

**GENERAL DESIGN AND CODE INFORMATION:**

- A. THE CONSTRUCTION OF THIS STRUCTURE SHALL CONFORM WITH THE BUILDING CODE DEFINED AS 2009 INTERNATIONAL BUILDING CODE WITH LOCAL AMENDMENTS.
B. CONCRETE: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318, LATEST EDITION).
C. PRECAST CONCRETE: PCI MANUAL 116 FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF STRUCTURAL PRECAST CONCRETE PRODUCTS (LATEST EDITION).
D. STRUCTURAL STEEL: LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LATEST EDITION). AMERICAN INSTITUTE OF STEEL CONSTRUCTION (LATEST EDITION.)
E. OPEN WEB STEEL JOISTS: STANDARD SPECIFICATIONS, LOAD TABLES, AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, SJI (LATEST EDITION).
F. STEEL DECK: STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, ROOF DECKS, AND CELLULAR METAL FLOOR DECK WITH ELECTRICAL DISTRIBUTION (LATEST EDITION).
G. MASONRY: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530, LATEST EDITION) AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1, LATEST EDITION).
H. STRUCTURAL METAL STUDS: SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE (LATEST EDITION).
I. CONTRACTOR SHALL MAKE ALLOWANCE FOR SUPPLYING AND ERECTING FIVE TONS OF STRUCTURAL STEEL (INCLUDING THREE TONS OF STEEL BAR JOISTS) AND FOUR TONS OF REINFORCING STEEL OF VARIOUS SIZES TO BE USED IN PLACE AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

**DESIGN LOADS:**

- A. THE ROOF IS DESIGNED FOR A LIVE LOAD OF 20 PSF OR THE ROOF IS DESIGNED FOR SNOW LOADS IN ACCORDANCE WITH THE ABOVE NOTED CODE WITH DISTRIBUTION COEFFICIENTS APPLIED TO THE BASE LOAD AS REQUIRED. WHERE SNOW LOADS DO NOT GOVERN, MEMBERS ARE DESIGNED FOR A LIVE LOAD OF 20 PSF. THE FOLLOWING COEFFICIENTS WERE USED:
1. FLAT SNOW LOAD (PF).....5.5 PSF
2. SNOW EXPOSURE FACTOR (CE).....1.0
3. SNOW LOAD IMPORTANCE FACTOR (I).....1.1
4. THERMAL FACTOR (CT).....1.0

- B. DESIGN LOADS FOR THE FLOOR ARE INDICATED ON THE STRUCTURAL DRAWINGS.

- C. THE STRUCTURE WAS DESIGNED FOR THE FOLLOWING WIND LOADS:
1. ULTIMATE WIND SPEED.....90 MPH
2. RISK CATEGORY.....III
3. WIND EXPOSURE.....C
4. WIND IMPORTANCE FACTOR.....1.15

- D. THE STRUCTURE WAS DESIGNED FOR THE FOLLOWING SEISMIC LOADS:

- 1. OCCUPANCY CATEGORY.....III
SEISMIC IMPORTANCE FACTOR.....1.25
2. MAPPED SPECTRAL RESPONSE ACCELERATIONS
Ss.....0.132
S1.....0.053
3. SITE CLASS.....D
4. SDS.....0.141
SD1.....0.086
5. SEISMIC DESIGN CATEGORY.....B
6. BASIC SEISMIC FORCE RESISTING SYSTEM - BEARING WALL SYSTEM, ORDINARY REINFORCED CONCRETE WALLS
7. BASE SHEAR.....0.0441 x W KIPS
8. SEISMIC RESPONSE COEFFICIENT (Cs).....0.0441
9. RESPONSE MODIFICATION FACTOR (R).....4.0
10. ANALYSIS PROCEDURE.....EQUIVALENT LATERAL FORCE

