

**FIBERGLASS BAFFLE WALL SYSTEMS**



*Chlorine Contact Basin at Keegan's Bayou WWTP, Houston, TX*

**Glass·Steel, Inc.**  
PO Box 7155  
The Woodlands, TX 77387-7155  
18468 FM 1314  
Conroe, TX 77302  
(281) 572-2211 office  
(281) 572-2212 fax  
[www.GlassSteelinc.com](http://www.GlassSteelinc.com)

---

## **BAFFLE WALL SYSTEM**

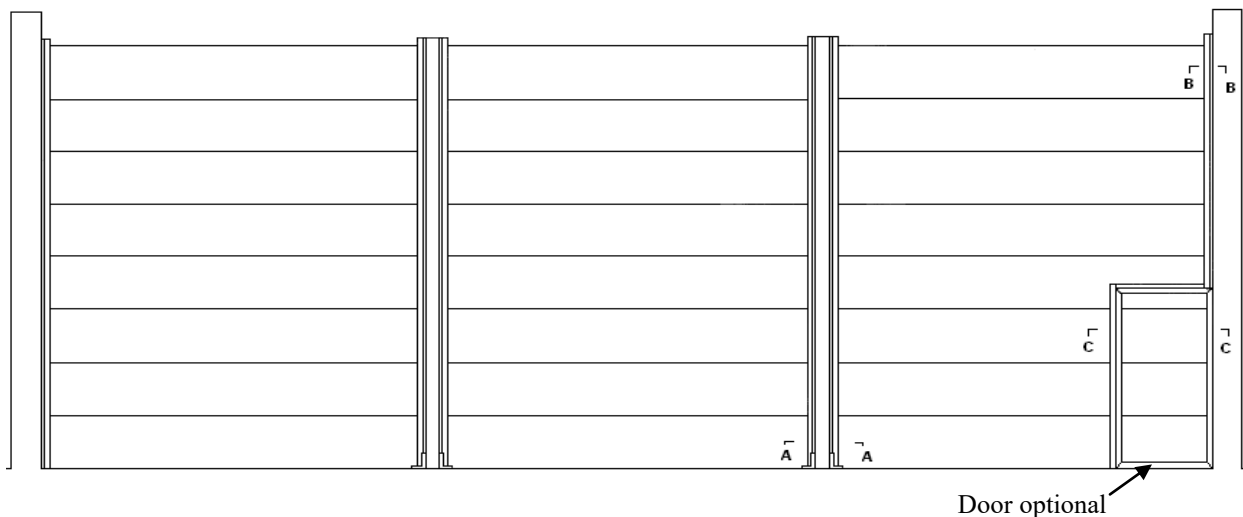
Glass-Steel, Inc. custom designs and fabricated baffle wall systems for use in clearwells. Chlorine Contact Chambers, Flocculation Basins, Aeration Chambers, Mixer Basins, etc. Glass-Steel, Inc. baffle wall systems are all fiberglass systems utilizing NSF approved pultruded components. Fiberglass baffle walls are usually lower in cost than clearheart redwood with stainless steel angle supports, poured in place concrete, aluminum, or stainless. Our fiberglass columns are less expensive than poured in place concrete, aluminum, or stainless of comparable strength.

Fiberglass of course will not rot, rust, or corrode and is virtually maintenance free. Fiberglass is lighter than most other materials and less costly to install. Our baffle wall system and components allow us to build framed door ways into the system for access to equipment when equipment needs to be maintained. Because all of our components are Strongwell's EX-TREN products we have a vast range of structural configurations we can use to accommodate special design considerations. Our support columns are generally a 6" x 9" x 5/16" tubular column or a 6" x 18" x 5/16" tubular column, but we also can utilize I Beams 18" deep, 24" deep, or even our 36" double web column. Our base connection is generally a 7" x 3-1/2" x 3/4" angle with a glass configuration and corner radius designed to maximize the effective strength of the component.

