ángulo de fricción (°)

Summary:

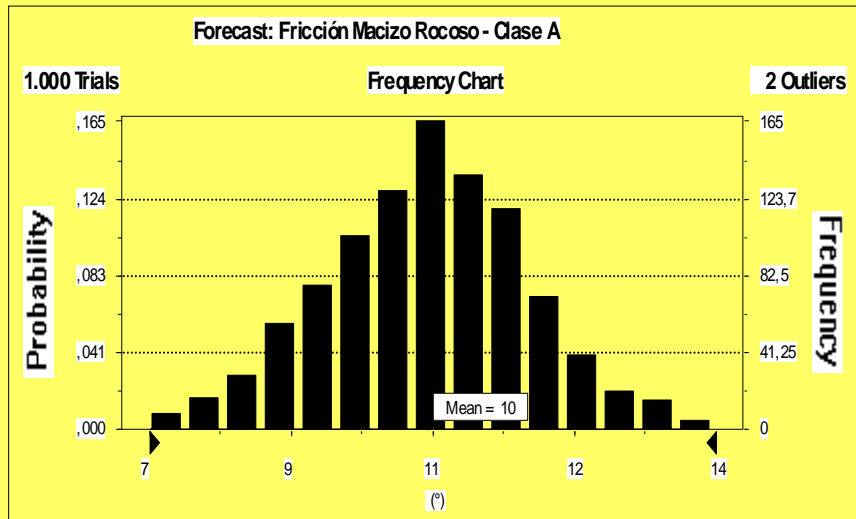
Display Range is from 7 to 14 (°)

Entire Range is from 7 to 14 (°)

After 1.000 Trials, the Std. Error of the Mean is 0

Statistics:

	Value
Trials	1000
Mean	10
Median	10
Mode	---
Standard Deviation	1
Variance	2
Skewness	-0,10
Kurtosis	2,93
Coeff. of Variability	0,12
Range Minimum	7
Range Maximum	14
Range Width	7
Mean Std. Error	0,04



Cohesión (MPa)

Summary:

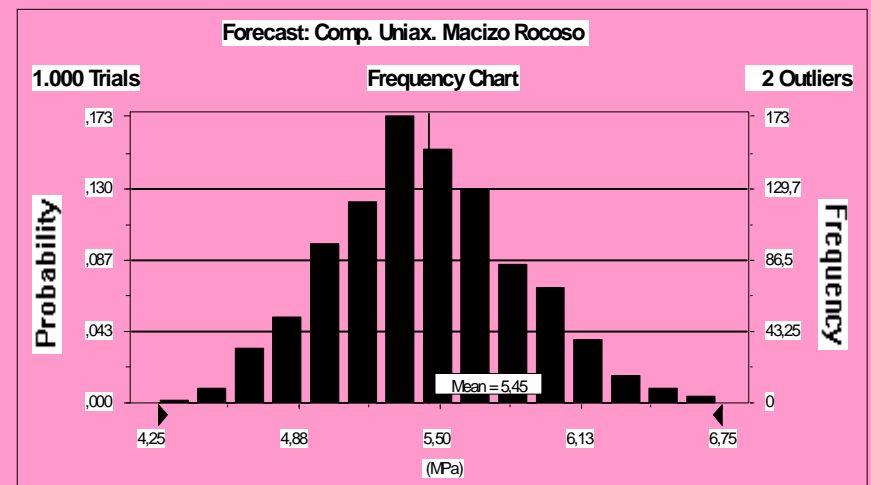
Display Range is from 4,25 to 6,75 (MPa)

Entire Range is from 4,28 to 6,86 (MPa)

After 1.000 Trials, the Std. Error of the Mean is 0,01

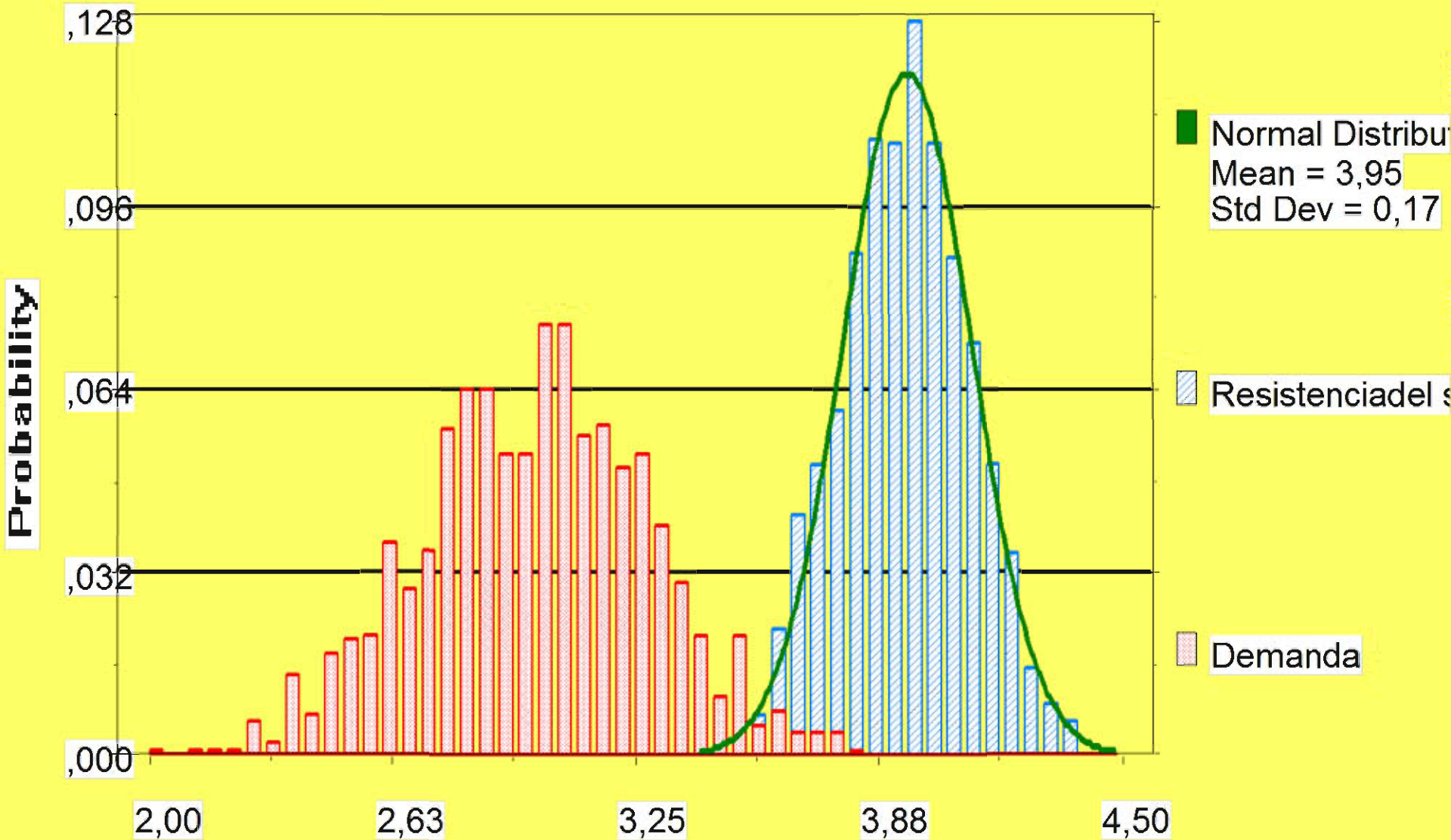
Statistics:

	Value
Trials	1000
Mean	5,45
Median	5,43
Mode	---
Standard Deviation	0,42
Variance	0,18
Skewness	0,23
Kurtosis	2,94
Coeff. of Variability	0,08
Range Minimum	4,28
Range Maximum	6,86
Range Width	2,58
Mean Std. Error	0,01