- E. THE CONTRACTOR IS TO VERIFY ALL DIMENSIONS AND COORDINATE WITH ARCHITECTURAL DRAWINGS. IMMEDIATELY NOTIFY ARCHITECT OF ANY DISCREPANCIES.
F. SHOP DRAWINGS SHALL NOT BE REVIEWED FOR APPROVAL UNLESS CHECKED BY THE FABRICATOR AND APPROVED BY THE CONTRACTOR. SUBMIT A MAXIMUM OF FOUR SETS OF DRAWINGS FOR REVIEW. ONE SET WILL BE KEPT BY OUR OFFICE.
G. SHOP DRAWINGS SHALL NOT CONTAIN DETAILS COPIED OR REPRODUCED FROM THE CONTRACT DOCUMENTS. REPRODUCTION OF THE CONTRACT DOCUMENTS SHALL RESULT IN A REJECTION OF THE SHOP DRAWINGS.
H. RESTL IS NOT RESPONSIBLE FOR THE DESIGN AND DETAILING OF LOUVERS, SUNSHADES, GATES, RAILS AND OTHER NOT-STRUCTURAL ELEMENTS UNLESS SPECIFICALLY SHOWN IN THE STRUCTURAL CONTRACT DOCUMENTS.

**SPECIAL INSPECTION:**

- A. SPECIAL INSPECTIONS ARE REQUIRED PER THE ABOVE REFERENCED CODE FOR THE FOLLOWING PORTIONS OF CONSTRUCTION.
1. CONCRETE
2. BOLTS INSTALLED IN CONCRETE
3. REINFORCING STEEL
4. STRUCTURAL WELDING AND BOLTING
5. STRUCTURAL MASONRY

**FOUNDATION DESIGN INFORMATION:**

- A. THE FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS CONTAINED IN THE SOILS REPORT ENTITLED "GEOTECHNICAL INVESTIGATION" REPORT, DATED JANUARY 24, 2014 BY GEO SCIENCE ENGINEERING & TESTING, INC. PROJECT No. 14-066051
B. IT IS IMPERATIVE THAT THE CONTRACTOR OBTAIN, READ, AND UNDERSTAND THE REPORT. ALL RECOMMENDATIONS WITHIN THE REPORT SHOULD BE FOLLOWED.
C. ASSUMED SAFE SUBGRADE BEARING CAPACITIES LISTED BELOW SHALL BE CONFIRMED IN THE FIELD BY A REGISTERED GEOTECHNICAL ENGINEER. BORED CAST-IN SITU CAISSONS PIER WITH 24" DIAMETER NET ALLOWABLE BEARING CAPACITY = 25000 PSF SKIN FRICTION BEARING CAPACITY = 2500 PSF
D. WALLS RETAINING EARTH ARE DESIGNED FOR LATERAL EARTH PRESSURE OF 45 PCF. BACKFILL ALL WALLS WITH FREE DRAINING CRUSHED STONE. PROVIDE A DRAIN SYSTEM THAT IS PART OF THE STRUCTURE.
E. FOUNDATION WALLS WITHOUT CANTILEVERED FOOTINGS SHALL NOT BE BACKFILLED UNTIL SHORED OR PERMANENTLY SUPPORTED AT THE TOP OF THE WALL.
F. FOOTINGS ARE TO BEAR ON IMPROVED SOILS, FIRM UNDISTURBED EARTH, APPROVED CONTROLLED FILL, WEATHERED ROCK, OR SOUND ROCK PER GEOTECHNICAL REPORT RECOMMENDATIONS.
G. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL OR MATERIAL NOTED IN GEOTECHNICAL REPORT.
H. THE REGISTERED GEOTECHNICAL ENGINEER IN THE FIELD SHALL VERIFY THAT ALL SITE PREPARATION FILL OPERATIONS AND BEARING CONDITION COMPLY WITH THE SOILS REPORT.

