WinCore

Version 3.1

C:\Jobs\1140225001\1140225001\_Box Culvert\_GBC-1.CLG

County Brazoria

Highway Green Tee Trail
CSJ 0912-31-291

DRILLING LOG

Structure Station Offset

Box Culvert 28+88.56 49.83' RT

District Houston Date 01-11-18 Grnd. Elev. 37.84 ft GW Elev. N/A

1 of 1

FIGURE A-1

		L			Triaxi	al Test		Prop	ertie	s	
Eld (fi	ev. )	O	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)			PI	Wet Den. (pcf)	Additional Remarks
		-		FILL, clay, very soft to stiff,			25.8			125.5	w/grass roots 0'-2' w/calcareous and ferrous
				reddish brown, dark gray, gray and brown	4	21	24.5	54	32	124.7	nodules 2'-6'
	_	-	3 (6) 4 (6)				21.3				% passing #200 sieve = 82% w/sand seams 2'-13'
	5	4									
		1			-		26.2				w/calcareous nodules 8'-13'
	10	-	4 (6) 4 (6)				25.0				
	10	4			40	40	25.0		20	405.7	8/i #200 eieue = 778/
24.8		1			13	13		- 63	39	125.7	% passing #200 sieve = 77%
	15	1	4 (6) 4 (6)	CLAY, soft, reddish brown and gray w/calcareous and ferrous			25.6				
	13			nodules (CH)			29.3	02	E 4		% passing #200 sieve = 99%
		K						. 03	54		76 passing #200 sieve = 5576
17.8	20	L	4 (6) 5 (6)				28.3				
17.0	20	+									
		7									
	25										
		+									
	30	_									
	-	-									
		7									
	35	_									
	•	-									
		7									
	40	1									
		+									
		-									
	45										
		-									
		-									
	50	_									
		1									
		+									
	55	7		A. Control							
		$\pm$									
	60										
Re	marl	ks: 1	Dry auger to 20.0	ft.							
Th	e gro	ound	water elevation was	not determined during the course of this b	oring.						
	_										
D	iller	: Elli	ot Van Antwerp	Logger: John A. Gen	try			0	rgan	ization:	Geotest Engineering, Inc.

#### DRILLING LOG

WinCore Version 3.1

County Brazoria Highway Green Tee Trail CSJ 0912-31-291

C:\Jobs\1140225001\1140225001\_Trail Pavement\_GBT-2,6,7.CLG

Trail Pavement Structure 47+10.59 Station 64.36' RT Offset

GBT-2

District Houston Date 01-27-18 Grnd. Elev. 40.27 ft GW Elev. N/A

1 of 1

FIGURE A-2

				Triaxi	al Test		Prop	ertie	es	
Elev. (ft)	C G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	мс	LL	PI	Wet Den. (pcf)	Additional Remarks
			FILL, clay, sandy, gray and yellowish brown w/sand seams, ferrous nodules				38	21	128.3	% passing #200 sieve = 78%
	+		and ferrous stains			12.7				
35.3 5	14	6 (6) 7 (6)	21.00			15.2				
			CLAY, soft, gray w/calcareous and ferrous nodules and ferrous stains (CH)			25.5	68	43		% passing #200 sieve = 94% yellowish brown & gray 8'-10
	-	9 (6) 9 (6)				21.2				
30.3 10	_	3 (0) 3 (0)								
	4									
	] [									
15	-									
	1									
	-									
20										
20	-									
	1									
	- 1									
25	1									
	+ 1									
30	7									
	<b>d</b> I									
	7									
35	1									
33	-									
	+ 1									
40										
	+									
	1									
45	-									
	1									
	-									
50										
-	-									
	-									1
55										
60										
Remar	'ks: 1)	Dry auger to 10.0	J π.							
The gro	ound v	water elevation wa	s not determined during the course of this b	oring.						
		hard Smith	Logger: John A. Gen							Geotest Engineering, Inc.

NO. DATE REVISION APPROV.



Houston, Texas 77079 T 281 589 7257 USInfrastructure@rpsgroup.com Formerly Klotz Associates, Inc. Texas PE Firm Reg. #F-929



#### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

BORE LOGS

SHEET 1 OF 5

FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.	
6				86	
STATE	DIST.		COUNTY		
TEXAS	HOU	BR.	AZORIA / H	HARRIS	
CONT.	SECT.	JOB	HIG	HWAY NO.	
0912	31	291		VA	

# WinCore Version 3.1

County Brazoria

C:\Jobs\1140225001\1140225001\_Pedestrian Bridge\_GBB-3,4.CLG

Highway Green Tee Trail
CSJ 0912-31-291

### DRILLING LOG

Structure Station 6+23.78 Offset

Pedestrian Bridge 30.92' RT

District Houston Date 01-10-18 Grnd. Elev. 36.88 ft GW Elev. N/A

1 of 2

		L	Texas Cone		Triaxi	al Test		Prop	ertie	s	
Ele (ft		O	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
				FILL, clay, gray, brown and dark			22.4			121.2	
34.9		-		gray w/grass roots	4	28	22.4	72	46	121.2	% passing #200 sieve = 81%
		-		CLAY, very soft to very stiff, gray and brown w/ferrous nodules	4	20		12	40	141.4	w/calcareous nodules 3'11"-6'
	5		3 (6) 3 (6)	and ferrous stains (CH)			21.4	-			
30.9		1		,							w/sand seams, calcareous and
		-		CLAY, sandy, gray and yellowish			18.6	49	28		ferrous nodules 6.5'-8'
8.4				brown (CL)			26.2				
	10		5 (6) 7 (6)	CLAY, soft to very stiff, reddish brown and gray w/calcareous nodules							
		4/		and sand seams (CH)							
		$\exists$		and sand seams (On)	13	30	24.1	72	45	128,2	% passing #200 sieve = 99%
							27.9				
	15		7 (6) 10 (6)								
0.9	10	/									
		-100		SAND, silty, brown (SM)			22.2				% passing #200 sieve = 27%
7.0							18.4				
7.9	20		8 (6) 9 (6)	CLAY, soft to very stiff, reddish			1014				w/calcareous nodules 20'-35'
		-		brown w/ferrous nodules and ferrous							
				stains (CH)			22.2				
							21.7				
	25		8 (6) 12 (6)								
		4/									
		٦.			28	52	24.6	80	51	128.1	% passing #200 sieve = 94% gray and brown 28'-35'
							25.6				3.0,
	30	$\supset$	7 (6) 9 (6)								
	00	-[-	4								w/silt seams 32'-35'
		-[-	ļ				25.4	_			W/SIII Seams 32 -35
		1					26.8				
	35		8 (6) 10 (6)								reddish brown and
	00		1								gray 35'-41'8" slickensided 35'-50'
		-/	1				25.4				slickensided 55 -50
			ļ		40	14	26.3	85	54	126.9	% passing #200 sieve = 100%
	40		9 (6) 14 (6)								w/calcareous nodules 40'-50'
	40	_/	1								
			1				17.0	-			gray and brown 41'8"-45'
			1				17.2				
	45	ıI,	14 (6) 18 (6)	-							reddish brown and gray 45'-50'
	40	1	ļ								
							18.2				-
		4	1		50	21	20.3	77	48	129.3	% passing #200 sieve = 98%
	50		15 (6) 17 (6)								reddish brown and gray,
	50	'-Ľ.									slickensided 50'-56'
							24.7				- 1
			1				27.6				
	55		14 (6) 19 (6)								
	-	-	1								yellowish brown & gray 56'-63'
			1				19.3				-
			4		60	19	21.6	72	46	130.1	
	60		11 (6) 15 (6)								w/calcareous nodules 60'-75'
Re	mai	rks: 1	Dry auger to 18.0	ft., wet rotary from 18.0 ft. to 80.0 ft. 2) F	ree wat	er first e	ncoun	terec	l at 1	8.0 ft. d	uring drilling; after
		1	5 min. at 13.0 ft. 3	Water depth at 12.5 ft. and hole open to	74.7 ft.	on 1-11-	18.				
The	e ar	ound	water elevation was	s not determined during the course of this b	orina.						
1.119	- yı	Sund	TOTAL CICENTION WAS	action of the same of the same of the same of							
									-		
Dr	rille	r: Elli	ot VanAntwerp	Logger: Felipe Game:	Z			0	rgan	ization:	Geotest Engineering, Inc.
											FIGURE A-3
C:X	Jobs	1114022	25001\1140225001_Pede:	strian Bridge_GBB-3,4.CLG							

### DRILLING LOG

WinCore Version 3.1 County Brazoria Highway Green Tee Trail CSJ 0912-31-291

GBB-3 Pedestrian Bridge Structure 6+23.78 Station 30.92' RT Offset

District Houston Date 01-10-18 Grnd. Elev. 36.88 ft GW Elev. N/A

2 of 2

		L	T		Triaxi	al Test		Prop	ertie	\$	
Ele (ft)	v. )	O G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
	_			CLAY, soft to very stiff, reddish							
	-			brown w/ferrous nodules and ferrous			24.0				gray, yellow and reddish
	-			stains (CH)			26.1				brown 63'-65'
	65 -		19 (6) 15 (6)								slickensided 63'-70' brown and gray 65'-68'
	-				Ì		20.6				
	_				70	40		62	20	424.0	reddish brown and gray 68'-7' % passing #200 sieve = 83%
	-		14 (6) 18 (6)		70	42	20.1	62	38	131.9	% passing #200 sieve = 63%
	70 -	/									
	-						18.2				w/calcareous nodules 73'-75'
36.1	_			CLAY, sandy, gray and yellowish			16.8				-
38.1	75 -	1	22 (6) 19 (6)	brown (CL) SAND, silty, compact, yellowish	-						
				brown (SM)			18.3				% passing #200 sieve = 16%
	-										
	-		33 (6) 38 (6)								
43.1	80 -										
	-	1									
	_										
	85	- 1									
	_	1									
	-	1									
	90 -	1									
	-	-									
	-	1									
	95 -	1									
	-	1									
	_	1									
	-	1									
	100-	1									
	-	-									
	-	1									
	105	1									
	-	1									
	-	- 1									
	110-										
	,										
	-	1									
	115-	1									
		1									
	-										
	400	1 1									
	120	4				and the state of			-4-	0.04	hair dilling often
Rer	mark	s: 1)	Dry auger to 18.0	0 ft., wet rotary from 18.0 ft. to 80.0 ft. 2) i ) Water depth at 12.5 ft. and hole open to	rree wa	on 1-11-1	ncount	tered	at 7	o.u II. 0	uring urilling; after
		10	mini. at 10.0 It. o	, Traces aspende in the and note open to							

Driller: Elliot VanAntwerp

Logger: Felipe Gamez

Organization: Geotest Engineering, Inc.