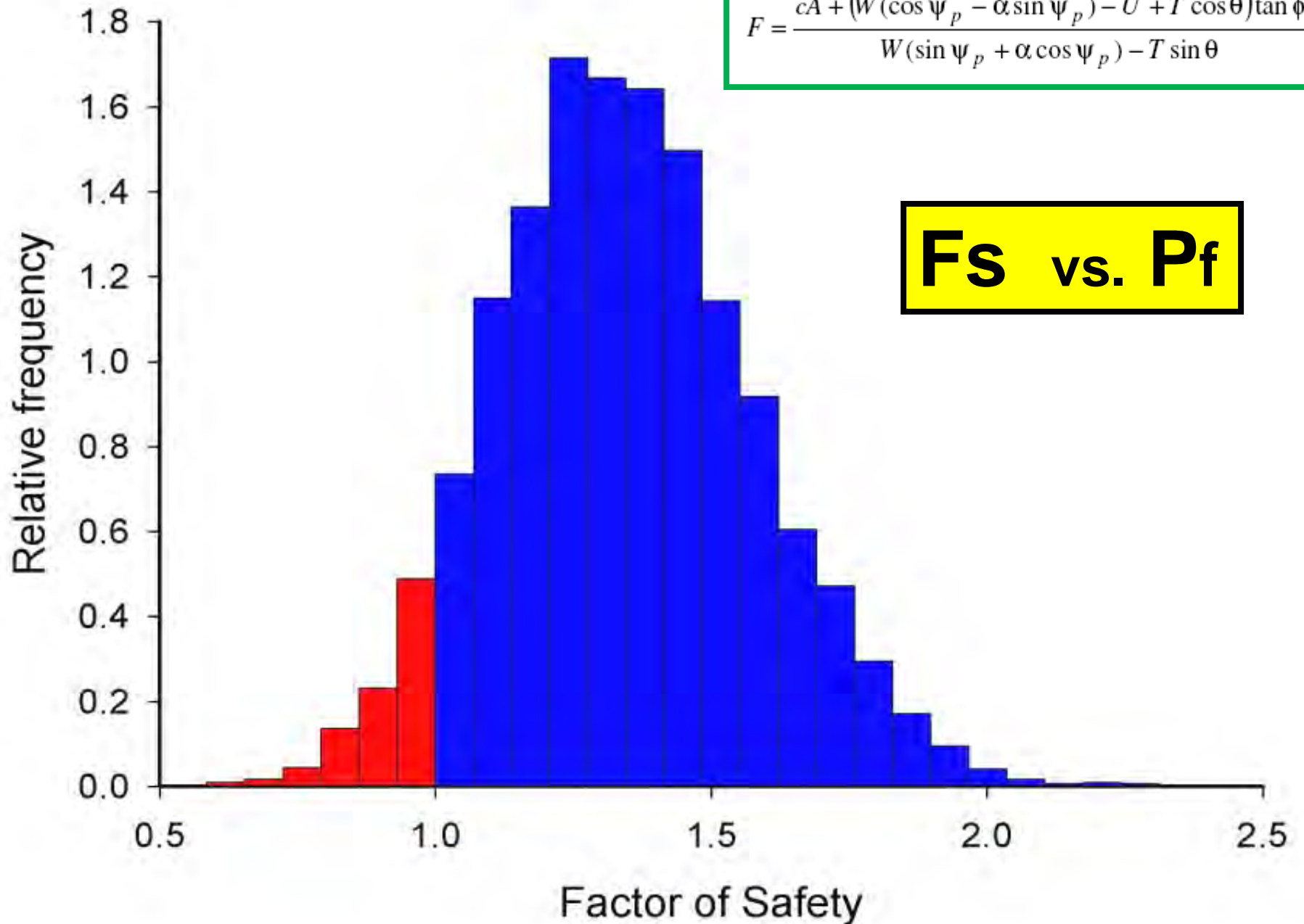


Overlay Chart

Capacidad vs. Demanda



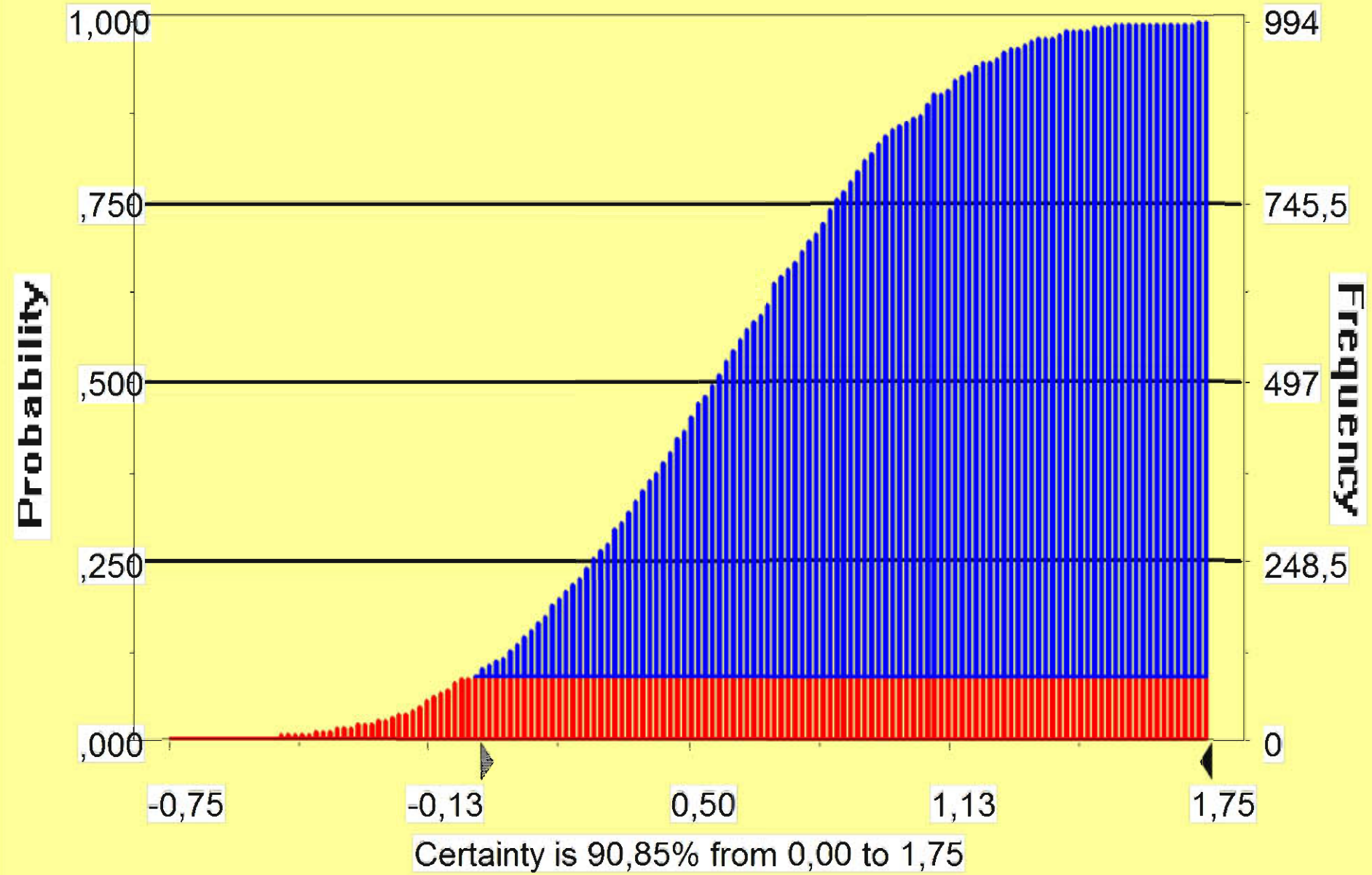
$$F = \frac{cA + (W(\cos \psi_p - \alpha \sin \psi_p) - U + T \cos \theta) \tan \phi}{W(\sin \psi_p + \alpha \cos \psi_p) - T \sin \theta}$$



Forecast: C-D

994 Trials

Cumulative Chart



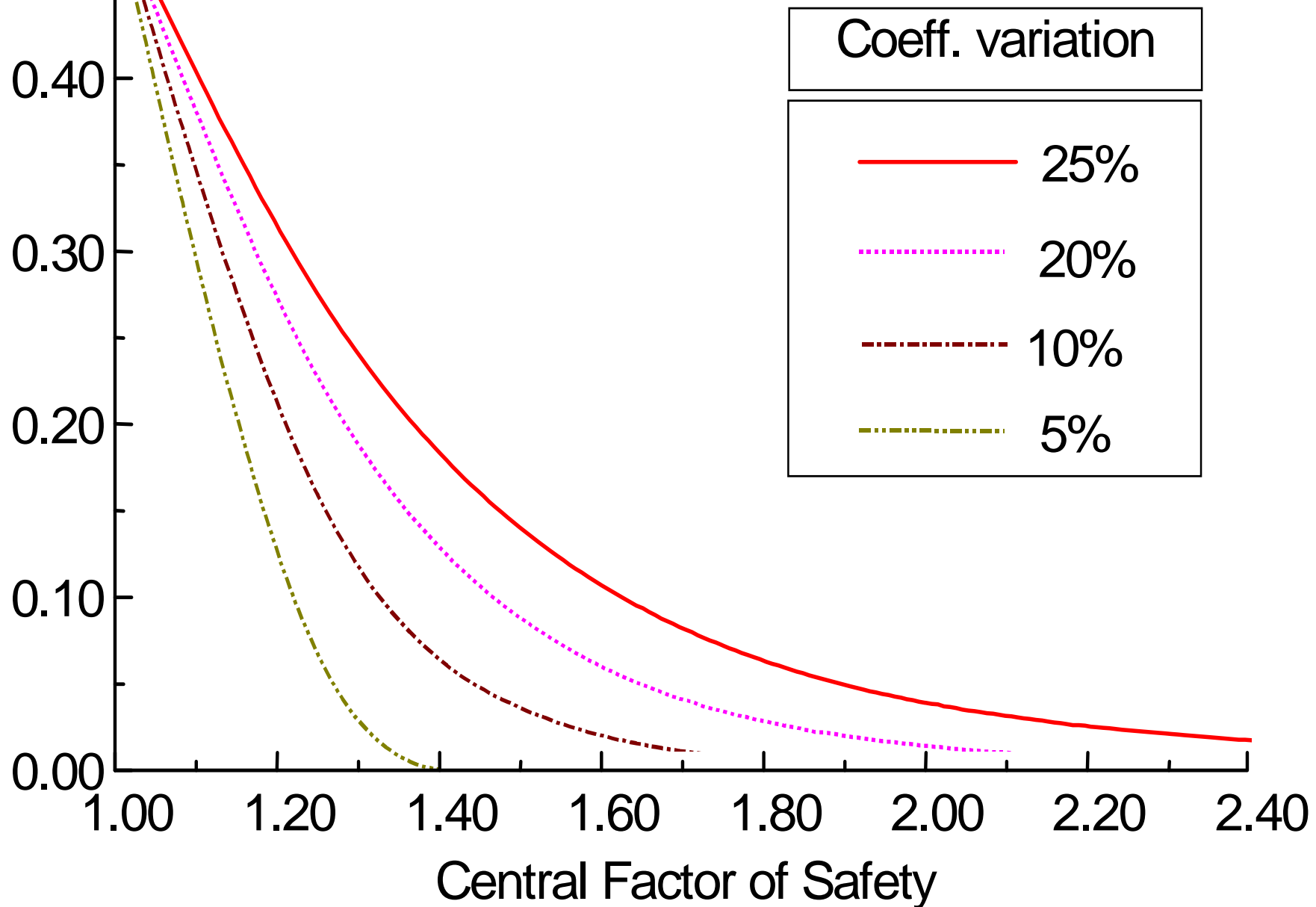
EJEMPLO N°1

\emptyset	δ	c (t/m ²)	δ	F.S. Determinístico	F.S. Estadístico	F.S. < 1 p (%)	F.S.<1,4 p(%)
25	0	10	0	1.569	1.569	0	0
25	2.5	10	2	1.569	1.411	1	48
25	2.5	15	5	2.147	1.743	2	--
25	2.5	20	8	2.703	2.120	2	--
25	2.5	20	10	2.703	1.973	6	--
25	2.5	10	3	1.569	1.325	9	65
25	2.5	15	8	2.147	1.544	13	--
25	2.5	10	4	1.569	1.228	21	75
25	2.5	15	10	2.147	1.398	23	--
25	2.5	10	5	1.569	1.136	34	77