**REINFORCED CONCRETE AND CONCRETE REINFORCEMENT:**

- A. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS (FC) SHALL BE AS FOLLOWS:
FOOTINGS.....4000 PSI
INTERIOR SLAB ON GRADE.....4000 PSI
SLABS ON METAL FORMS.....4000 PSI
TILT UP WALLS.....4000 PSI
PRECAST CONCRETE.....5000 PSI
ALL OTHER CONCRETE.....4000 PSI
B. CONCRETE EXPOSED TO FREEZING AND THAWING SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.50. ALL CONCRETE SUBJECTED TO DEICERS AND/OR REQUIRED TO BE WATERTIGHT SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45. PROVIDE ENTRAINED AIR AS PER ACI 318.
C. ALL PIPE PENETRATIONS THROUGH SLABS SHALL BE SLEEVED, UNLESS OTHERWISE NOTED.
D. ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL VIBRATORS.
E. NO REPAIR OR RUBBING OF THE CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND WITH THE APPROVAL OF THE ARCHITECT.
F. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60.
G. ALL LAP SPLICES FOR REINFORCING BARS SHALL BE "CLASS B", UNLESS NOTED OTHERWISE. LAP WWFA MINIMUM OF 6".
H. CONCRETE PLACEMENT SHALL NOT TAKE PLACE UNTIL INSPECTION BY AUTHORIZED AGENT OF THE ARCHITECT.
I. CLEAR COVER DISTANCE FOR REINFORCING STEEL SHALL BE PROVIDED AS FOLLOWS (UNLESS NOTED OTHERWISE).
FOOTINGS.....3" BOTTOM AND SIDES
GRADE BEAMS.....3" BOTTOM AND SIDES, 2" TOP WALLS.
PEDESTAL.....1 1/2"
SLABS.....3/4"

MAXIMUM DEVIATION IN BAR PLACEMENT SHALL BE PLUS OR MINUS 1/4" FOR SECTION 10" OR LESS AND 1/2" FOR SECTION OVER 10" THICK. SEE ACI 318 SECTION 2.7.1 FOR CONDITIONS NOT LISTED.

**CONCRETE MASONRY:**

- A. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY AT 28 DAYS AS DEFINED IN THE ACI 530/ASCE 6 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES SHALL BE 1m=1500 PSI.
B. MORTAR SHALL CONFORM TO THE FOLLOWING TYPES AS DEFINED IN THE ABOVE REFERENCED BUILDING CODE:
1. MASONRY IN CONTACT WITH EARTH: TYPE "M" MORTAR.
2. EXTERIOR BLOCK WALLS AND BEARING WALLS: TYPE "M" OR "S" MORTAR.
C. GROUT FOR FILLING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "MORTAR AND GROUT FOR REINFORCED MASONRY", ASTM C-476, AND SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. THE SLUMP SHALL BE BETWEEN 9" AND 11". WHERE THE MINIMUM DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3" OR LESS, USE FINE GROUT, OTHERWISE USE COURSE (PEA GRAVEL) GROUT.
D. MASONRY WALL CONTROL JOINTS SHALL BE LOCATED AS SHOWN ON THE ARCHITECTURAL DRAWINGS WITH A MAXIMUM SPACING OF 30'-0".
E. PROVIDE THE FOLLOWING MINIMUM CONTINUOUS HORIZONTAL MASONRY REINFORCING AT 16" C/C, UNLESS NOTED OTHERWISE: (USE REINFORCING MANUFACTURED BY DUR-O-WALL OR APPROVED EQUAL).

SINGLE WYTHE:
UNREINFORCED.....STANDARD WEIGHT TRUSS TYPE
REINFORCED: 8" WIDTH.....DUR-O-WALL SEISMIC LADDER TYPE
10" OR 12" WIDTH.....DUR-O-WALL SEISMIC LADDER TYPE

CAVITY WALLS:
UNREINFORCED.....STD. WEIGHT CONT. RECTANGULAR TAB TIE
REINFORCED: 8" WIDTH.....STD. WEIGHT CONT. RECTANGULAR TAB TIE
10" OR 12" WIDTH.....MEDIUM WEIGHT CONT. RECTANGULAR TAB TIE