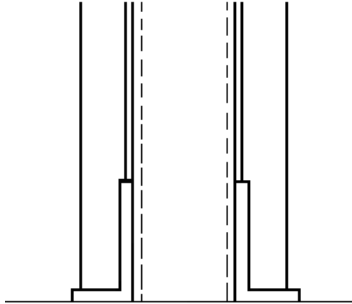
Glass-Steel, Inc. has installed numerous systems over the last several years and is constantly designing new components to improve the flexibility of the system and improve the cost effectiveness of the system. The following sketches give a few standard details currently being used but they in no way reflect our entire capability of options for these products. ■

3.70 LBS/ LINEAR FT (12" BAFFLE PANEL)

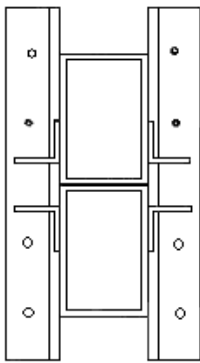
6.52 LBS/LINEAR FT (24" BAFFLE PANEL)



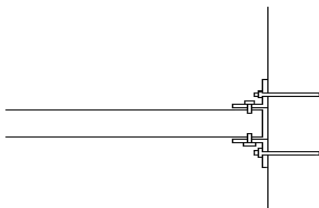
**Typical Baffle Wall Elevation**



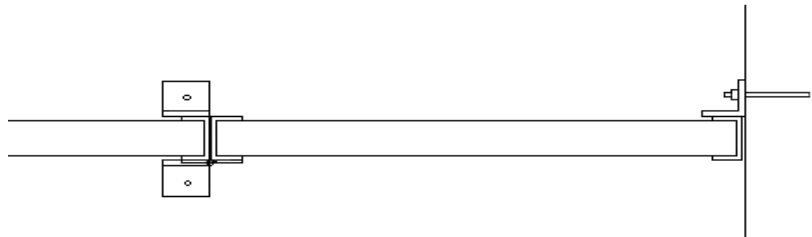
Column 6" x 18" x 5/16" rectangular tube  
All guide angles 3" x 3" x 3/8"  
Base Angles 7" x 3-1/2" x 3/4" x 2'0"  
Door and Jamb Framing 3-1/2" x 2" x 1/4" CHAN  
All anchors 1/2"ø  
All bolts for guide to column connection 3/8"ø