C:\Jobs\1140225001\1140225001\_Pedestrian Bridge\_GBB-3,4.CLG

FIGURE A-3a

Texas Department of Transportation

REVISION

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Formerly Klotz Associates, Inc. Texas PE Firm Reg. #F-929

APPROV.

#### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

BORE LOGS

SHEET 2 OF 5

NO. DATE

FED.RD. DIV.NO.		PROJECT NO.		SHEET NO.	
6				87	
STATE	DIST.		COUNTY		
TEXAS	HOU	BR.	AZORIA / H	HARRIS	
CONT.	SECT.	JOB	HIG	HWAY NO.	
0912	31	291		VA	$\neg$

WinCore

Version 3.1

### DRILLING LOG

County Brazoria

C:\Jobs\1140225001\1140225001\_Pedestrian Bridge\_GBB-3,4.CLG

Highway Green Tee Trail
CSJ 0912-31-291

GBB-4 Structure 7+78.90 Station Offset

District Houston Pedestrian Bridge Date 01-25-18 Grnd. Elev. 35.49 ft 60.92" RT GW Elev. N/A

1 of 2

FIGURE A-4

L			Triaxi	al Test		Prop	ertie	s	
Elev. O (ft) G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC		PI	Wet	Additional Remarks
34.5		FILL, clay	-		27.1	00	F0	124.9	dark gray, gray, reddish brown w/grass roots and sand
		CLAY, soft to very stiff, gray and yellowish brown and reddish			24.1	88	58		seams 0'-1'
5	7 (6) 8 (6)	brown and gray w/calcareous and ferrous nodules and ferrous stains	-		21.1				% passing #200 sieve = 88%
		(CH)			25.1				w/sand seams 3'-6'
10	5 (6) 6 (6)		10	26	23.4	61	38	129.5	% passing #200 sieve = 86%
/					24.1				
	40.40.47.40.				22.9				
15	16 (6) 17 (6)								
	1				20.3				
20	25 (6) 26 (6)		20	43	20.7	81	51	127.6	% passing #200 sieve = 99% reddish brown 20'-25'
	]				21.4				
					20.9				
25 -	24 (6) 27 (6)								gray and brown 25'-33'
	1				26.6				
1	40 40 00 40		30	24	27.5	85	54	126.3	% passing #200 sieve = 97%
30 -	18 (6) 23 (6)								
/					18.7				
35 -	18 (6) 19 (6)				25.0	78	50		% passing #200 sieve = 99% slickensided 35'-40'
	1				24.7				
	16 (6) 17 (6)		00	46	26.7	80	50	127.5	% passing #200 sieve = 100%
40	16 (6) 17 (6)				19.4				gray and brown 41'-47'
/	1				18.5				
45	20 (6) 26 (6)	-							
					16.6				very hard 48'-50'
	10 (0) 04 (0)		.50	92	18.5	65	40	135.1	% passing #200 sieve = 98%
50 -	19 (6) 21 (6)	-							
	1				26.0				-
	20 (6) 22 (6)				27.0				-
55 -	20 (6) 23 (6)	·							
+	1		-		24.3				-
60	19 (6) 20 (6)		60	42	29.3	74	46	125	% passing #200 sieve = 99% gray and brown 60'-63'
Remarks: 1	) Dry auger to 38.0	ft., wet rotary from 38.0 ft. to 80.0 ft. 2)	Water de	pth at 16	.6 ft. a	nd h	ole c	pen to	77.3 ft. on 1-26-18.
The ground	water elevation was	s not determined during the course of this	boring.						
Driller: De	nnis Smith	Logger: John A. Ger	itry		2110	0	rgan	ization:	Geotest Engineering, Inc.

#### DRILLING LOG

WinCore Version 3.1

County Brazoria Highway Green Tee Trail
CSJ 0912-31-291

Pedestrian Bridge Structure 7+78.90 Station 60.92' RT Offset

District Houston Date 01-25-18 Grnd, Elev. 35.49 ft GW Elev. N/A

2 of 2

Ele	v.	L	Texas Cone	Strata Description	Lateral	Deviator Stress	мс	LL	DI	Wet Den.	Additional Remarks
(ft	)	G	Penetrometer		(psi)	(psi)	IWC	LL	ы	(pcf)	
				CLAY, soft to very stiff, gray							
	-			and yellowish brown and reddish brown and gray w/calcareous and			23.3				
				ferrous nodules and ferrous stains			25,8				
	65 -		16 (6) 19 (6)	(CH)							
	_				68	46	21.5	65	40	131.2	% passing #200 sieve = 91%
	-						20.9				
	70 -		17 (6) 19 (6)		-		22.1				w/sand seams 70'-73'
	-	1					20.9				
37.5											
	-	H	32 (6) 43 (6)	CLAY, sandy, very stiff, gray	75	50	19.3	40	20	133.3	% passing #200 sieve = 84%
	75 -	Ħ	02 (0) 10 (0)	and yellowish brown w/calcareous nodules and sand seams (CL)							w/sandy silt 77'9"-78'
	-	H		, ,			14.9				% passing #200 sieve = 79%
42.5	-			SAND, silty, very dense, brown			19.6				w/sandy clay layer 79'-79.5'
44.5	80 -		50 (3.5) 50 (3.5)	(SM)	-						' ' '
	-	- 1									
	0.5	1									
	85 -										
	90 -	- 1									
		1									
		-									
	95 -										
		- 1									
		- 1			1						
	100	1 1									
		1									
	105	11									
		- 1									
		1									
		- 1									
	110-	1									
		-									
	115	+									
		-									
	120	1 1									
Rei		s: 1)	Dry auger to 38.0	ft., wet rotary from 38.0 ft. to 80.0 ft. 2)	Water d	epth at 16	3.6 ft. a	nd h	ole o	open to	77.3 ft. on 1-26-18.
The	a arc-	und -	vater elevation was	s not determined during the course of this	boring						
1.00	e groi	una V	vater elevation was	s not determined during the course of this	corning.						

Logger: John A. Gentry Driller: Dennis Smith

Organization: Geotest Engineering, Inc. FIGURE A-4a

C:\Jabs\1140225001\1140225001\_Pedestrian Bridge\_GBB-3,4.CLG

NO. DATE



REVISION

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Formerly Klotz Associates, Inc. Texas PE Firm Reg. #F-929

APPROV.

#### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

BORE LOGS

FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.	
6				88	
STATE	DIST.		COUNTY		
TEXAS	HOU	BR.	AZORIA / H	HARRIS	
CONT.	SECT.	JOB	HIG	HWAY NO.	
0912	31	291		VA	

SHEET 3 OF 5

C:\Jobs\1140225001\1140225001\_Parking Lot\_GBP-5.CLG

WinCore

Version 3.1

County Brazoria

Highway Green Tee Trail
CSJ 0912-31-291

# DRILLING LOG

Structure Station 1+28.03 Offset

Parking Lot 14.24' LT

District Houston Date 01-10-18 Grnd. Elev. 40.48 ft GW Elev. N/A

1 of 1

	-	Τ.				al Test		Prop	ertie	s	
Ele (ft	ev. :)	C	Donotromotor	Strata Description		Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
				FILL, clay, sandy, gray w/grass			25.6	41	22	121	% passing #200 sieve = 84%
38.5			-	CLAY, soft, gray and yellowish			22.9				
	_		5 (6) 5 (6)	brown w/ferrous nodules and ferrous			24.0				
	5	1		stains (CH)							brown and gray 6'-8'
			d .				23.6	73	45		% passing #200 sieve = 87.7 w/calcareous nodules 6'-10'
		1					30.4				reddish brown and gray 8'-10'
30.5	10	-	2 (6) 4 (6)		1						very soft 10'-11.5'
		1									
		+									
	15										
		+									
		7									
	20	. H									
	20	7									
		. +									
	25										
		-									
		1									
	30	-									
		1									
	35	; -									
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	40	, [		L							
		-									
		7									
	45	. 🗄									
	40	' 🗌									
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	50	' =									
		+									
		7									
	55	5 -									
1		7									
		$\exists$									
	60	1 -									
Re	ma	rks:	1) Dry auger to 10.0	) ft.							
Th	e gr	round	l water elevation wa	s not determined during the course of this b	oring.						
D	rille	r: El	liot VanAntwerp	Logger: John A. Gen	try			0	rgan	ization:	: Geotest Engineering, Inc.