- F. ALL VERTICAL REINFORCEMENT SHALL BE LOCATED IN THE CENTER OF THE WALL, UNLESS NOTED OTHERWISE.
G. ALL VERTICAL REINFORCEMENT SHALL BE LAPPED PER THE REINFORCING LAP SCHEDULE.
H. ALL OPENINGS IN CONCRETE MASONRY UNITS IN EXCESS OF 16" SHALL HAVE A BOND BEAM OR OTHER STRUCTURAL SUPPORT INSTALLED. SEE DETAILS. CUTTING OF OPENINGS LARGER THAN 16" AFTER MASONRY ERECTION MUST INCLUDE INSTALLATION OF SUPPORTING STRUCTURE. CONTACT STRUCTURAL ENGINEER FOR DESIGN.
I. PROVIDE BOND BEAMS WITH 2-#5 CONTINUOUS AT THE TOP OF FOUNDATION WALLS, TOPS OF PARAPETS, AT FLOOR LEVELS, AND WHERE SHOWN ON THE DRAWINGS UNLESS NOTED OTHERWISE.
J. PLACE REINFORCEMENT BARS BEFORE GROUTING. PLACE GROUT IN LIFTS NOT TO EXCEED FIVE FEET UNLESS HIGH LIFT GROUTING TECHNIQUES ARE SUBMITTED FOR RECORD. CONSOLIDATE EACH LIFT BY MECHANICAL VIBRATION. THE NEXT LIFT SHALL BE PLACED WHILE THE PRIOR LIFT IS STILL PLASTIC.

- K. REINFORCE MASONRY WALLS AS SHOWN ON SCHEDULE. DOWEL ALL MASONRY VERTICALS TO FOUNDATION.

**STRUCTURAL STEEL, STEEL STAIRS, AND METAL DECK/FORM:**

- A. STRUCTURAL STEEL
1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION.
2. ALL STRUCTURAL STEEL WIDE FLANGE MEMBERS SHALL BE ASTM A572 OR ASTM A992, GRADE 50. OTHER MISCELLANEOUS SHAPES SHALL BE ASTM-36, UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500, GRADE B, UNLESS NOTED OTHERWISE. CIRCULAR STRUCTURAL PIPING SHALL BE ASTM A53, GRADE B.
4. STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL BE ASTM A-325 BEARING TYPE CONNECTION, UNLESS NOTED OTHERWISE. BOLTS IN MOMENT CONNECTIONS, X-BRACES SKYLIGHT FRAMES, OR OTHER LOCATIONS NOTED SHALL BE INSTALLED WITH APPROVED DIRECT TENSION INDICATORS, OR BE APPROVED INDICATOR BOLTS. BOLTS IN TYPICAL SHEAR CONNECTIONS SHALL BE SNUG TIGHT ONLY. HORIZONTAL SLOTS CAN BE UTILIZED FOR GRAVITY CONNECTIONS ONLY.
5. ANCHOR BOLTS SHALL BE F1554 FORGED HEADED STUDS, Fy = 36 KSI (UNO).
6. FRAMED BEAM CONNECTIONS SHALL DEVELOP THE REACTION SHOWN FOR THE ENDS OF BEAMS ON STRUCTURAL PLANS. IN NO CASE SHALL THE LENGTH OF THE FRAMED CONNECTION BE LESS THAN 1.2 (OR 1.4 FOR COMPOSITE BEAMS) THE ALLOWABLE UNIFORM LOAD FOR LATERALLY SUPPORTED BEAMS AS SHOWN IN PART 2 OF THE AISC MANUAL.
7. WELDS SHOWN ON THE STRUCTURAL DRAWINGS ARE THE MINIMUM REQUIRED BY DESIGN. THE FABRICATORS DRAWINGS SHALL SHOW WELDS AND THEY SHALL CONFORM TO A W.S. SPECIFICATIONS. ALL WELDING SHALL BE DONE WITH E-70 SERIES ELECTRODES. MINIMUM WELD SIZE SHALL BE 3/16".
8. PAINT ALL STRUCTURAL STEEL WITH A HIGH GRADE RUST-INHIBITING PRIMER. USE RED OXIDE PRIMER UNLESS OTHERWISE NOTED. THE COMPATIBILITY OF PRIMER AND ANY TOP COAT SHALL BE VERIFIED BEFORE ANY PAINTING IS STARTED. TOUCH-UP ALL EXPOSED STEEL AFTER FIELD INSTALLATION. ALL STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO COATS OF EXTERIOR ENAMEL WHICH IS COMPATIBLE TO THE PRIMERED SURFACE. DO NOT PAINT STEEL TO RECEIVE SPRAYED-ON FIREPROOFING.
9. ALL STEEL EXPOSED TO WEATHER IN THE COMPLETE STRUCTURE SHALL BE GALVANIZED OR COATED WITH A ZINC RICH PRIMER.
10. DETAILS AND CONNECTIONS COMPLETELY DETAILED IN THE CONTRACT DOCUMENTS SHALL NOT BE ALTERED WITHOUT WRITTEN APPROVAL BY THE ENGINEER OF RECORD.
11. STEEL CONNECTIONS NOT SPECIFICALLY DETAILED IN THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS OF SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.