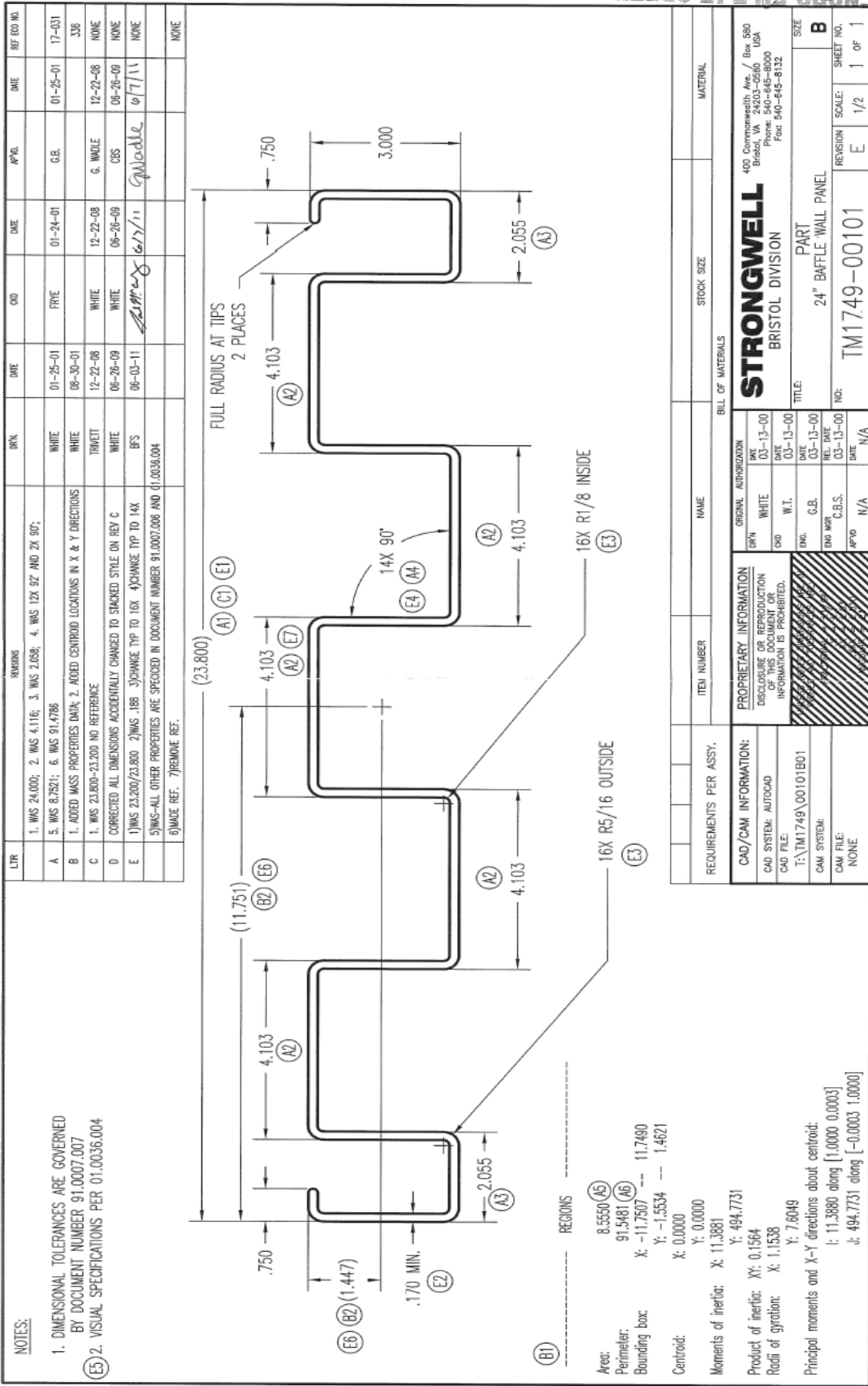
Section A-A  
Column & base plate detail