DRILLING LOG

WinCore Version 3.1 County Brazoria Highway Green Tee Trail CSJ 0912-31-291

Structure Trail Pavement 83+72.15 Station Offset 30.88' LT

District Houston Date 01-11-18 Grnd. Elev. 37.85 ft GW Elev. N/A

1 of 1

	L	Texas Cone			al Test		Prop			
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
35.9			FILL, clay, reddish brown and gray w/grass roots				52	31	126.2	% passing #200 sieve = 79% w/sand seams and calcareou nodules 0'-2'
	$\overline{}$		CLAY, soft, gray and brown w/calcareou and ferrous nodules (CH)	IS		19.1				nodules 0 -2
5	1	9 (6) 10 (6)	and leffous floudies (Off)			16.0				
31.9			CLAY, sandy, soft, brown and gray			8.7	30	13		% passing #200 sieve = 82%
	-		w/sand seams, calcareous and ferrous	\$		8.0				, -
7.9 10		5 (6) 7 (6)	nodules (CL)			0.0				
	-									
	1									
15	-									
15	7									
	1									
	- 1									
20	7									
	- 1									
	7									
25	1 1									
	+ $ $									
	1									
30	- 1									
	11									
	<u> </u>									
35	-									
	11									
	- 1									
40	4 i									
	<u> </u>									
	-									
45	<b>1</b>					-				
	-									
	7									
50										
50	-									
	1									
	-									
55	7									
	1									
	-									
60 Bomar	ker 41	Day suggests 40	n ft							
rtemar	ns. 1)	Dry auger to 10.0	V 16.							

NO. DATE REVISION APPROV.



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Texas Department of Transportation

**GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL** 

BORE LOGS

SHEET 4 OF 5

SHEEL	4 OF 5				- 1
FED. RD. DIV. NO.		PROJECT NO.	SHEET NO.		
6				89	
STATE	DIST.		COUNTY		
TEXAS	HOU	BR.	AZORIA / H	HARRIS	
CONT.	SECT.	JOB	HIG	HWAY NO.	
0912	31	291		VA	

FIGURE A-5

Driller: Elliot Van Antwerp

Logger: John A. Gentry

Organization: Geotest Engineering, Inc.

FIGURE A-6

C:\Jobs\1140225001\1140225001\_Trail Pavement\_GBT-2,6,7.CLG

WinCore

Version 3.1

County Brazoria Highway Green Tee Trail CSJ 0912-31-291

C:\Jobs\1140225001\1140225001\_Trail Pavement\_GBT-2,6,7.CLG

# DRILLING LOG

Structure Station Offset

Trail Pavement 13+95.95 6.40' LT

District Houston Date 01-10-18 Grnd. Elev. 44.26 ft GW Elev. N/A

1 of 1

FIGURE A-7

		L	T 0		Triax	ial Test		Prop	ertie	s	
Ele (ft	ev. )	O G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
				FILL, clay, gray, yellowish brown and gray w/sand seams			19.5 19.8	51	30	123	w/grass roots and limestone 0'-2' % passing #200 sieve = 88%
40.4	5		5 (6) 5 (6)	CLAY, soft, gray w/ferrous nodules and ferrous stains (CH)			19.6				Annual Property of the Control of th
							25.0				
34.3	10		4 (6) 6 (6)				24,3				
	15										
	20										
		-									
	25										
	30										
	35	-									
		3									
	40										
		3									
	45										
		1									
	50	-									
		3									
	55	7									
В-	60	<u> </u>	Des essente 40 /	4							
Re	mari	KS: 1	) Dry auger to 10.0	л.							
Th	e gro	ound	water elevation wa	s not determined during the course of this b	oring.						
D	riller	: Elli	ot Van Antwerp	Logger: John A. Gen	try			Or	rgan	ization	: Geotest Engineering, Inc.

NO. DATE

I160 Dairy Ashford, Suite 500 Houston, Texas 77079 T 281 589 7257 USInfrastructure@rpsgroup.com Formerly Klotz Associates, Inc. Texas PE Firm Reg. #F-929

Texas Department of Transportation

REVISION

### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

BORE LOGS

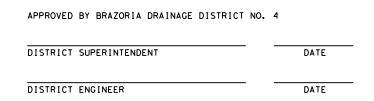
CUEET E OF E

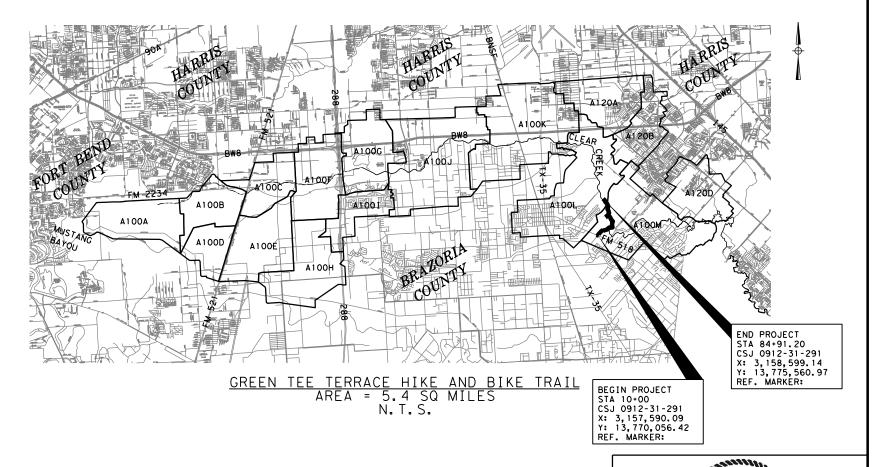
SHEET	5 OF 5				- 1		
FED. RD. DIV. NO.		PROJECT NO. SHEET NO.					
6				90			
STATE	DIST.		COUNTY				
TEXAS	HOU	BR.	BRAZORIA / HARRIS				
CONT.	SECT.	JOB	HIGHWAY NO.				
0012	7.1	201	) 1 VA				

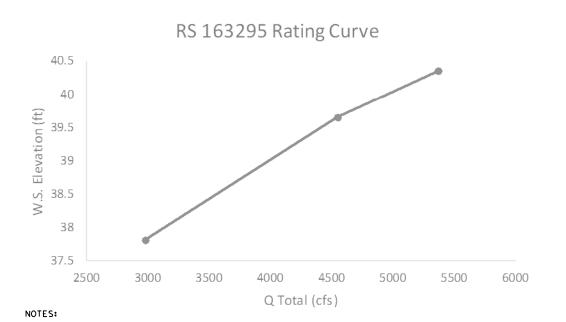
0912 31 291

	DOWNSTREAM	FREQUENCY	FLOW	COMPUTE	D WATE	R SURFACE	ELEVATION (fi
HEC-RAS STATION	REACH LENGTHS	(YR)	(CFS)	Effective	Rev Ex	Proposed	DIFF (Prop - Rev Ex
167264.8		10	2982	39.05	39.06	38.98	(0.08
	98.2	50	4553	40.78	40.78	40.73	(0.05
		100	5376	41.39	41.39	41.36	(0.03
167166.6		10	2982	39	39.01	38.93	(0.08
	540.7	50	4553	40.71	40.71	40.67	(0.04
		100	5376	41.33	41.33	41.29	(0.04
		10	2982	38.93	38.94	38.85	(0.0)
166625.9	642	50	4553	40.64	40.64	40.59	(0.0)
		100	5376	41.27	41.26	41.23	(0.03
		10	2982	38.79	38.8	38.71	(0.0)
165983.9	747.8	50	4553	40.49	40.5	40.44	(0.0)
		100	5376	41.12	41.12	41.08	(0.0)
		10	2982	38.6	38.61	38.51	(0.1
165236.1	678.4	50	4553	40.31	40.31	40.25	(0.0
		100	5376	40.95	40.94	40.9	(0.0
		10	2982	38.36	38.37	38.26	(0.1
164557.7	755.4	50	4553	40.1	40.1	40.04	(0.0
		100	5376	40.76	40.75	40.71	(0.0
		10	2982	38.02	38.03	37.9	(0.1
163802.3	502.3	50	4553	39.83	39.83	39.75	(0.0
		100	5376	40.52	40.52	40.46	(0.0
		10	2982		37.83	37.8	(0.0
163300	5	50	4553		39.66	39.63	(0.0
		100	5376		40.36	40.34	(0.0)
		10	2982		37.82	37.81	(0.0
163295	19	50	4553		39.65	39.65	-
		100	5376		40.36	40.35	(0.0)
163286		Propo	osed Hike and Bi	ke Pedest	rian Bric	lge	
		10	2982		37.82	37.81	(0.0)
163276	10	50	4553		39.65	39.64	(0.0
		100	5376		40.35	40.35	-
		10	2982		37.81	37.77	(0.0
163266	190.6	50	4553		39.65	39.61	(0.0
		100	5376		40.35	40.31	(0.0
		10	2982	37.75	37.75	37.75	-
163075.4		50	4553	39.59	39.59	39.59	-
		100	5376	40.3	40.3	40.3	-

\* SEE BRIDGE LAYOUT PEDESTRIAN BRIDGE OVER CLEAR CREEK FOR BRIDGE PROFILES & ADDITIONAL DATA.