EJEMPLO N°3

\emptyset	δ	c (t/m ²)	δ	F.S. Determinístico	F.S. Estadístico	F.S. < 1 p (%)	F.S.<1,4 p (%)
25	2.5	7	0	1.584	1.587	0	0
25	2.5	7	2.5	1.584	1.579	2	34
25	2.5	7	3	1.584	1.573	4	35
25	2.5	15	8	2.400	2.445	4	--
25	2.5	15	10	2.400	2.456	7	--

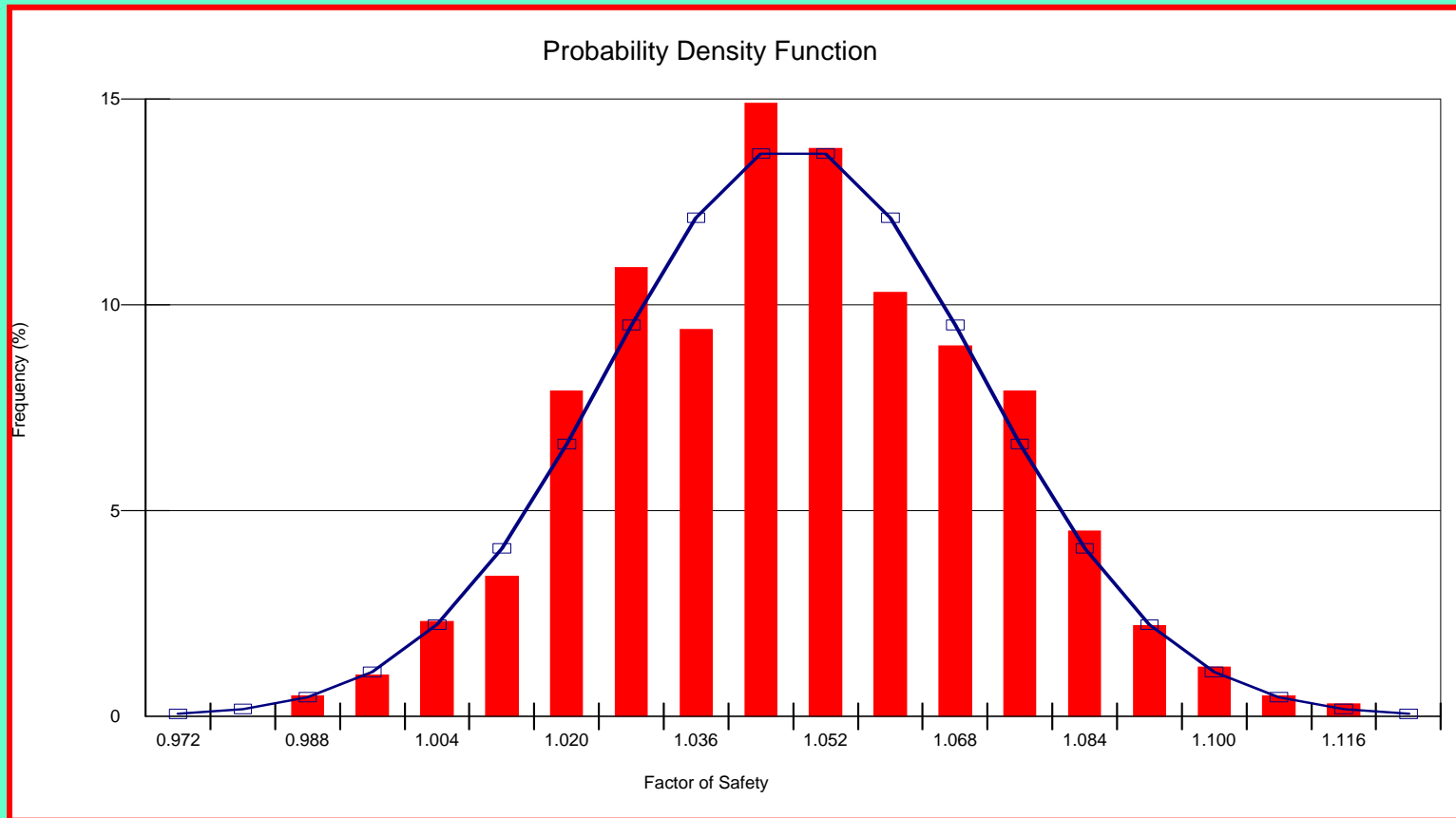
Probability of failure, P_f



Fs Vs. Pf

No hay dudas que constituye un salto cualitativo importante y positivo el pasar del concepto **“Factor de Seguridad”** al concepto **“Probabilidad de Falla”**, aunque debe reconocerse que **“el impacto emocional”** de proponer al público no especialista que existe siempre un riesgo finito (aunque pequeño) de falla asociado al diseño de un talud, es tal que dificulta la propuesta de reemplazar el factor de seguridad estándar con la probabilidad de falla o con el índice de confiabilidad de un talud” (Hoek, 1998).