Section B-B  
End wall connection

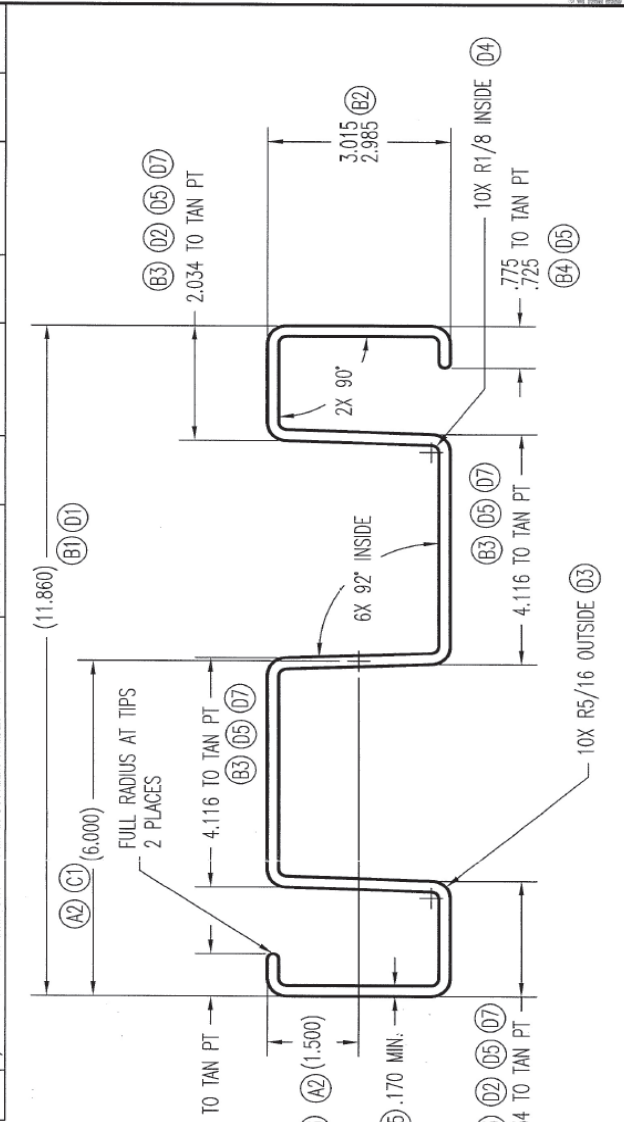


Section C-C  
Section through door



ENGINE LIFE IF GREEN  
 APPR'D BY: 6/7/11

LTR	REV	DATE	BY	CHKD	DATE	APPROV	REF EDO NO.
A	1.	08-30-01	WHITE	W.T.	08-30-01	G.B.	336
B	1.	05-22-02	WHITE	C.B.	05-22-02	GBS	814
C	1.	10-25-02	WHITE	W.T.	10-25-02	GBS	NONE
D	1.	06-03-11	BFS	W.T.	6/7/11	Quiselle	6/7/11



REQUIREMENTS PER ASSY.	ITEM NUMBER	NAME	BILL OF MATERIALS	STOCK SIZE	MATERIAL
CAD/CAM INFORMATION:					
CAD SYSTEM: AUTOCAD					
CAD FILE:					
CAM SYSTEM:					
CAM FILE:					
<b>STRONGWELL</b> BRISTOL DIVISION 400 Commonwealth Ave. / Box 860 Bristol, VA 24203-0860 USA Phone: 540-644-8100 Fax: 540-644-8132					
TITLE: PART NO: TM1748-00101 REVISION: D SCALE: 1/2 SHEET NO: 1 of 1					

NOTES:  
 1. DIMENSIONAL TOLERANCES ARE GOVERNED BY DOCUMENT NUMBER 91.0007.007  
 2. VISUAL SPECIFICATIONS PER 01.0036.004

REGIONS -----  
 (A1)

Area: 4.9699  
 Perimeter: 52.2208  
 Bounding box: X: -6.0000 --- 6.0000  
 Y: -1.5000 --- 1.5000

Centroid:  
 X: 0.0000  
 Y: 0.0000

Moments of inertia: X: 6.2217  
 Y: 83.8947

Product of inertia: XY: -1.1040  
 Radii of gyration: X: 1.1189  
 Y: 4.1086

Principal moments and X-Y directions about centroid:  
 I: 6.2061 along [0.9999 -0.0142]  
 J: 83.9104 along [0.0142 0.9999]

APPROVED BY: GREEN  
 DATE: 6/7/11

## LAB TEST RESULTS

### ASTM D695 Compression Test

CW	Ultimate Load	Ultimate Stress
Specimen-1	2,078	21,054
Specimen-2	2,133	22,198
Specimen-3	2,065	22,421
Specimen-4	2,031	21,957
Average	2,077	21,907
Std. Dev.	42	599
% Variance	2.04%	2.74%

### ASTM D695 Compression Test

LW	Ultimate Load	Ultimate Stress	Modulus
Specimen-1	5,898	60,679	3.069181
Specimen-2	5,796	63,414	3.382721
Specimen-3	5,217	58,225	2.857323
Specimen-4	5,561	58,722	2.892075
Average	5,618	60,260	3.045325
Std. Dev.	302	2,264	0.231641
% Variance	5.38%	3.91%	7.59%

### ASTM D2344 Short Beam Shear Test

CW	Ultimate Load	Ultimate Stress
Specimen-1	242	3,891
Specimen-2	183	2,942
Specimen-3	250	3,682
Specimen-4	254	3,774
Average	232	3,572
Std. Dev.	33	429
% Variance	14.30%	12.00%

### ASTM D790 Flexural Test

CW	Ultimate Load	Ultimate Stress	Modulus
Specimen-1	106	26,765	1.542683
Specimen-2	109	26,924	1.551389
Specimen-3	105	25,936	1.539374
Specimen-4	109	27,191	1.633966
Average	108	26,704	1.566853
Std. Dev.	2	541	0.045028
% Variance	1.76%	2.03%	2.87%

### ASTM D2344 Short Beam Shear Test

LW	Ultimate Load	Ultimate Stress
Specimen-1	292	4,695
Specimen-2	326	4,962
Specimen-3	325	4,880
Specimen-4	297	4,885
Average	310	4,856
Std. Dev.	18	113
% Variance	5.81%	2.34%