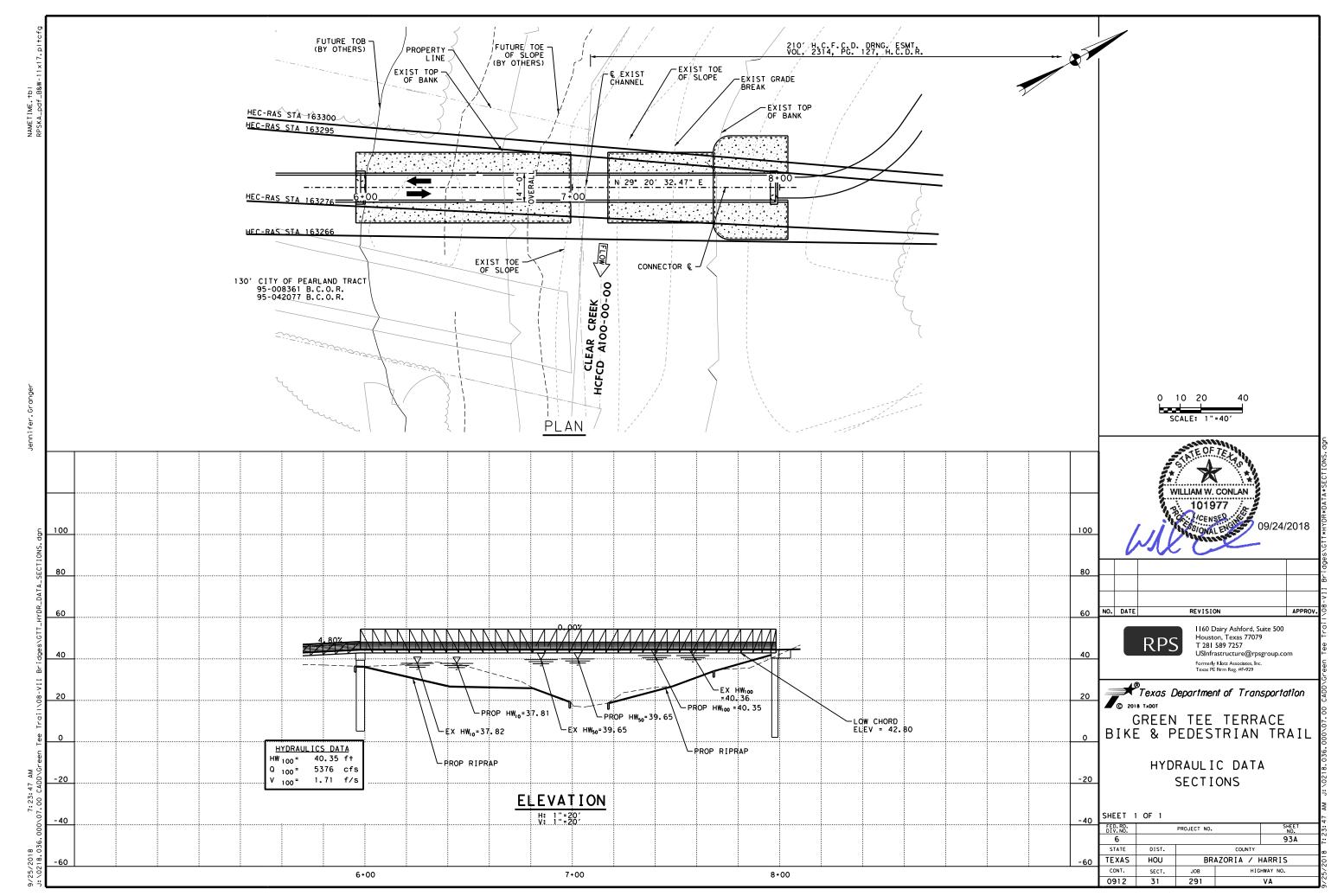


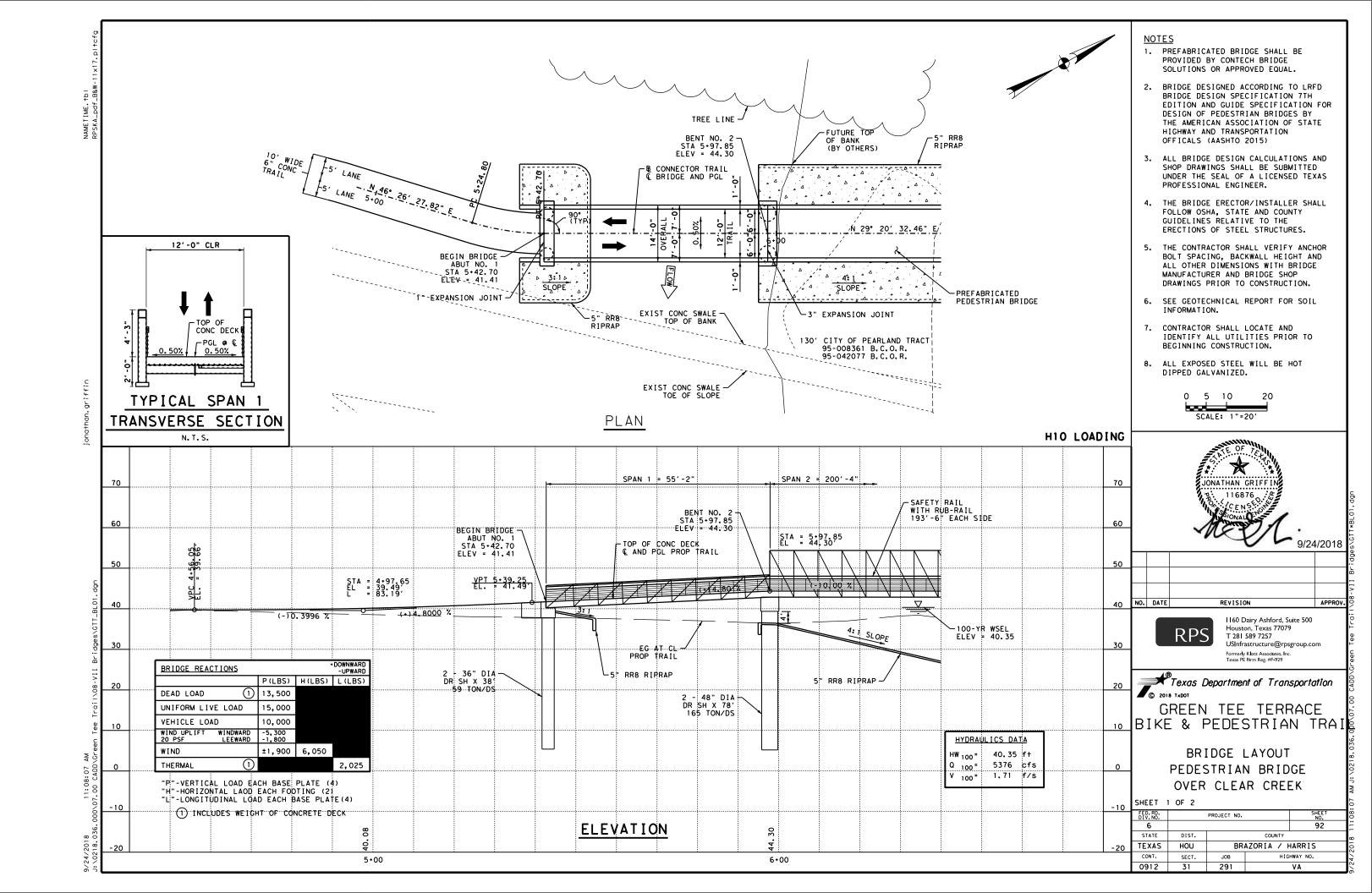


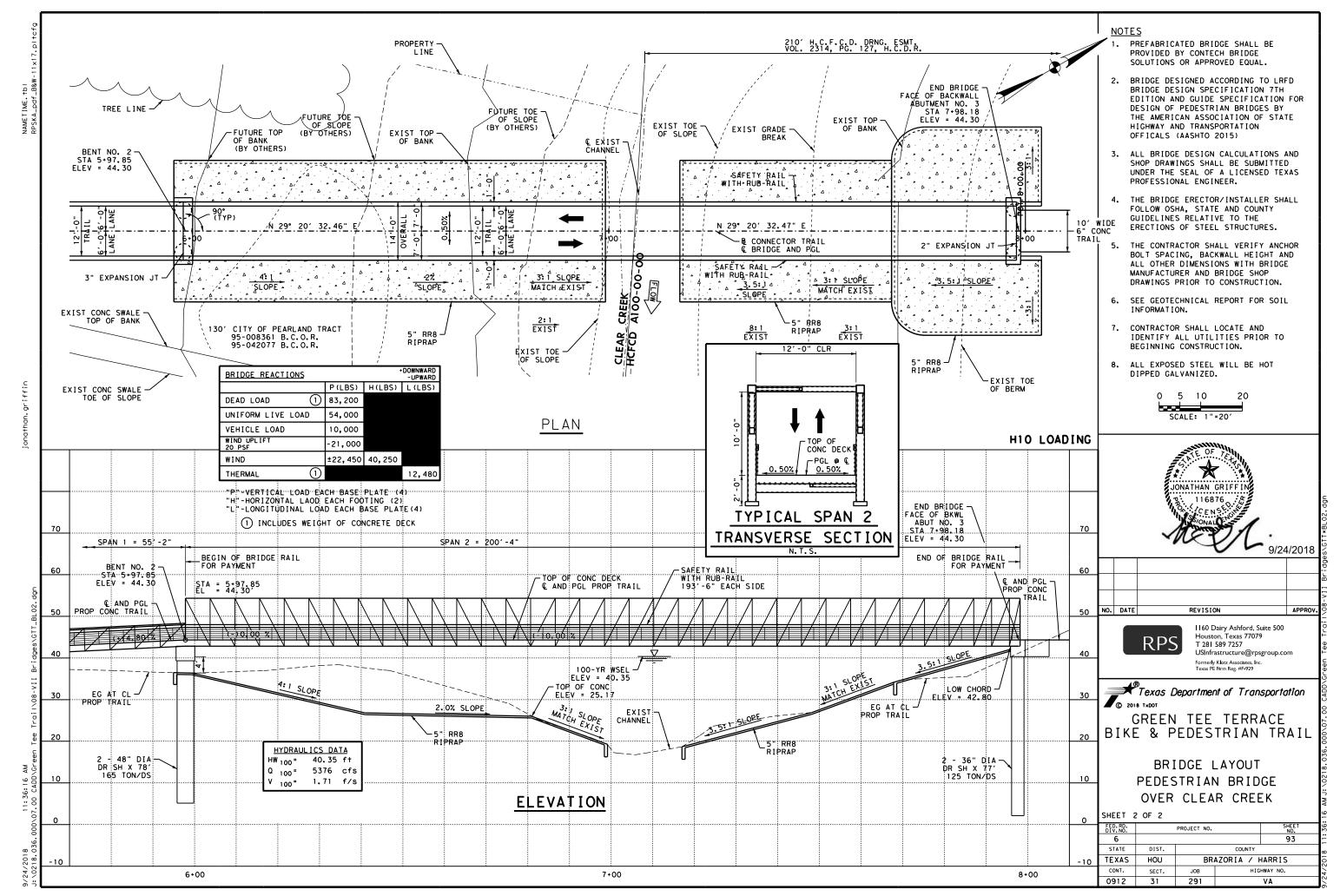
- 1. MODEL PULLED FROM THE HCFCD MODEL AND MAP MANAGEMENT (M3) SYSTEM
- 2. HEC-RAS VERSION 3.0.1 FOR HYDRALIC DESIGN ANALYSIS
- 3. ELEVATIONS BASED ON NAVD 88 VERTICAL DATUM 2001 ADJUSTMET
- 4. DRAINAGE IMPACT STUDY = "DRAINAGE IMPACT ANALYSIS GREEN TEE TERRACE TRAIL CLEAR CREEK (A100-00-00)" DATED 9/18/2018 HCFCD PROJECT NO. 1807090097
- 5. FLOODPLAIN COORDINATION DATE HCFCD: JANUARY 10, 2018. BDD4: DECEMBER 1, 2017.



SHEET	1 OF 1						
FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.			
6				91			
STATE	DIST.		COUNTY				
TEXAS	HOU	BR.	AZORIA / H	HARRIS			
CONT.	SECT.	JOB	HIGHWAY NO.				
0912	31	291	VΔ				







		SUN	MARY OF BR	IDGE QUANTI	ΓIES
		0400 6005	416 6004	0416 6006	C
PROJECT TRAIL ALIGNMENT	LOCATION	CEM STABIL BKFL	DRILL SHAFT (36 IN)	DRILL SHAFT (48 IN)	CL C (
		CY	LF	LF	
TRAIL CONNECTOR	1+00 TO 8+00	22	230	156	
PROJECT	TOTALS	22	230	156	
PROJEC		7640 SS	7640 SS		
TRAIL ALIGNMENT	LOCATION	PEDESTRIAN TRUSS BRIDGE SPAN	PEDESTRIAN APPROACH RAMP		
		EA	EA		
TRAIL CONNECTOR	1+00 TO 8+00	1	1		
PROJECT	TOTALS	1	1		

0420 6013

CY

18.2

18.2

0420 6025

CY

9.3

9.3

CL C CONC (ABUT) CL C CONC (BENT)

0432 6008

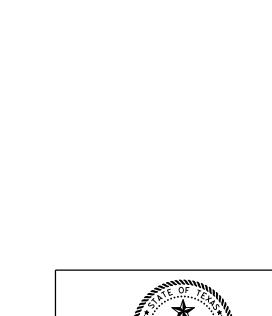
RIPRAP (CONC)(CL

B) (RR8&RR9)

CY

249

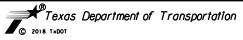
249



NO. DATE REVISION



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### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

BRIDGE QUANTITIES

SHEET	1 OF 1				╝		
FED. RD. DIV. NO.		SHEET NO.					
6				94			
STATE	DIST.		COUNTY				
TEXAS	HOU	BRAZORIA / HARRIS					
CONT.	SECT.	JOB	HIGHWAY NO.				
0912	3.1	201	VA				

2-21/2"

3'-0"

ELEV. 42.30' -

ANCHOR BOLT SPACING DRILLED SHAFT

ELEV. 42.30' -

BARS S SPA~3"

FWD

11'-7"

10'-0" 16'-0"

<u>PL AN</u>