Portal Palmarito Norte Talud sin Estabilizar H = 15m Ru = 0,25

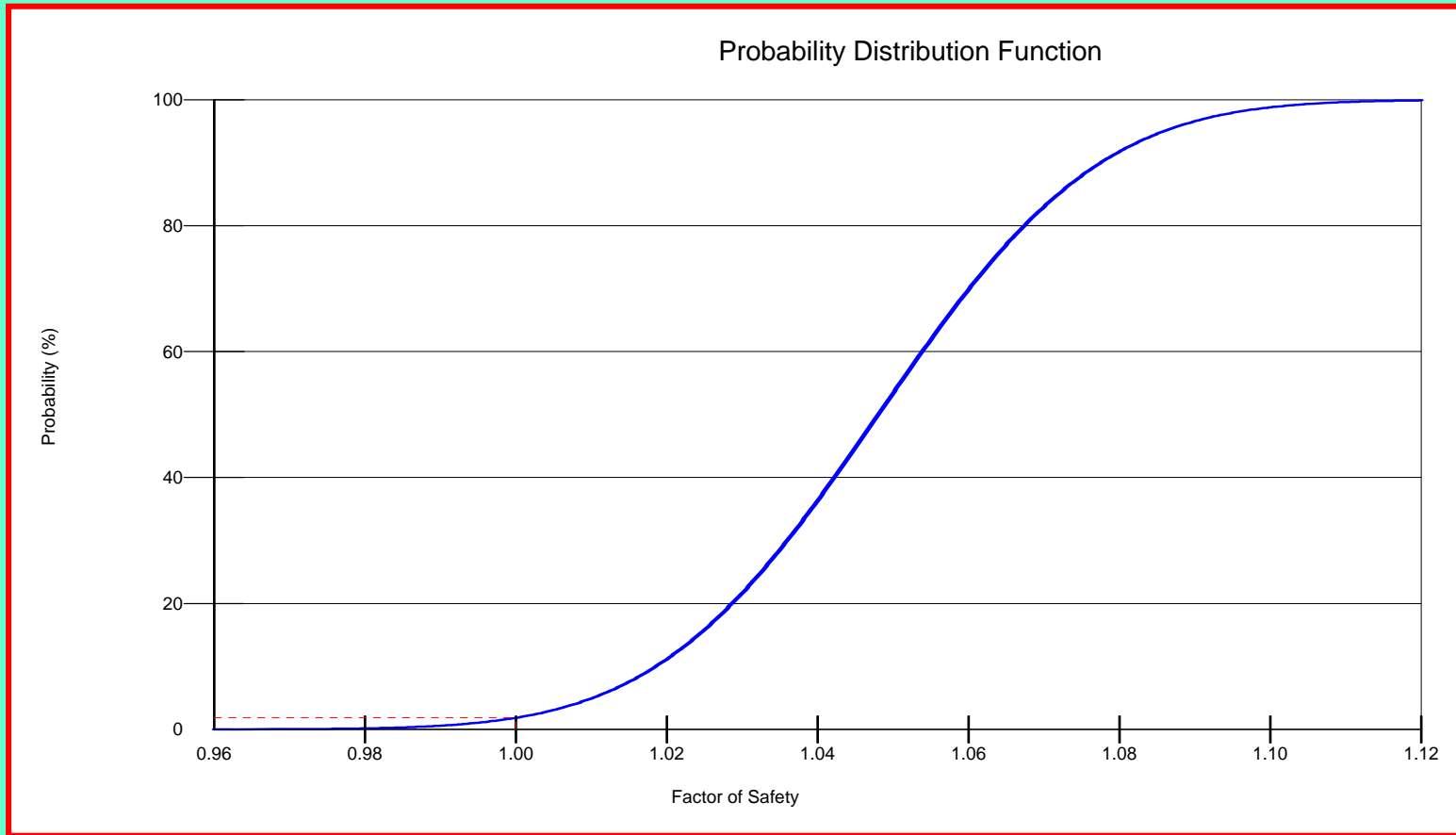


Mean FS=1.048
Standard Dev=0.0230

Reliability Index=2.088
Min FS=0.9840

P(Failure) (%)=1.829
Max FS=1.119

Portal Palmarito Norte Talud sin Estabilizar H = 15m Ru = 0,25

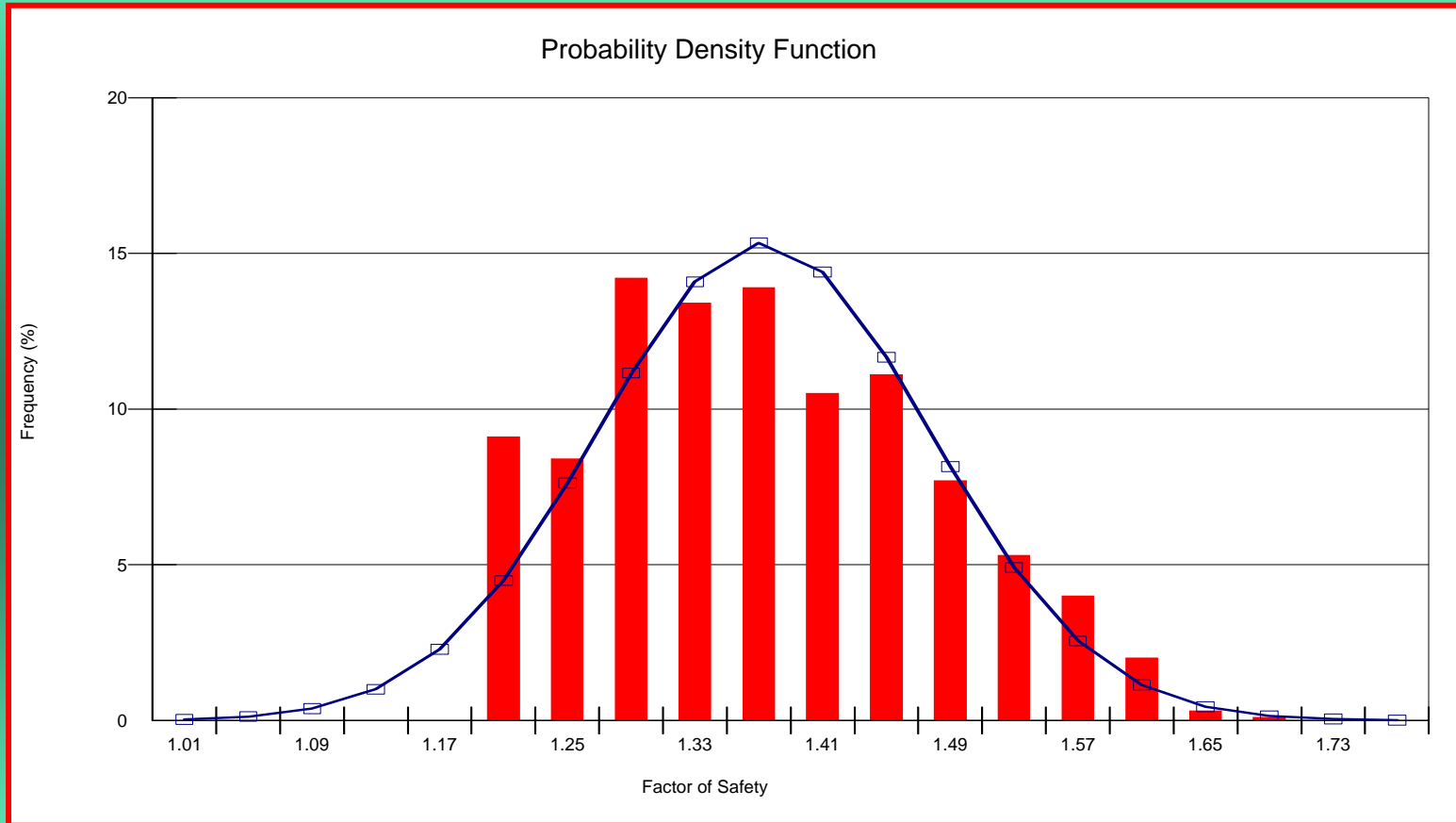


Mean FS=1.048
Standard Dev=0.0230

Reliability Index=2.088
Min FS=0.9840

P(Failure) (%)=1.829
Max FS=1.119

Portal Palmarito Norte Talud Estabilizado con anclajes de 30 t (2x 2 m) H = 15m Ru = 0,25

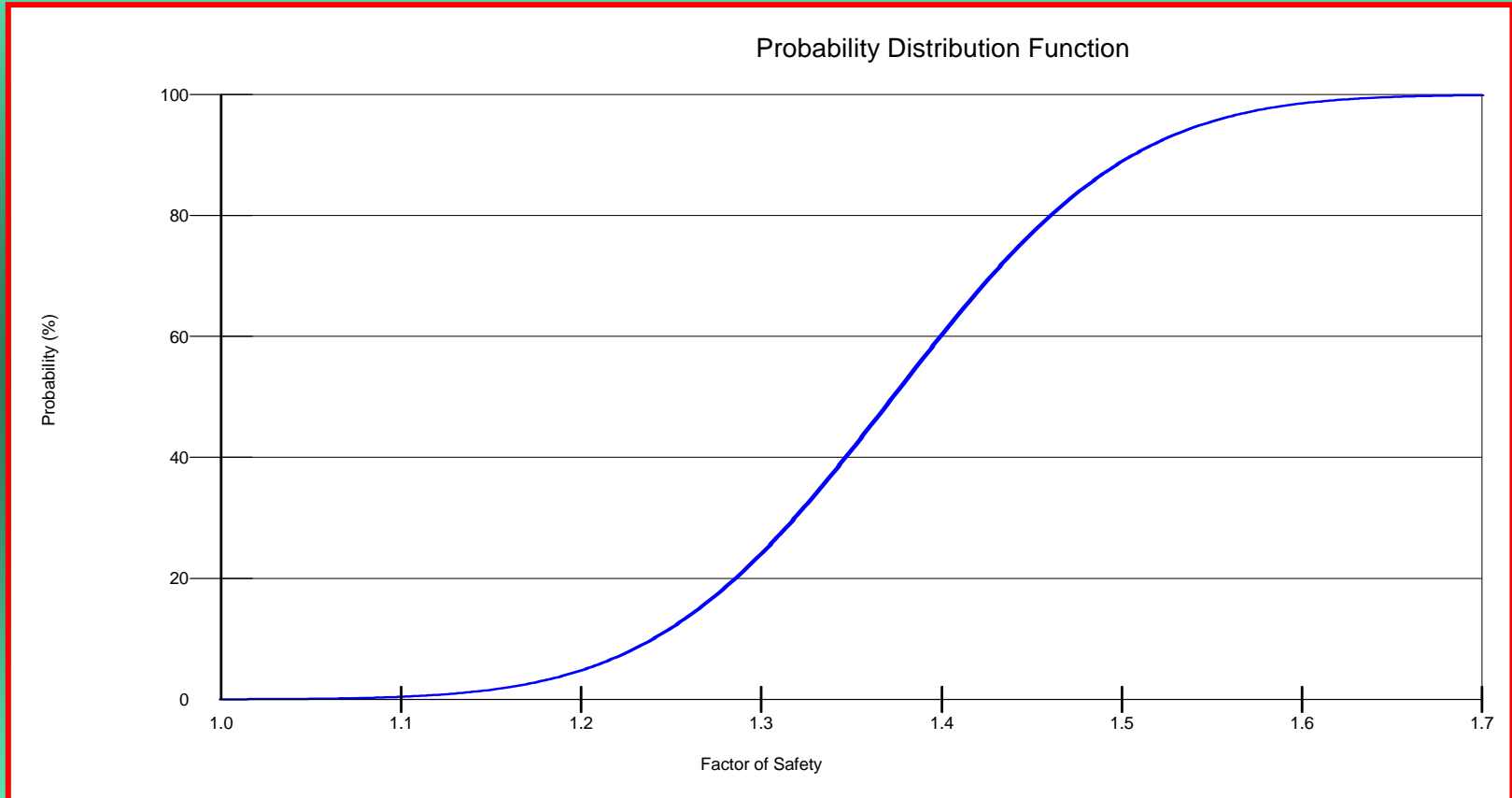


Mean FS=1.373
Standard Dev=0.1040

Reliability Index=3.586
Min FS=1.210

P(Failure) (%)=0.017
Max FS=1.671

Portal Palmarito Norte **Talud Estabilizado con anclajes de 30 t (2x 2 m)** **H = 15m** **Ru = 0,25**