- B. METAL DECK/FORM
1. WELD WASHERS SHALL BE USED ON ALL METAL DECK UNITS WITH A METAL THICKNESS LESS THAN 0.028 INCHES. CONFORM TO THE METAL DECK MANUFACTURER'S RECOMMENDATIONS.
2. METAL FORM AND DECK SHALL BE CONTINUOUS OVER THREE (3) SPANS AND INSTALLED AS PER MANUFACTURER'S RECOMMENDATIONS. UNLESS NOTED OTHERWISE, ALL STEEL DECK DRAWINGS SHALL BE ACCOMPANIED BY A LETTER FROM AN ENGINEER REGISTERED IN THE STATE WHERE THE DECK IS TO BE INSTALLED, WHO SHALL STATE THAT THE DECK HAS BEEN DESIGNED AND FABRICATED IN ACCORDANCE WITH THE SDI SPECIFICATION.
C. STEEL STAIRS
1. RAILINGS, POSTS, AND CONNECTIONS SHALL BE CAPABLE OF RESISTING A HORIZONTAL LOADING OF 50 PLF OR 200 LBS. APPLIED AT TOP RAIL WITHOUT EXCEEDING ALLOWABLE STRESSES INCREASED BY ONE-THIRD. MAXIMUM SPACING OF 1 1/2" STD. STEEL PIPE POSTS SHALL BE 4'-0". DESIGN OF RAILS, POSTS, AND CONNECTIONS SHALL BE THE RESPONSIBILITY OF THE STAIR MANUFACTURER. SUBMIT SIGNED AND STAMPED CALCULATIONS WITH SHOP DRAWINGS.
2. STEEL STAIR WITH CONCRETE FILLED STEEL PAN OR PRECAST CONCRETE TREADS: SEE ARCHITECTURAL DRAWINGS FOR STAIR DIMENSIONS AND LOCATIONS. STAIR MANUFACTURER SHALL SUBMIT SIGNED AND STAMPED STRUCTURAL CALCULATIONS FOR STAIR MEMBERS AND CONNECTIONS WITH STAIR SHOP DRAWINGS FOR REVIEW. STAIR SHOP DRAWINGS SHALL INDICATE ALL CONNECTIONS TO BUILDING STRUCTURE, INCLUDING CONNECTORS EMBEDDED IN CONCRETE