ELEV. 38.80'

FD STANDARD FOR -ALL FOUNDATION DETAILS AND NOTES

8 EQ SPA =6'-0"

BENT NO. 2 -STA 5+97.85

-48" DRILLED SHAFT

8'-0"

2-21/2"

1'-9"

3'-0"

-@ BENT AND DRILLED SHAF1

- C GREEN TEE TERRACE HIKE & BIKE TRAIL

8'-0"

ANCHOR BOLT SPACING

-ELEV. 42.30'

BARS S SPA~3"

BARS S

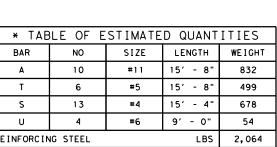
-ELEV. 38.80'

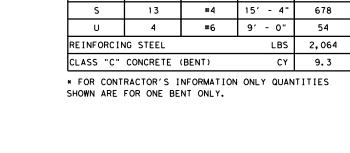
-ELEV. 42.30'

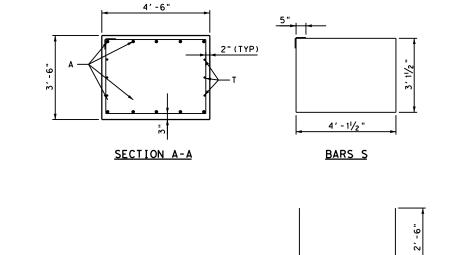
ANCHOR BOLTS (BY BRIDGE FABRICATOR)



* TAE	* TABLE OF ESTIMATED QUANTITIES								
BAR	NO	SIZE	LENGTH	WEIGHT					
Α	10	#11	15′ - 8"	832					
Т	6	#5	15′ - 8"	499					
S	13	#4	15' - 4"	678					
U	4	#6	9′ - 0"	54					
RE INFORC IN	REINFORCING STEEL LBS 2,064								
CLASS "C"	CLASS "C" CONCRETE (BENT) CY 9.3								





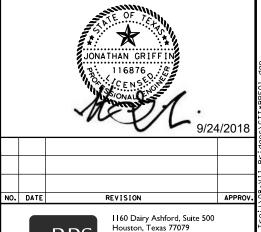




4'-0"

- BRIDGE BENT DESIGNED ACCORDING TO LRFD BRIDGE DESIGN SPECIFICATION 7TH EDITION AND GUIDE SPECIFICATION FOR DESIGN OF PEDESTRIAN BRIDGES BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICALS (AASHTO 2015)
- 2. ALL CONCRETE STRENGTH f'c=3600 psi.
- 3. REINFORCING SHALL BE GRADE 60.
- SEE FD STANDARD FOR FOUNDATION DETAILS AND NOTES.
- SEE BRIDGE LAYOUT FOR DRILLED SHAFT LENGTHS.
- 6. SEE ABUTMENT DETAILS SHEET FOR ADDITIONAL INFORMATION.

  CONTRACTOR MUST VERIFY ALL ELEVATIONS
- AND DIMENSIONS OF PREFABRICATED SPANS UPON DELIVERY.
- ALL PARTS ASSOCIATED WITH THE ANCHORAGE OF THE PREFABRICATED BRIDGE TO THE PROPOSED ABUTMENT ARE TO BE DESIGNED BY THE BRIDGE MANUFACTURER PRIOR TO THE CONSTRUCTION OF THE ABUTMENT. THE CONTRACTOR MUST ALSO VERIFY THE LOCATION, NUMBER AND SIZE OF THE ANCHOR BOLTS WITH THE BRIDGE MANUFACTURER PRIOR TO CONSTRUCTION.



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Texas Department of Transportation

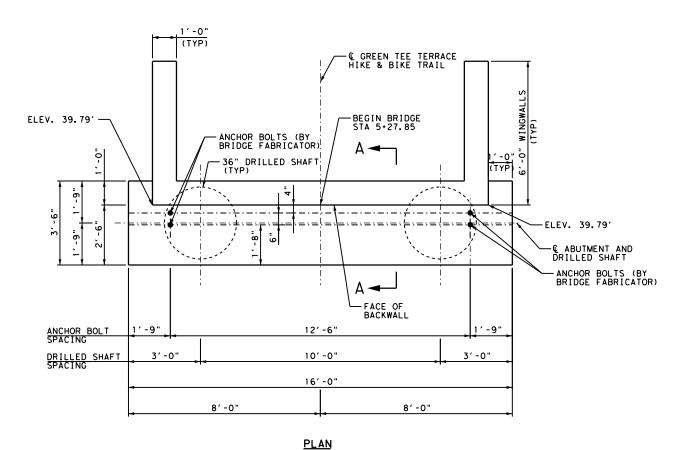
GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL