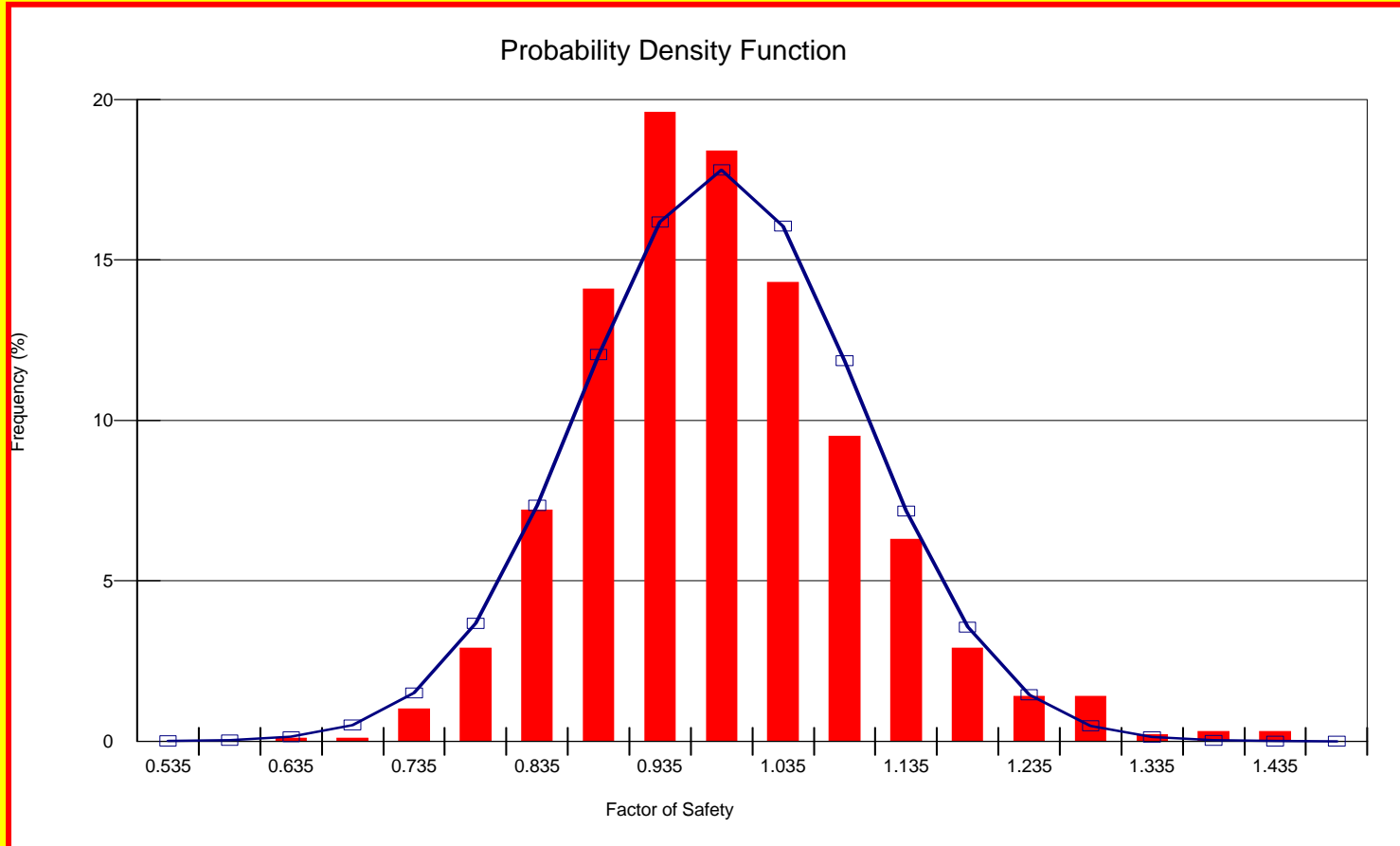


Mean FS=1.373
Standard Dev=0.1040

Reliability Index=3.586
Min FS=1.210

P(Failure) (%)=0.017
Max FS=1.671

Portal Palmarito Norte Talud con Sismo sin Estabilizar H = 15m S_h = 0,3

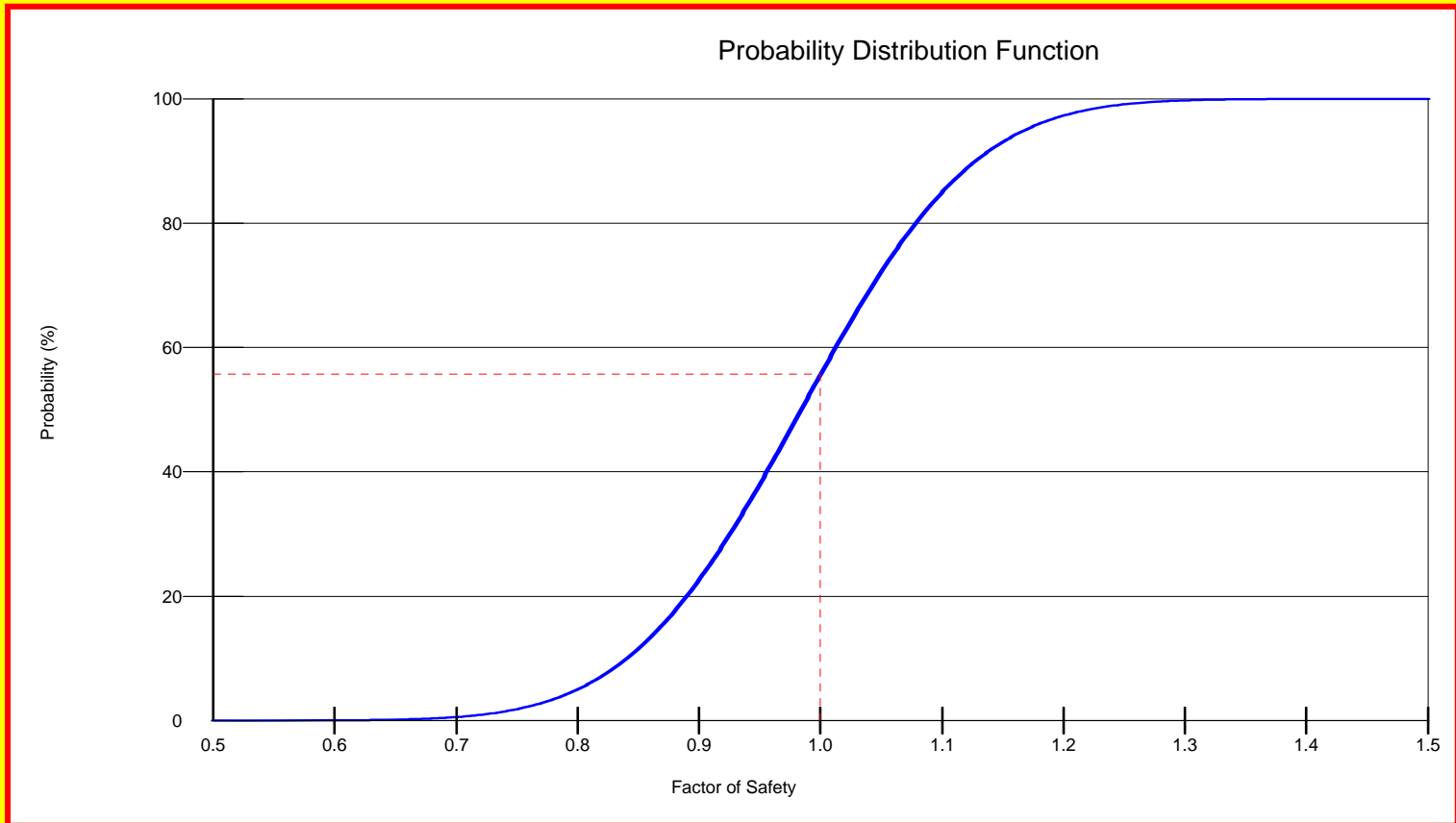


Mean FS=0.984
Standard Dev=0.1120

Reliability Index=0.1440
Min FS=0.653

P(Failure) (%)=45.736
Max FS=1.438

Portal Palmarito Norte Talud con Sismo sin Estabilizar H = 15m S_h = 0,3

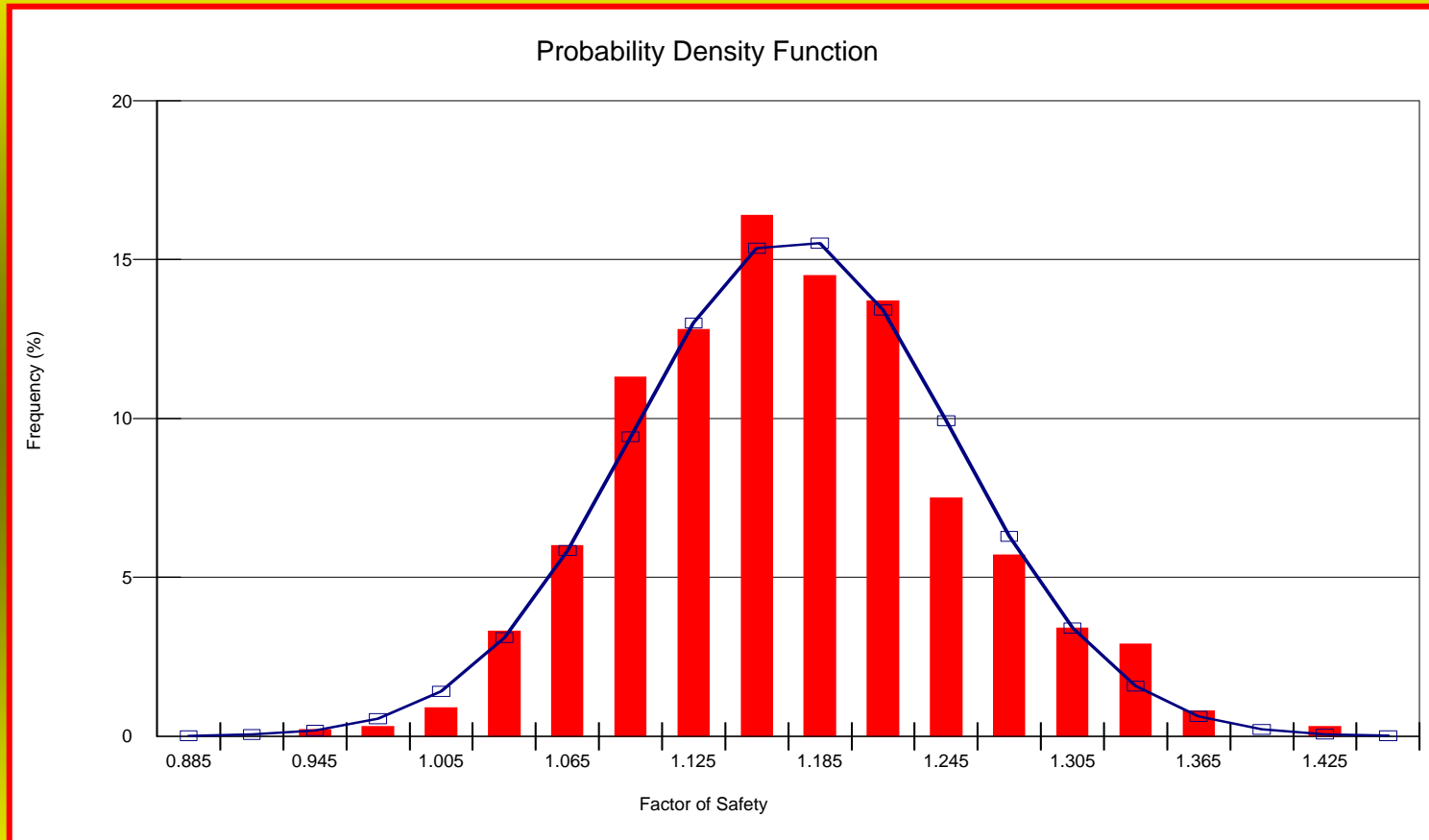


Mean FS=0.984
Standard Dev=0.1120

Reliability Index=0.1440
Min FS=0.653

P(Failure) (%)=45.736
Max FS=1.438

Portal Palmarito Norte Talud con Sismo Estabilizado con anclajes de 30 t (2x 2 m) H = 15m $S_h = 0,3$