### ASTM D790 Flexural Test

LW	Ultimate Load	Ultimate Stress	Modulus
Specimen-1	123	31,158	1.49941
Specimen-2	128	36,060	1.715262
Specimen-3	128	30,794	1.494734
Specimen-4	117	28,338	1.498601
Average	124	31,588	1.552002
Std. Dev.	5	3,234	0.106658
% Variance	4.22%	10.24%	7.01%

Glass %

Spec Actual  
50% min 50.00%

ASTM D750 Water Absorbtion

Spec Actual  
0.6 max. (Extren) 0.22%

### ASTM D638 Tensil Test

LW	Ultimate Load	Ultimate Stress	Modulus
Specimen-1	3,634	39,934	2.734163
Specimen-2	3,850	40,704	2.674926
Specimen-3	4,370	47,707	3.089276
Specimen-4	3,995	41,485	2.589634
Average	3,962	42,306	2.767000
Std. Dev.	310	3,686	0.225367
% Variance	7.82%	8.67%	8.14%

### 24" BAFFLE CONCENTRATED LOAD TABLE

Result of actual test full section bending. Test done in Strongwell, Bristol, VA lab.

SPAN (ft.)	50 lbs.	100 lbs.	200 lbs.	300 lbs.	400 lbs.	500 lbs.	600 lbs.	Modulus (10 <sup>6</sup> psi)
8	.034"	.087"	.134"	.202"	.269"	.337"	.404"	2.51
10	.058"	.113"	.231"	.346"	.462"	.577"	.693"	2.86
12	.095"	.190"	.380"	.571"	.761"	.951"	1.141"	3.00
14	.148"	.298"	.595"	.894"	1.192"	1.491"	1.789"	3.04
16	.217"	.433"	.867"	1.301"	1.734"	2.168"	2.602"	3.12
18	.308"	.617"	1.235"	1.852"	2.485"	3.088"	3.704"	3.12
20	.413"	.828"	1.651"	2.477"	3.303"	4.128"	4.954"	3.20

Notes:

- 1) Concentrated load is applied across the width of the panel
- 2) These typical deflections could vary by 15%
- 3) Testing based on a simple beam deflection of a 24 ft. long baffle

Deflection in inches based on the following differential head pressures.

Simple spans ends not fixed.

Head Span	1"	2"	3"	4"	5"	6"	7"	8"	9"	10"	11"	12"	Effective Modulus of Elasticity
8'	0.017	0.035	0.052	0.070	0.087	0.105	0.122	0.140	0.157	0.175	0.192	0.211	2.405 x 10 <sup>6</sup>
9'	0.026	0.052	0.078	0.105	0.131	0.157	0.183	0.209	0.235	0.262	0.288	0.314	2.577 x 10 <sup>6</sup>
10'	0.037	0.074	0.112	0.149	0.186	0.223	0.261	0.298	0.335	0.372	0.410	0.447	2.749 x 10 <sup>6</sup>
11'	0.053	0.107	0.160	0.214	0.267	0.321	0.374	0.428	0.481	0.535	0.588	0.642	2.812 x 10 <sup>6</sup>
12'	0.074	0.148	0.222	0.296	0.371	0.445	0.519	0.593	0.667	0.741	0.815	0.889	2.875 x 10 <sup>6</sup>
13'	0.101	0.203	0.304	0.406	0.507	0.609	0.710	0.811	0.913	1.014	1.116	1.217	2.893 x 10 <sup>6</sup>
14'	0.136	0.271	0.407	0.542	0.678	0.813	0.949	1.085	1.220	1.356	1.491	1.627	2.911 x 10 <sup>6</sup>
15'	0.176	0.353	0.529	0.706	0.882	1.059	1.235	1.411	1.588	1.764	1.941	2.117	2.947 x 10 <sup>6</sup>
16'	0.226	0.452	0.677	0.903	1.129	1.355	1.580	1.806	2.032	2.258	2.484	2.709	2.983 x 10 <sup>6</sup>
17'	0.288	0.576	0.863	1.151	1.439	1.727	2.014	2.302	2.590	2.878	3.165	3.453	2.983 x 10 <sup>6</sup>
18'	0.362	0.723	1.085	1.447	1.809	2.170	2.532	2.894	3.255	3.617	3.979	4.340	2.983 x 10 <sup>6</sup>
19'	0.443	0.886	1.329	1.772	2.214	2.657	3.100	3.543	3.986	4.429	4.872	5.315	3.022 x 10 <sup>6</sup>
20'	0.537	1.074	1.610	2.147	2.684	3.221	3.758	4.295	4.832	5.368	5.905	6.442	3.062 x 10 <sup>6</sup>