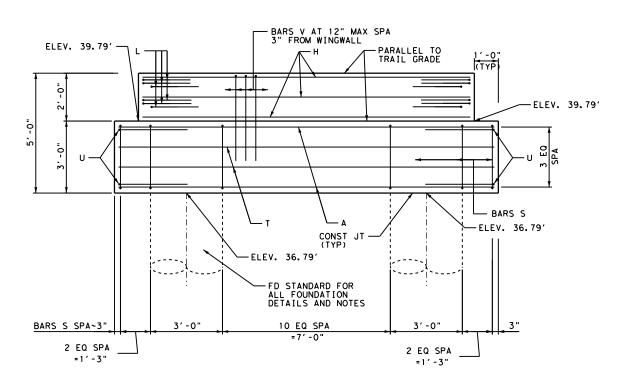
BENT 2

SHEET	1 OF
-------	------

SHEET	i OF i						
FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.			
6				95			
STATE	DIST.		COUNTY				
TEXAS	HOU	BR.	BRAZORIA / HARRIS				
CONT.	SECT.	JOB	HIGHWAY NO.				
0912	31	291	VA				

H10 LOADING

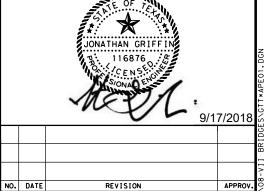




ELEVATION

#### NOTES:

- 1. BRIDGE ABUTMENT DESIGNED ACCORDING TO LRFD BRIDGE DESIGN SPECIFICATION 7TH EDITION AND GUIDE SPECIFICATION FOR DESIGN OF PEDESTRIAN BRIDGES BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICALS (AASHTO 2015)
- 2. ALL CONCRETE STRENGTH f'c=3600 psi.
- REINFORCING SHALL BE GRADE 60.
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- SEE BRIDGE LAYOUT FOR DRILLED SHAFT LENGTHS.
- 6. SEE ABUTMENT DETAILS SHEET FOR ADDITIONAL INFORMATION.
- CONTRACTOR MUST VERIFY ALL ELEVATIONS AND DIMENSIONS OF PREFABRICATED SPANS UPON DELIVERY.
- . ALL PARTS ASSOCIATED WITH THE ANCHORAGE OF THE PREFABRICATED BRIDGE TO THE PROPOSED ABUTMENT ARE TO BE DESIGNED BY THE BRIDGE MANUFACTURER PRIOR TO THE CONSTRUCTION OF THE ABUTMENT. THE CONTRACTOR MUST ALSO VERIFY THE LOCATION, NUMBER AND SIZE OF THE ANCHOR BOLTS WITH THE BRIDGE MANUFACTURER PRIOR TO CONSTRUCTION.





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#### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

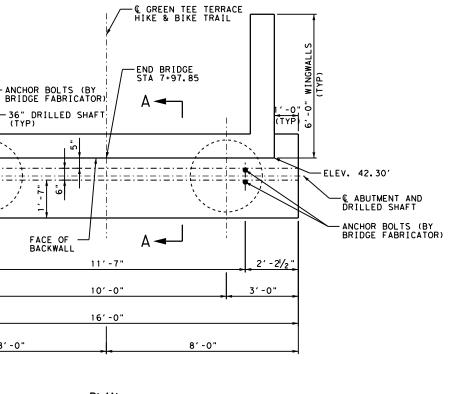
ABUTMENT 1

SHEET 1 OF 1

FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.	7
6				96	
STATE	DIST.		COUNTY		Π,
TEXAS	HOU	BR.	AZORIA / H	HARRIS	$\exists$
CONT.	SECT.	JOB	HIG	HWAY NO.	7
0912	31	291		VΔ	7

NOTES:

- . BRIDGE ABUTMENT DESIGNED ACCORDING TO LRFD BRIDGE DESIGN SPECIFICATION 7TH EDITION AND GUIDE SPECIFICATION FOR DESIGN OF PEDESTRIAN BRIDGES BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICALS (AASHTO 2015) 2. ALL CONCRETE STRENGTH f'c=3600 psi.
- 3. REINFORCING SHALL BE GRADE 60.
- SEE FD STANDARD FOR FOUNDATION DETAILS AND NOTES.
- 5. SEE BRIDGE LAYOUT FOR DRILLED SHAFT LENGTHS.
- . SEE ABUTMENT DETAILS SHEET FOR ADDITIONAL INFORMATION.
- CONTRACTOR MUST VERIFY ALL ELEVATIONS AND DIMENSIONS OF PREFABRICATED SPANS UPON DELIVERY.
- ALL PARTS ASSOCIATED WITH THE ANCHORAGE OF THE PREFABRICATED BRIDGE TO THE PROPOSED ABUTMENT ARE TO BE DESIGNED BY THE BRIDGE MANUFACTURER PRIOR TO THE CONSTRUCTION OF THE ABUTMENT. THE CONTRACTOR MUST ALSO VERIFY THE LOCATION, NUMBER AND SIZE OF THE ANCHOR BOLTS WITH THE BRIDGE MANUFACTURER PRIOR TO CONSTRUCTION.



#### <u>PLAN</u>

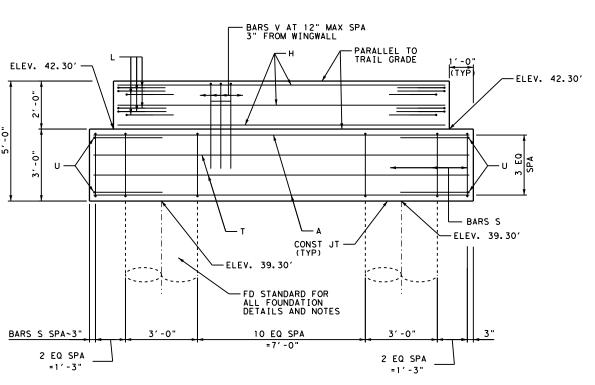
1'-0"

8'-0"

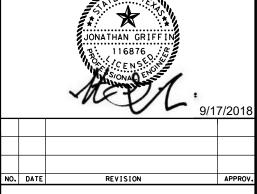
ELEV. 42.30' -

ANCHOR BOLT SPACING

DRILLED SHAFT



ELEVATION





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#### **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

ABUTMENT 3

SH	FF	т	1	ΩF	

SHEET	i OF i					
FED. RD. DIV. NO.		PROJECT NO.		SHEET NO.		
6				97		
STATE	DIST.		COUNTY			
TEXAS	HOU	BR.	AZORIA / H	HARRIS		
CONT.	SECT.	JOB	HIGHWAY NO.			
0912	31	291		VA		

3'-6"

SECTION A-A

BACKWALL

BARS V

ANCHOR

BOLTS

3' - 11/2 "

BARS S

6'-0" 7 EQ SPA PARALLEL TO ROADWAY SLOPE BOLTS

WINGWALL ELEVATION

(2) SILICONE SEALANT

2'-0"

BARS wU

BARS L

2" END COVER BARS A & U

<u>CAP</u>

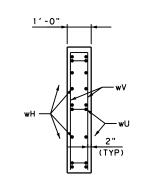
3'-0"

BARS U

CORNER DETAILS

1 BACKER ROD

ABUTMENT



SECTION B-B

-PREFORMED BITUMINOUS FIBER MATERIAL

PREFABRICATED BRIDGE

-TOOL 1/4" R

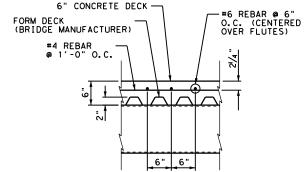
EXPANSION JOINT DETAIL 3

\* FOR CONTRACTOR'S INFORMATION ONLY QUANTITIES

# TABLE OF ESTIMATED QUANTITIES

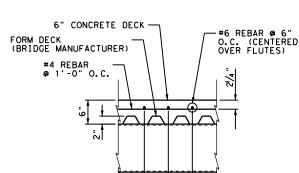
BAR	NO	SIZE	LENGTH	WEIGHT		
Α	8	#11	15' - 8"	666		
Н	6	#5	13' - 8"	86		
L	12	#5	4' - 0"	50		
S	17	#4	12' - 4"	140		
Т	4	#5	15' - 8"	65		
U	4	#6	8' - 0"	48		
٧	13	#5	7′ - 4"	99		
wH1	12	#6	5′ - 8"	102		
wH2	12	#6	8' - 2"	147		
wU	12	#4	1' - 7"	13		
₩∨	32	#5	4′ - 8"	156		
REINFORCIN	IG STEEL		LBS	1,572		
CLASS "C"	CONCRETE	(ABUT)	CY	9.1		

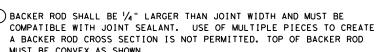
SHOWN ARE FOR ONE ABUTMENT ONLY.



TYP SLAB REINFORCEMENT DETAIL F'c=4,000 PSI (MINIMUM 28 DAY STRENGTH) GRADE 60 REINFORCING (Fy=60,000 PSI)

- 1) BACKER ROD SHALL BE 1/4" LARGER THAN JOINT WIDTH AND MUST BE A BACKER ROD CROSS SECTION IS NOT PERMITTED. TOP OF BACKER ROD MUST BE CONVEX AS SHOWN.
- 2 CLASS 7 SILICONE SEALANT THAT CONFORMS TO DMS-6310. INSTALL WHEN AMBIENT TEMPERATURE IS BETWEEN 55°F AND 85°F AND RISING. ENGINEER TO DETERMINE ALLOWABLE HOURS FOR SEALANT APPLICATION.
- (3) SEE BRIDGE LAYOUT FOR EXPANSION JOINT LOCATIONS.
- (4) SEE CONTECH DESIGN DATA SHEETS FOR JOINT WIDTH.