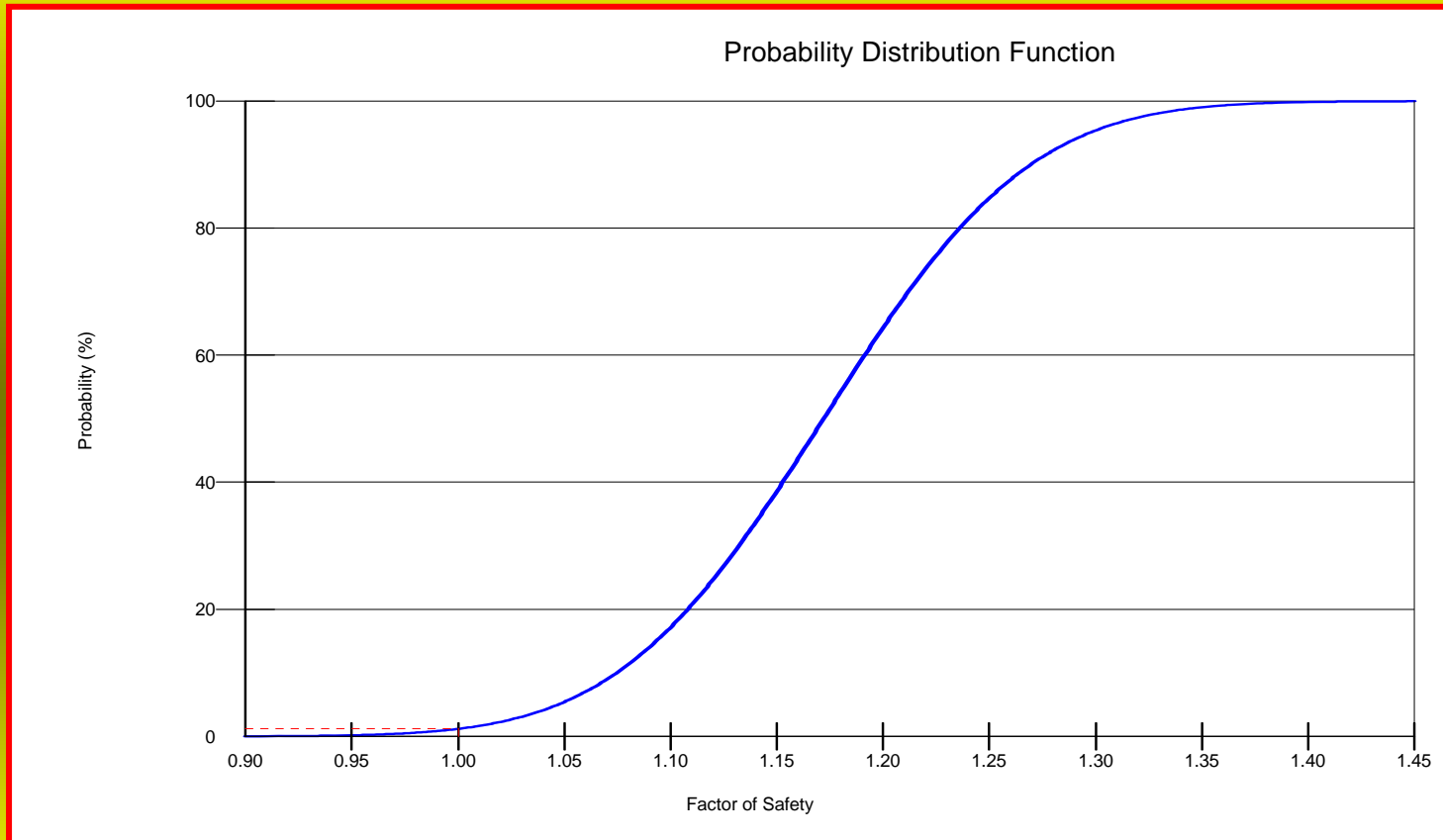


Mean F S=1.172
Standard Dev=0.0760

Reliability Index=2.245
Min F S=0.941

P (Failure) (%)=1.232
Max FS=1.438

Portal Palmarito Norte Talud con Sismo Estabilizado con anclajes de 30 t (2x 2 m) H = 15m $S_h = 0,3$

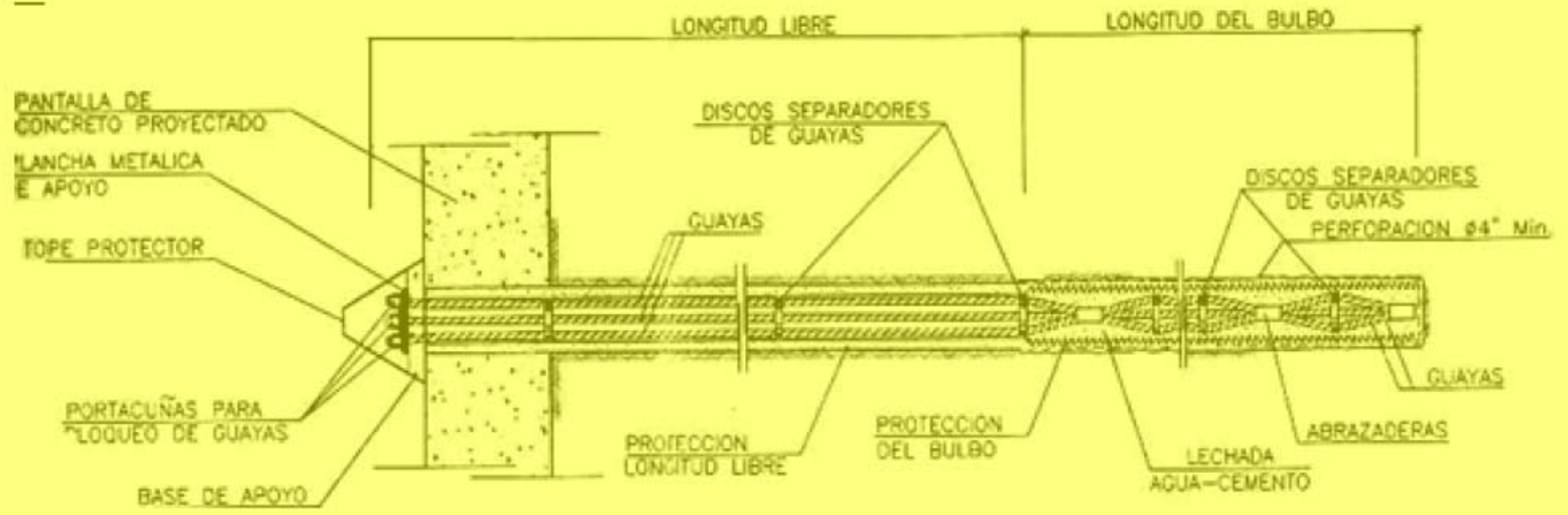
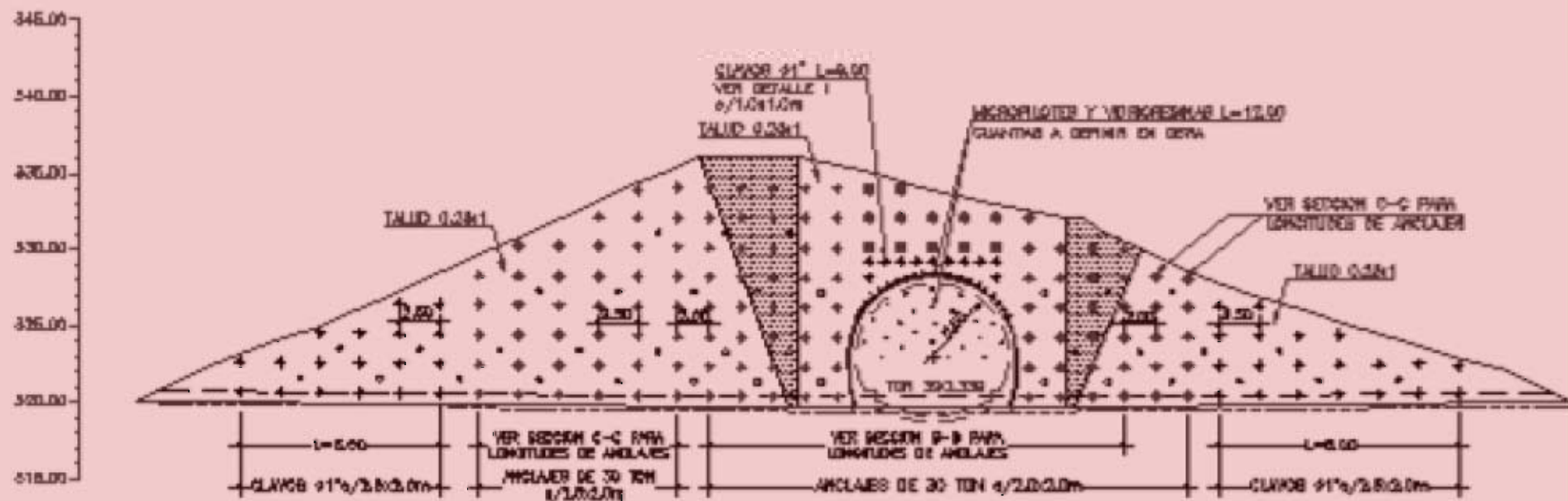