Deflection in inches based on the following wind pressures.

Simple spans ends not fixed.

Wind Span	10psf	15psf	20psf	25psf	30psf	35psf	40psf	Effective Modulus of Elasticity
8'	0.067	0.101	0.135	0.168	0.202	0.235	0.269	2.405 x 10 <sup>6</sup>
9'	0.101	0.151	0.201	0.251	0.302	0.352	0.402	2.577 x 10 <sup>6</sup>
10'	0.144	0.216	0.288	0.359	0.431	0.503	0.575	2.749 x 10 <sup>6</sup>
11'	0.206	0.309	0.411	0.514	0.617	0.720	0.823	2.812 x 10 <sup>6</sup>
12'	0.285	0.428	0.570	0.713	0.855	0.998	1.140	2.875 x 10 <sup>6</sup>
13'	0.390	0.585	0.780	0.975	1.170	1.365	1.506	2.893 x 10 <sup>6</sup>
14'	0.521	0.782	1.043	1.304	1.564	1.825	2.086	2.911 x 10 <sup>6</sup>
15'	0.679	1.018	1.357	1.697	2.036	2.375	2.715	2.947 x 10 <sup>6</sup>
16'	0.868	1.303	1.737	2.171	2.605	3.040	3.474	2.983 x 10 <sup>6</sup>
17'	1.107	1.660	2.214	2.767	3.320	3.874	4.427	2.983 x 10 <sup>6</sup>
18'	1.391	2.087	2.782	3.478	4.173	4.869	5.564	2.983 x 10 <sup>6</sup>
19'	1.703	2.555	3.407	4.259	5.110	5.962	6.814	3.022 x 10 <sup>6</sup>
20'	2.065	3.097	4.130	5.162	6.194	7.227	8.259	3.062 x 10 <sup>6</sup>



Deflection in inches based on the following differential head pressures.

Simple spans ends fixed.

Head	1"	2"	3"	4"	5"	6"	7"	8"	9"	10"	11"	12"	Effective Modulus of Elasticity
Span													
8'	0.003	0.007	0.010	0.014	0.017	0.021	0.024	0.028	0.031	0.035	0.038	0.042	2.405 x 10 <sup>6</sup>
9'	0.005	0.010	0.016	0.021	0.026	0.031	0.037	0.042	0.047	0.052	0.058	0.063	2.577 x 10 <sup>6</sup>
10'	0.007	0.015	0.022	0.030	0.037	0.045	0.052	0.060	0.067	0.074	0.082	0.089	2.749 x 10 <sup>6</sup>
11'	0.011	0.021	0.032	0.043	0.053	0.064	0.075	0.086	0.096	0.107	0.118	0.128	2.812 x 10 <sup>6</sup>
12'	0.015	0.030	0.044	0.060	0.074	0.089	0.104	0.119	0.133	0.148	0.163	0.178	2.875 x 10 <sup>6</sup>
13'	0.020	0.041	0.061	0.081	0.101	0.122	0.142	0.162	0.183	0.203	0.223	0.243	2.893 x 10 <sup>6</sup>
14'	0.027	0.054	0.081	0.108	0.136	0.163	0.190	0.217	0.244	0.271	0.298	0.325	2.911 x 10 <sup>6</sup>
15'	0.035	0.071	0.106	0.141	0.176	0.212	0.247	0.282	0.318	0.353	0.388	0.423	2.947 x 10 <sup>6</sup>
16'	0.045	0.090	0.135	0.181	0.226	0.271	0.316	0.361	0.406	0.452	0.497	0.542	2.983 x 10 <sup>6</sup>
17'	0.058	0.115	0.173	0.230	0.288	0.345	0.403	0.460	0.518	0.576	0.633	0.691	2.983 x 10 <sup>6</sup>
18'	0.072	0.145	0.217	0.289	0.362	0.434	0.506	0.579	0.651	0.723	0.796	0.868	2.983 x 10 <sup>6</sup>
19'	0.089	0.177	0.266	0.354	0.443	0.531	0.620	0.709	0.797	0.886	0.976	1.063	3.022 x 10 <sup>6</sup>
20'	0.107	0.215	0.322	0.429	0.537	0.644	0.752	0.859	0.986	1.074	1.181	1.288	3.062 x 10 <sup>6</sup>