# Texas Department of Transportation

NO. DATE

# **GREEN TEE TERRACE BIKE & PEDESTRIAN TRAIL**

REVISION

1160 Dairy Ashford, Suite 500

USInfrastructure@rpsgroup.com

Houston, Texas 77079 T 281 589 7257

Formerly Klotz Associates, Inc. Texas PE Firm Reg. #F-929

9/17/2018

APPRO

BRIDGE ABUTMENT DESIGNED ACCORDING TO

LRFD BRIDGE DESIGN SPECIFICATION 7TH EDITION AND GUIDE SPECIFICATION FOR DESIGN OF PEDESTRIAN BRIDGES BY THE

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICALS (AASHTO 2015)

SEE FD STANDARD FOR FOUNDATION DETAILS

CONTRACTOR MUST VERIFY ALL ELEVATIONS

AND DIMENSIONS OF PREFABRICATED SPANS

ALL PARTS ASSOCIATED WITH THE ANCHORAGE OF THE PREFABRICATED BRIDGE TO THE PROPOSED ABUTMENT ARE TO BE DESIGNED BY THE BRIDGE MANUFACTURER PRIOR TO THE CONSTRUCTION OF THE ABUTMENT. THE

CONTRACTOR MUST ALSO VERIFY THE LOCATION,

NUMBER AND SIZE OF THE ANCHOR BOLTS WITH THE BRIDGE MANUFACTURER PRIOR TO

CONSTRUCTION. ANCHOR BOLTS SHALL CONFORM

TO TXDOT SPECIFICATION 449.

ALL CONCRETE STRENGTH f'c=3600 psi.

. SEE BRIDGE LAYOUT FOR DRILLED SHAFT

REINFORCING SHALL BE GRADE 60.

AND NOTES.

UPON DELIVERY.

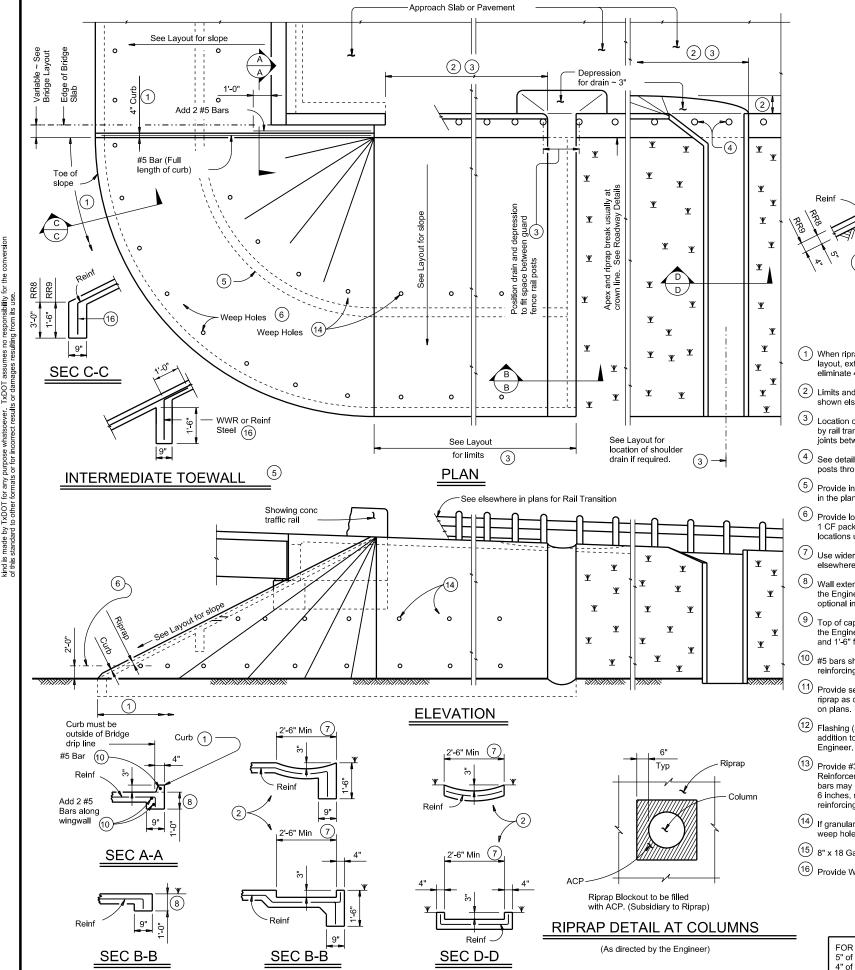
LENGTHS.

ABUTMENT DETAILS

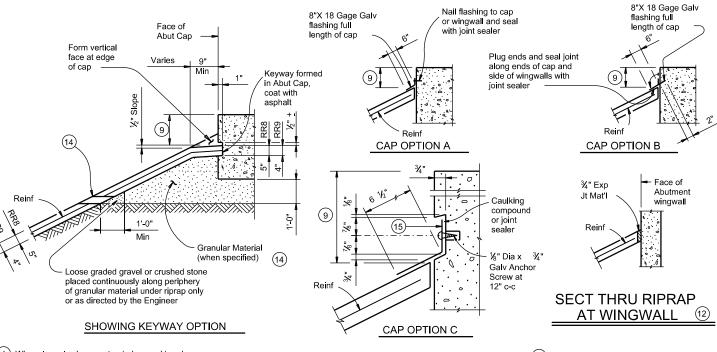
SHEET 1 OF 1

FED. RD. DIV. NO.		PROJECT NO.	SHEET NO.	7.7				
6				98	ċ			
STATE	DIST.		COUNTY		_ 0			
TEXAS	HOU	BR.	BRAZORIA / HARRIS					
CONT.	SECT.	JOB	HIGHWAY NO.					
0912	31	291	VA					

HIO LOADING



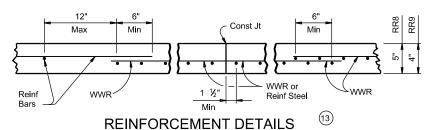
(Shoulder Drain)



(1) When riprap is shown extended around header on layout, extend slab and toewall as shown and

- 2 Limits and configuration of drains and depressions are as shown elsewhere in plans or as directed by the Engineer.
- Location of shoulder drain must consider limitations imposed by rail transition. Do not locate shoulder drains at expansion joints between approach slab and concrete paveme
- (4) See details elsewhere in plans for installation of guard fence posts through concrete riprap.
- 5 Provide intermediate toewall only when designated elsewhere in the plans or included in the specifications.
- 6 Provide lower level of 2" Dia weep holes at 10' c-c backed by 1 CF packet of gravel and galvanized hardware cloth at all locations unless directed by the Engineer to eliminate.
- Use wider or other drain configurations if shown elsewhere in plans or if directed by the Engineer.
- Wall extension may be reduced or modified if approved by the Engineer. Increase wall extension to 1'-6" whenever the optional intermediate toewall is called for in the plans.
- Top of cap to top of riprap dimension varies as directed by the Engineer. Should be 9" Min for beam/slab type bridges and 1'-6" for slab span, box beam, or slab beam bridges.
- 10 #5 bars shown are required even when synthetic fiber reinforcing option is selected.
- Provide sealing option for joint between the face of cap and riprap as designated by the Engineer or as shown elsewhere
- 12 Flashing (shown in Cap Option A) may be used at wingwall in addition to Exp Jt Mat'l if shown on plans or directed by the
- Provide #3 reinforcing bars at 18" Spa c-c. Provide Welded Wire Reinforcement (WWR) as 6x6-D3xD3. Combinations of WWR and reinforcing bars may be used if both are permitted. Use lap splices of a minimum 6 inches, measured from the transverse wire of WWR, and the ends of
- 14 If granular material is specified, provide upper level of 2" Dia weep holes at 10' c-c backed by galvanized hardware cloth.
- 8" x 18 Gage Galv Sheet Metal
- Provide WWR or #3 bars, with 1'-0" extension into slope.





See General Notes for optional sythetic fiber reinforcement

GENERAL NOTES:
Provide Class "B" concrete with a minimum compressive strength of 2,000 psi unless noted elsewhere in plans.

Provide Grade 60 reinforcing steel.

Provide synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Install construction joints or grooved joints extending the full slant slope height at intervals of approximately 20 feet unless otherwise directed by the Engineer.

Hardware cloth, loose grade stone behind weep holes, flashing, or other sealing material are subsidiary to the bid item "Riprap".

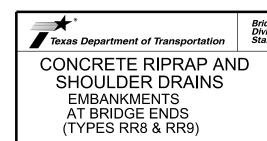
Use reinforcing bars, deformed Welded Wire Reinforcing (WWR), or any suitable combination of both types for riprap reinforcing, unless

specified elsewhere in the plans.

See Layout for limits of riprap.

RR8 is to be used on stream crossings.

RR9 is to be used on other embankments.