**STEEL JOISTS AND GIRDERS:**

- A. ALL STEEL JOISTS AND GIRDERS SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE LATEST AISC-SJI SPECIFICATION. ALL STEEL JOISTS SHOP DRAWINGS SHALL BE ACCOMPANIED BY A LETTER FROM AN ENGINEER REGISTERED IN THE STATE WHERE THE JOISTS ARE TO BE ERECTED, WHO SHALL CERTIFY THAT THE JOISTS SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE AISC-SJI SPECIFICATIONS.
B. SUBMIT STAMPED CALCULATIONS FOR ALL JOISTS REQUIRED TO BE DESIGNED FOR SPECIAL LOADING CONDITIONS STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE JOISTS ARE ERECTED.
C. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO COORDINATE CONSTRUCTION AND ERECTION OF ALL MASONRY WALLS, STEEL JOISTS AND STEEL DECKING TO INSURE COMPATIBILITY OF ROOF SYSTEMS AND WALL SYSTEMS WHEN CONSIDERING PITCH AND CAMBER OF STEEL JOISTS.
D. NO ATTACHMENT SHALL BE MADE TO A STEEL JOIST THAT PRODUCES TORSION IN THE JOIST MEMBER.
E. CONTRACTOR SHALL VERIFY ALL LOCATIONS, PLAN DIMENSIONS, WEIGHTS, WEIGHT DISTRIBUTION, AND ROOF OPENING SIZE AND LOCATION FOR ALL MECHANICAL UNITS AND CONCENTRATED LOADS SUPPORTED BY STEEL JOISTS. TEEL JOISTS SHALL BE MODIFIED AND STRENGTHENED AS REQUIRED TO SAFELY SUPPORT THESE LOADS. PROVIDE TOP CHORD BRACING WHERE DECK IS OMITTED UNDER UNITS.

**LIGHT-GAUGE METAL FRAMING:**

- A. DESIGN CALCULATIONS FOR LIGHT-GAUGE METAL FRAMING SHALL BE SUBMITTED FOR APPROVAL TO THE ARCHITECT AND BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE FRAMING IS TO BE ERECTED.
B. LOAD BEARING METAL STUDS SHALL BE 18 GAUGE MINIMUM, TPE CSJ OR EQUAL. EXTERIOR, NON-LOAD BEARING STUDS SHALL BE 6" X 18 GAGE MINIMUM TYPE CSJ OR EQUAL. INTERIOR STUDS SHALL BE PER ARCHITECTURAL DRAWINGS.
C. VERTICAL STUDS ROLLING FRAMING MEMBERS SHALL BE 100% END BEARING.
D. PROVIDE COLD-ROLLED BRACING FOR ALL VERTICAL LOAD BEARING STUDS AT 48" ON CENTER MAXIMUM. PROVIDE SIMPSON STRONG-TIE TYPE TB BRIDGING AT 8'-0" ON CENTER MAXIMUM FOR ALL JOIST FLOOR FRAMING.
E. VERTICAL STUDS INTERRUPTED BY WALL OPENINGS SHALL BE LOCATED EQUALLY ON EACH SIDE OF THE OPENING. WELD STUD FLANGES TOGETHER WITH FILLET WELDS AT 6".
F. PROVIDE SHOP DRAWINGS SHOWING PLANS, ELEVATIONS, AND CONNECTION DETAILS FOR ALL LIGHT-GAUGE FRAMING.
G. ALL TRUSS TO TRUSS CONNECTION SHALL BE ENGINEERED BY THE TRUSS MANUFACTURER AND DETAILED ON THE TRUSS SHOP DRAWINGS.

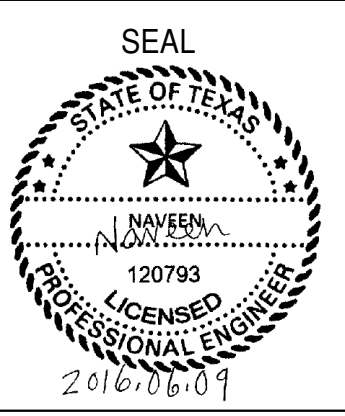
STRUCTURAL GENERAL GENERAL NOTES CONTINUED ON SHEET S.01A



RADHA KRISHNA TEMPLE
Owner
ALLEN TEXAS

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STRUCTURAL GENERAL NOTES