Deflection in inches based on the following wind pressures.

Simple spans ends fixed.

Wind Span	10psf	15psf	20psf	25psf	30psf	35psf	40psf	Effective Modulus of Elasticity
8'	0.013	0.020	0.027	0.034	0.040	0.047	0.054	2.405 x 10 <sup>6</sup>
9'	0.020	0.030	0.040	0.050	0.060	0.070	0.080	2.577 x 10 <sup>6</sup>
10'	0.029	0.043	0.058	0.072	0.086	0.101	0.115	2.749 x 10 <sup>6</sup>
11'	0.041	0.062	0.082	0.103	0.123	0.144	0.165	2.812 x 10 <sup>6</sup>
12'	0.057	0.086	0.114	0.143	0.171	0.200	0.228	2.875 x 10 <sup>6</sup>
13'	0.078	0.117	0.156	0.195	0.234	0.273	0.301	2.893 x 10 <sup>6</sup>
14'	0.104	0.156	0.209	0.261	0.313	0.365	0.417	2.911 x 10 <sup>6</sup>
15'	0.136	0.204	0.271	0.339	0.407	0.475	0.543	2.947 x 10 <sup>6</sup>
16'	0.174	0.261	0.347	0.434	0.521	0.608	0.695	2.983 x 10 <sup>6</sup>
17'	0.221	0.332	0.443	0.553	0.664	0.775	0.885	2.983 x 10 <sup>6</sup>
18'	0.278	0.417	0.556	0.696	0.835	0.974	1.123	2.983 x 10 <sup>6</sup>
19'	0.341	0.511	0.681	0.852	1.022	1.192	1.363	3.022 x 10 <sup>6</sup>
20'	0.413	0.619	0.826	1.032	1.239	1.445	1.652	3.062 x 10 <sup>6</sup>



*Fiberglass baffle walls in  
Clear Well at St. Cloud, FL*



*Access door in Baffle Wall showing hinge  
connection on opening side at St. Cloud,  
FL*



*Clearwell Baffle Wall and round  
concrete tank at St. Cloud, FL*



*Rear View of access door at St. Cloud, FL*





*“Vertical” installation of Fiberglass baffle walls  
Installed at Russian River, CA*



*“Vertical” installation of Fiberglass baffle walls  
Installed at Russian River, CA*