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<b>©</b> TXDOT	January 2015	CONT	SECT	JOB		HIGHWAY	
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		DIST	COUNTY SHEE			SHEET NO.	
		НОП					00

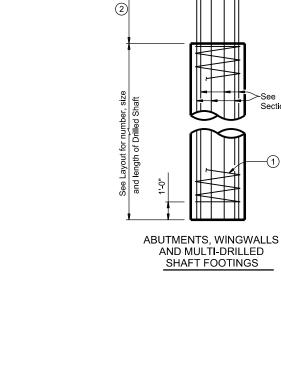
CRR

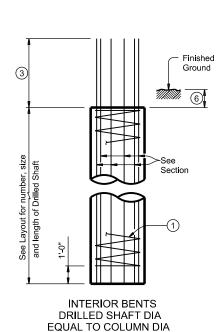
FOR CONTRACTOR'S INFORMATION ONLY: 5" of RR8 = 0.015 CY/SF 4" of RR9 = 0.012 CY/SE #3 Reinf at 18" c-c = 0.501 Lbs/SF 6x6-D3xD3 = 0.408 Lbs/SF

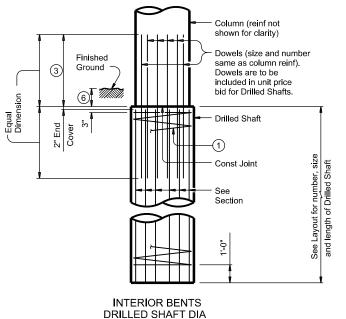
(No Drain)

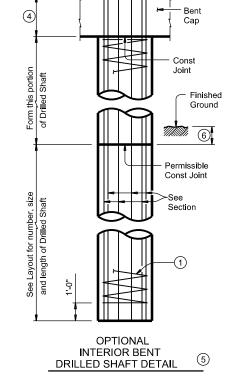
(Shoulder Drain

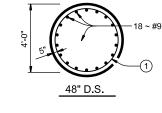
integral with riprap)

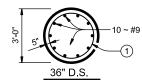












#### DRILLED SHAFT SECTIONS

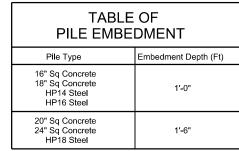
top & bottom).

 $\bigodot$  #4 Spiral at 9" pitch (One and a half flat turns

2 Min extension into supported element:

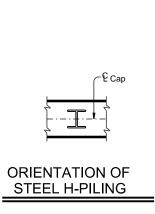
#### DRILLED SHAFT DETAILS

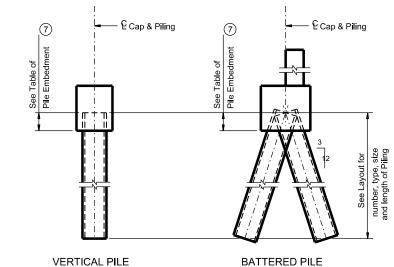
GREATER THAN COLUMN DIA

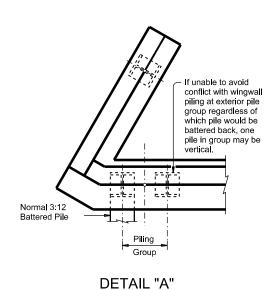


See standard CP for additional details on concrete pile embedment.

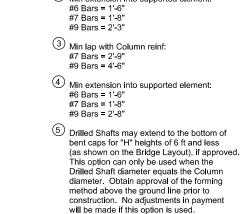
**ELEVATION** 







(Showing Plan View of a

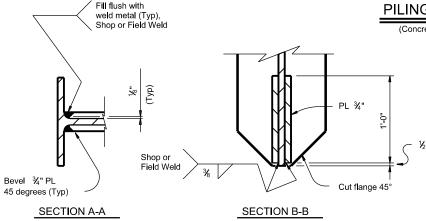


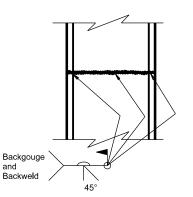
6 1'-0" Min

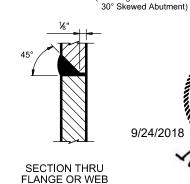
7 Or as shown on plans.

PILING DETAILS

(Concrete or Steel H)











SHEET 1 OF 2

# Texas Department of Transportation

## **COMMON FOUNDATION DETAILS**

FD(MOD)

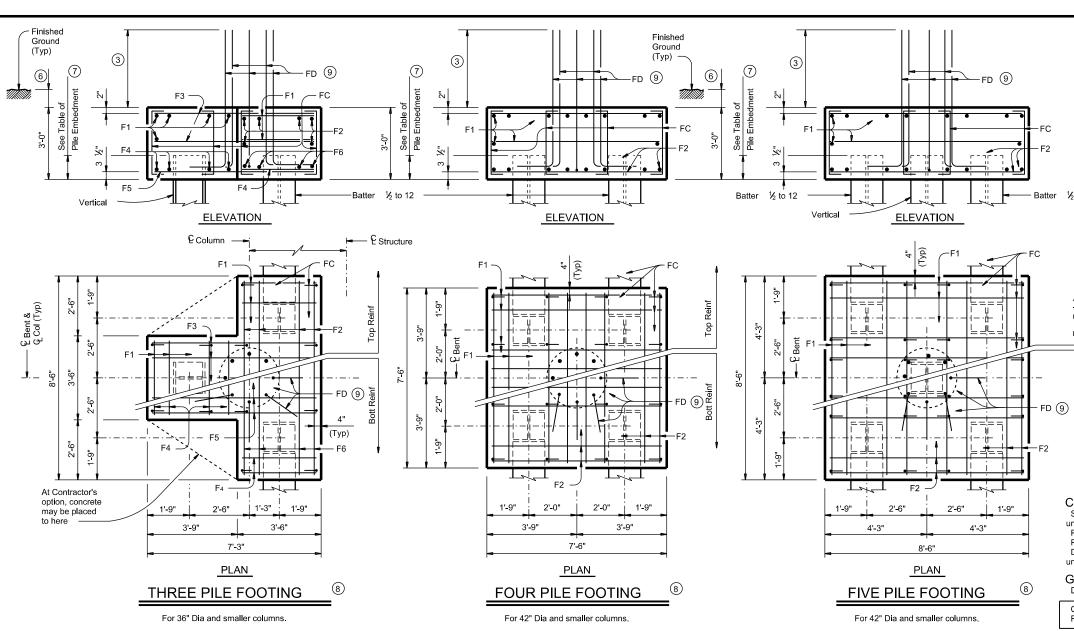
Bridge Division Standard

FILE:	fdstde01.dgn	DN: TXDOT CK: TXDOT DW: TXD		TxDOT	CH	c TxDOT		
<b>C</b> TXDOT	January 2015	CONT	SECT	JOB		HIGHWAY		'AY
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		DIST		COUNTY		SHE	EET NO.	
		HOU		BRAZORIA /	НΛΕ	PIG		100

STEEL H-PILE SPLICE DETAIL Use when required

See Item 407 "Steel Piling" to determine when tip reinforcement is required and for options to the details shown.

STEEL H-PILE TIP REINFORCEMENT



3 Min lap with Column reinf: #7 Bars = 2'-9" #9 Bars = 4'-6"

- 6 1'-0" Min
- 7 Or as shown on plans.
- 8 See Layout for Type, Size and length
- Number and size of FD bars must match Column reinforcing. Tie FD bars to the top of the bottom reinforcing mat.
- (0) For 24" Columns, use #7 FD bars (6'-6") in place of #9 bars and deduct 130 lbs. For 36" Columns, add 2 FD bars (59 lbs).
- 1) For 24" Columns, use #7 FD bars (6'-6") in place of #9 bars and deduct 130 lbs. For 36" Columns, add 2 FD bars (59 lbs). For 42" Columns, add 6 FD bars (177 lbs) (42" Columns disallowed on 3 Pile Footings)

#### TABLE OF FOOTING **QUANTITIES FOR** 30" COLUMNS

ONE 3 PILE FOOTING									
Bar	No.	Size	Length		Weight				
F1	11	#4	3'- 2"		23				
F2	6	#4	8'- 2"		33				
F3	6	#4	6'- 11"		28				
F4	8	#9	3'- 2"		86				
F5	4	#9	6'- 11"		94				
F6	4	#9	8'- 2"		111				
FC	12	#4	3'- 6"		28				
FD (10)	8	#9	8'- 8"		236				
Reinfo	rcing St	eel		Lb	639				
Class	"C" Con	crete		CY	4.8				
ONE 4 PILE FOOTING									
Bar	No.	Size	Length	Weight					
F1	20	#4	7'- 2"	96					
F2	16	#8	7'- 2"	306					
FC	16	#4	3'- 6"		37				
FD (11)	8	#9	8'- 8"		236				
Reinfo	rcing St	eel		Lb	675				
Class	"C" Con	crete		CY	6.3				
		ONE 5 P	ILE FOOTING	3					
Bar	No.	Size	Length	Weight					
F1	20	#4	8'- 2"		109				
F2	16	#9	8'- 2"	8'- 2"					
FC	24	#4	3'- 6"		3'- 6"		3'- 6"		56
FD (1)	8	#9	8'- 8"		236				
Reinfo	rcing St	Lb	845						

6"\_ BARS FC

1'-2" #7 Bars 1'-7" #9 Bars

BARS FD 9

#9 Bars

CONSTRUCTION NOTES:
See Bridge Layout for foundation type required. Use these foundation details

Provide Class C Concrete (fc = 3600 psi), unless shown otherwise.
Provide Grade 60 reinforcing steel.
Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile unless shown otherwise.

#### **GENERAL NOTES:**

Designed according to AASHTO LRFD Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

#### **DESIGNER NOTES:**

Do not use the Drilled Shaft details shown on this standard for retaining wall, noise wall, barrier or sign foundations without structural evaluation.

Do not use the footings shown on this standard in direct contact with salt water

or exposed to salt water spray. Maximum allowable pile loads for the footings shown are : 72 Tons/Pile with 24" Dia Columns

- 80 Tons/Pile with 30" Dia Columns
- 100 Tons/Pile with 36" Dia Columns
- 120 Tons/Pile with 42" Dia Columns

SHEET 2 OF 2



Bridge Division Standard

# **COMMON FOUNDATION DETAILS**

		FD						
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CTXDOT	January 2015	CONT	SECT	JOB		HIGHWAY		HWAY
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		DIST	COUNTY SHE			SHEET NO.		
		HOU	BRAZORIA / HARRIS					101