Mean F S=1.172
Standard Dev=0.0760

Reliability Index=2.245
Min F S=0.941

P (Failure) (%)=1.232
Max FS=1.438













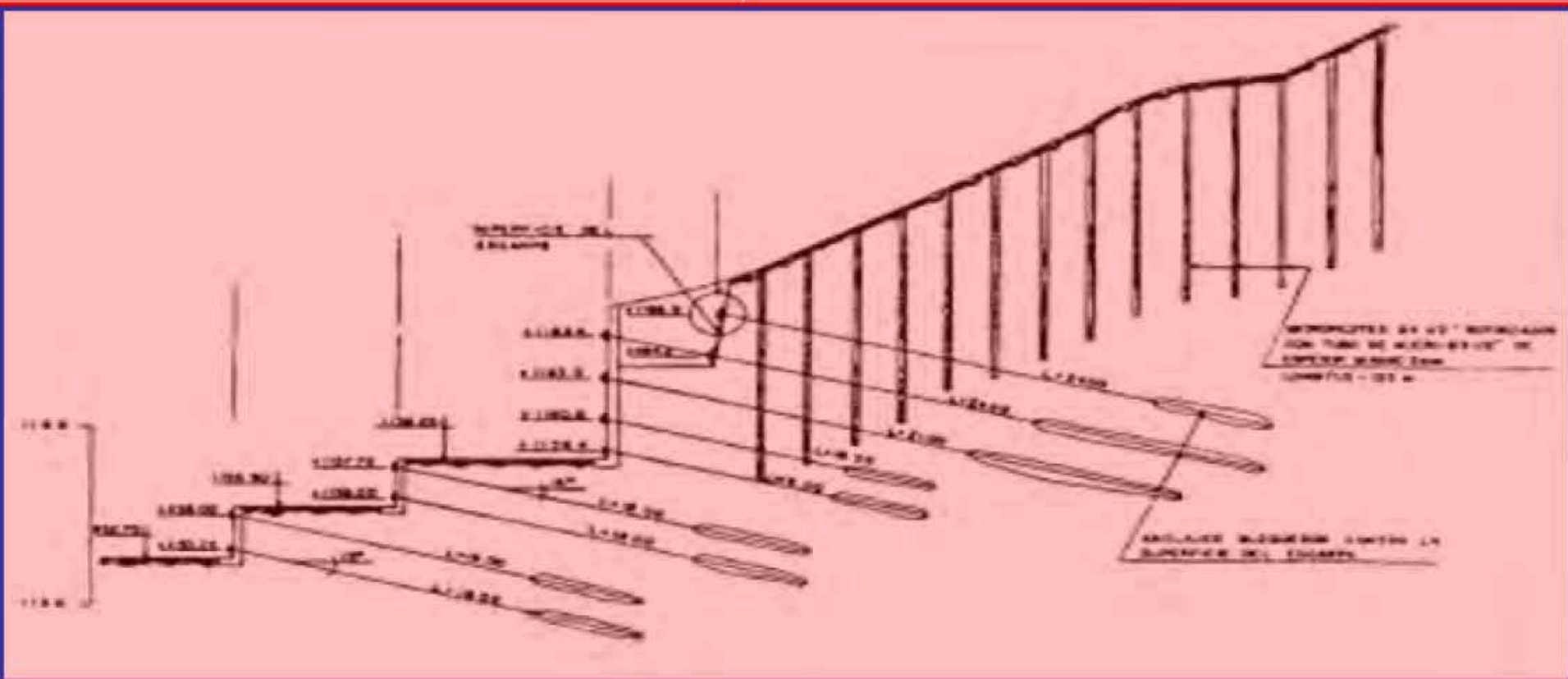
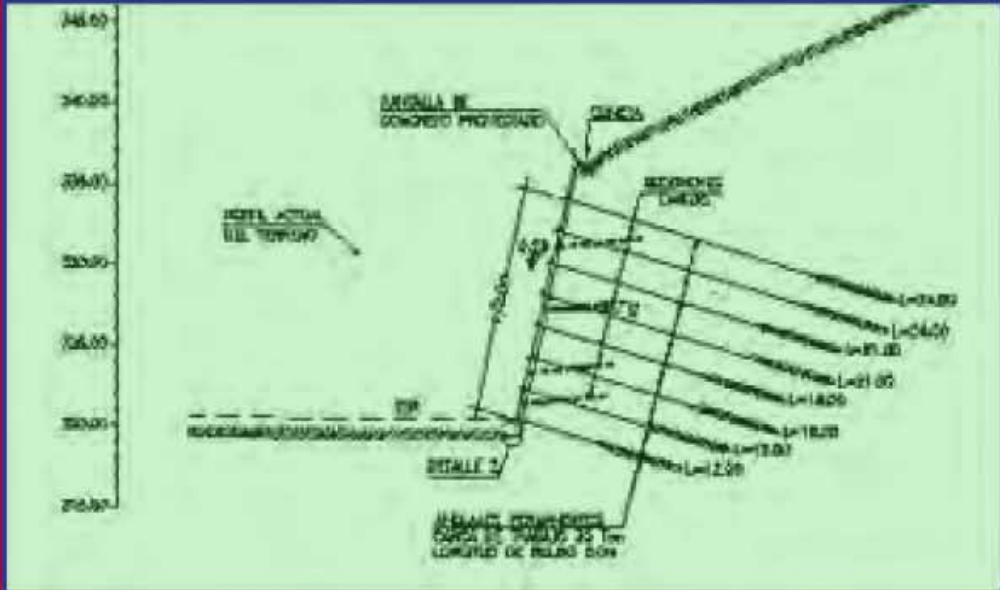
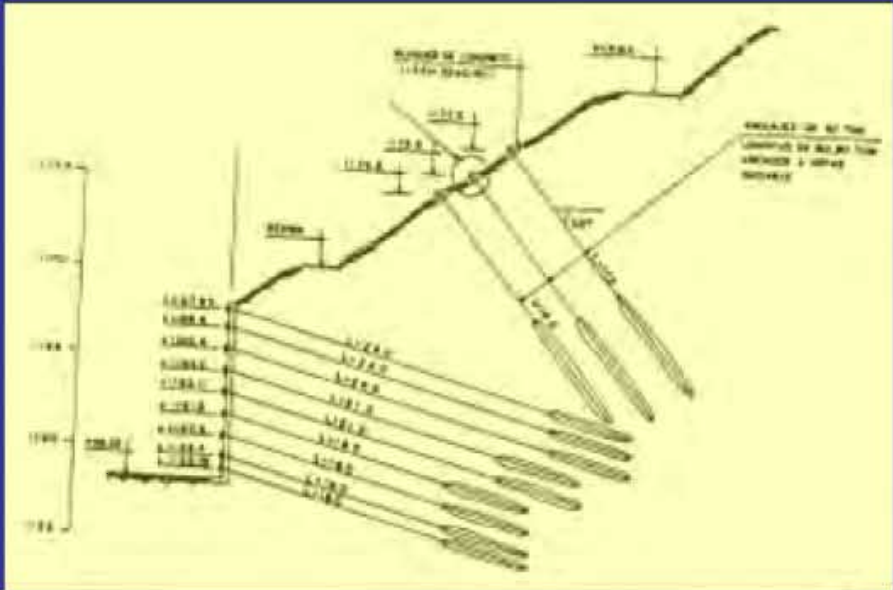