*Basin Installation at  
Russian River, CA*

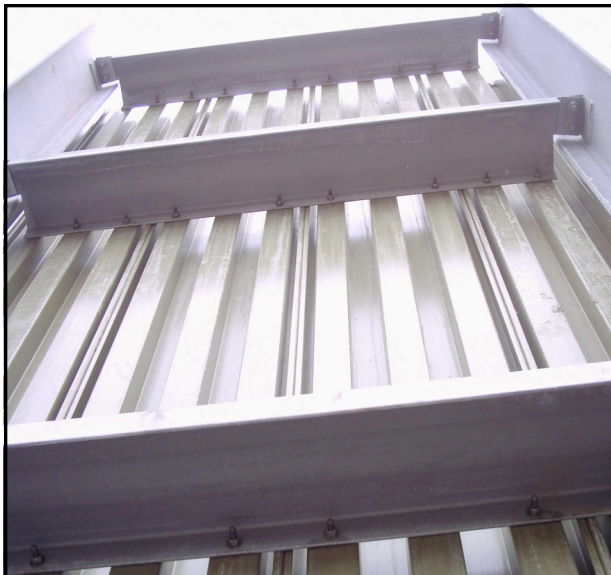


*Typical Baffle Wall Installation  
Keegan’s Bayou  
Houston, TX*

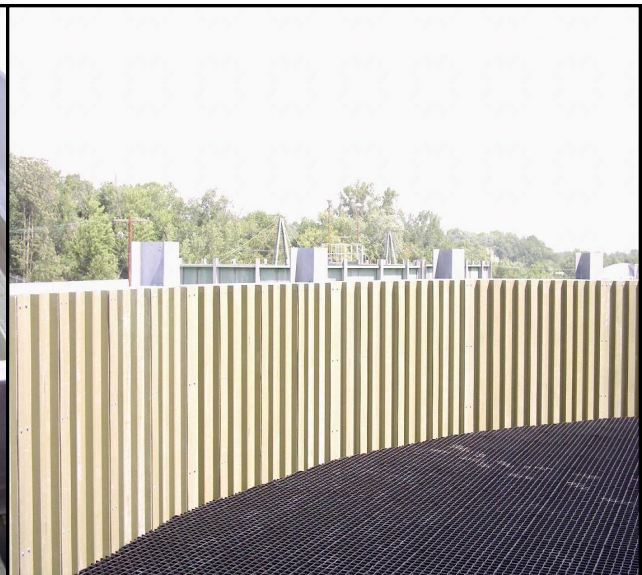




*Trickling Filter at Leavenworth, KS using baffle panel in vertical position with batten strips over seams.*



*Exterior close up showing attachment*



*Interior close-up showing batten strip over seams*

The baffle panel system is designed around Extren pultruded fiberglass products. The Extren product line has been around for over (40) years. Current composite designs have been fine tuned from this vast experience in laminate design. Fiberglass pultrusions are anisotropic in physical characteristics by nature of the process and by loading requirements. In normal structural applications we design the laminates to exhibit greater strength in the lengthwise direction than in the crosswise direction. In small compact parts such as round and square bar we use no transverse reinforcements. This allows our longitudinal values to soar to  $6.0 \times 10^6$  Modulus of Elasticity and 100,000 psi tensile strength. Our glass content on these parts is 70%. When parts become wider and thicker we drop our glass content to 50% to allow for fiberglass reinforcements to be placed in the transverse direction also. When fiberglass must overlap fiberglass in the longitudinal direction it takes up more space and reduces the amount of fiberglass reinforcement that can be placed into the part. Providing little, or no, transverse reinforcements in a wide section part can cause the laminate to crack during rough handling or when stresses are unexpectedly placed on the laminate in the transverse direction.

We are capable of altering our standard panel laminate design to a higher glass content and therefore attaining higher longitudinal properties. However the buyer should be aware of the potential consequences of neglecting transverse properties.

The load tables in this brochure indicate the deflection values of our standard laminate which is a 50% glass content part as indicated in the lab test also included. Higher glass content parts would result in lower deflection values.

The two chart sets displayed are for non fixed ends and fixed ends. On non fixed ends the panels are set between the guide angles loose for ease of potential removal or adjustment. The deflection charts for fixed ends are based on the guide angles being fixed to the wall or column in a continuous manner. Then the panels are stacked and bolted or screwed to the guide angles to attain fixity of the ends against little or no rotation of the members relative to each other. The angle between intersecting members is virtually unchanged under the design load because the deflections are so low ( $L/D < 360$ ) under most loads.

Most design loads are for when the structure is empty of water (when wind loads are used) or when the structure is being loaded or unloaded with water (when differential head is used) otherwise during operation loads are approximately equal.■

**Glass·Steel, Inc.**  
PO Box 7155  
The Woodlands, TX 77387-7155  
18468 FM 1314  
Conroe, TX 77302  
(281) 572-2211 office  
(281) 572-2212 fax  
[www.GlassSteelinc.com](http://www.GlassSteelinc.com)