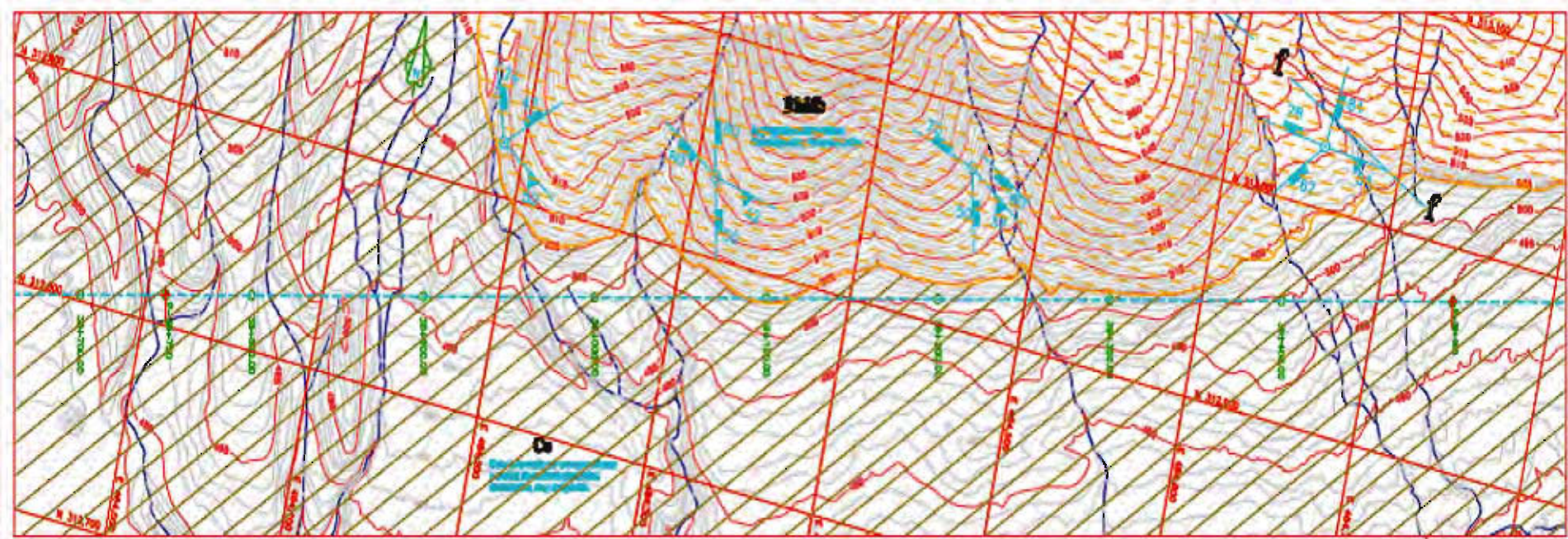






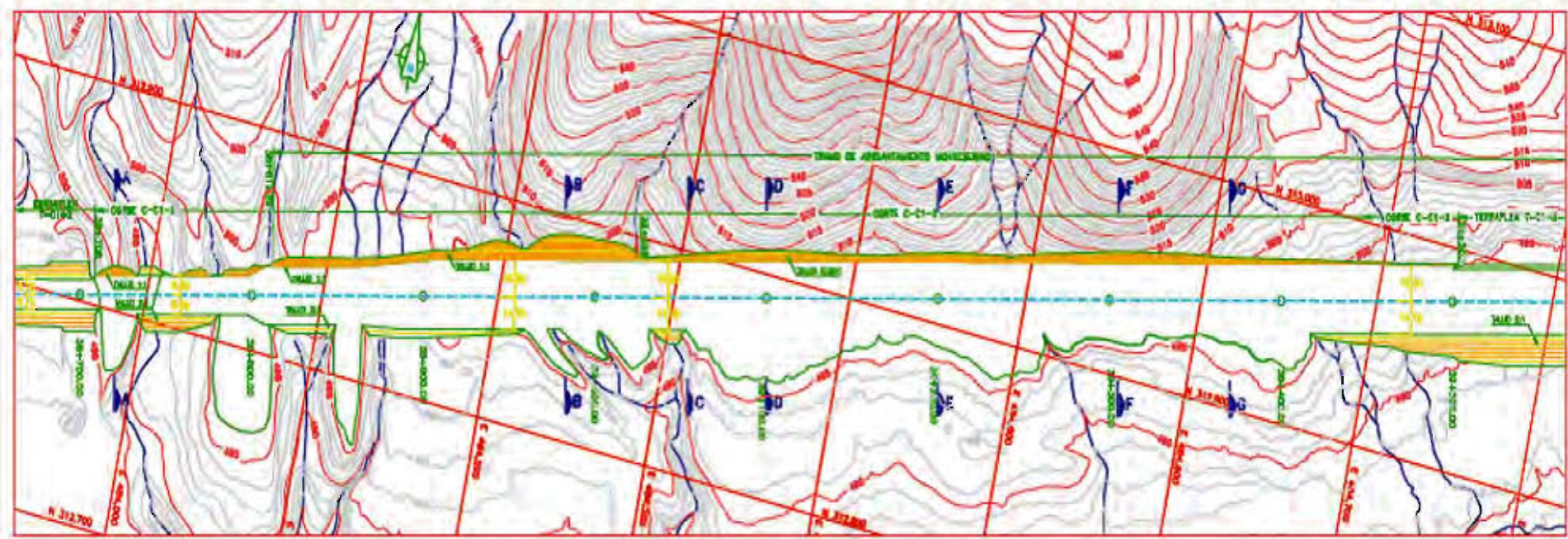




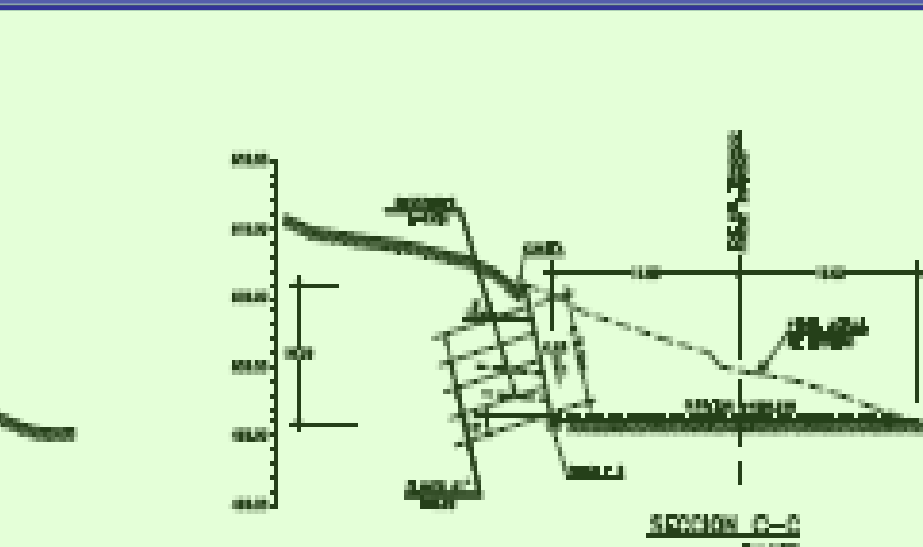
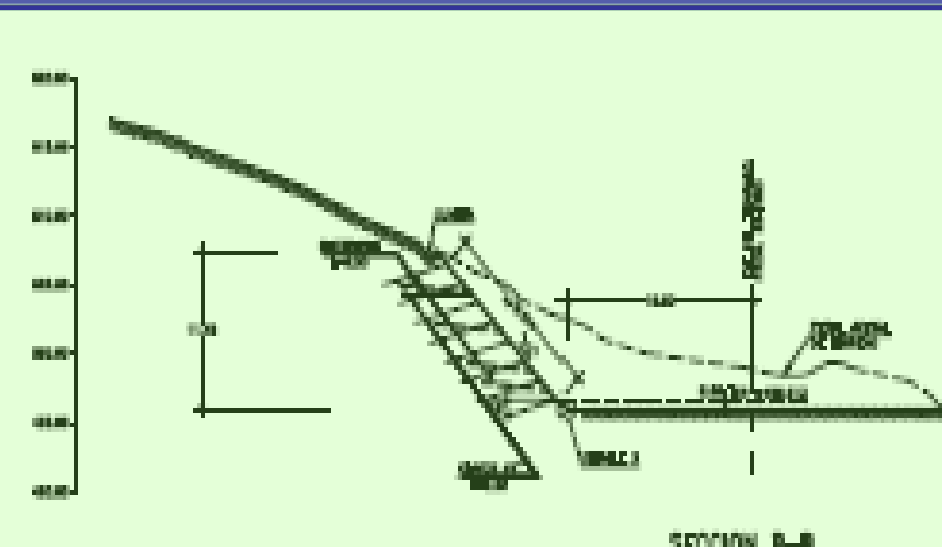
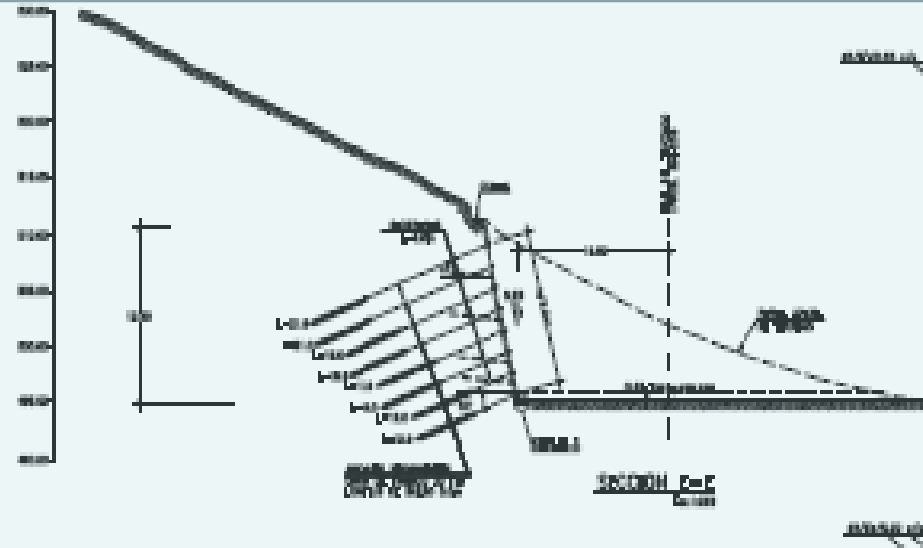
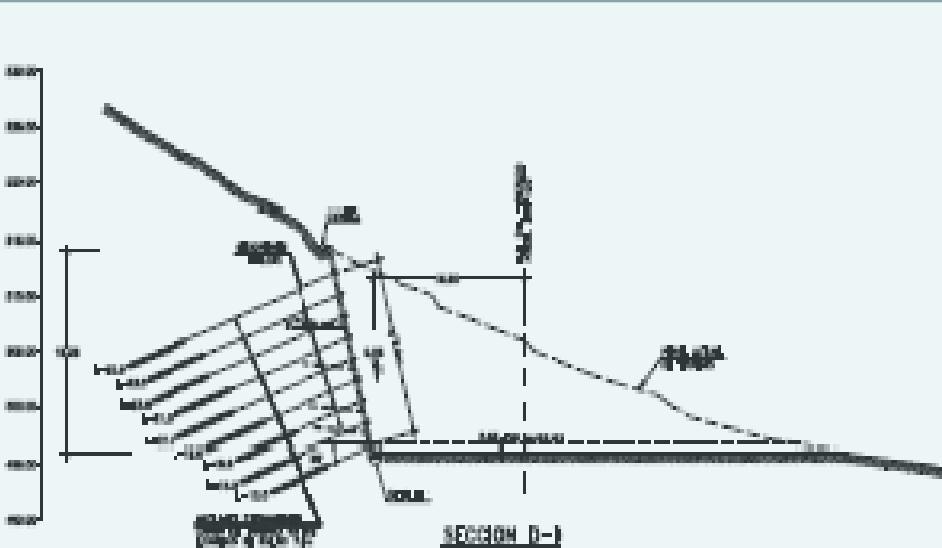
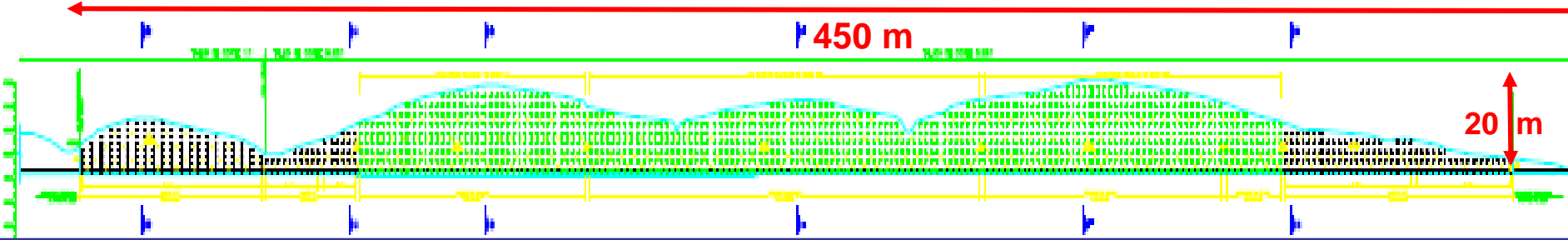


TOPOGRAFIA ACTUAL

PONTON DE CALDERA
 CANAL



TOPOGRAFIA MODIFICADA















2008/02/06 13:28



2008/02/06 13:27