



RCAT 43rd Annual Event
OCTOBER 10 - 12, 2018
GAYLORD TEXAN RESORT &
CONFERENCE CENTER
Grapevine, Texas

Update on roofing industry technical issues

presented by

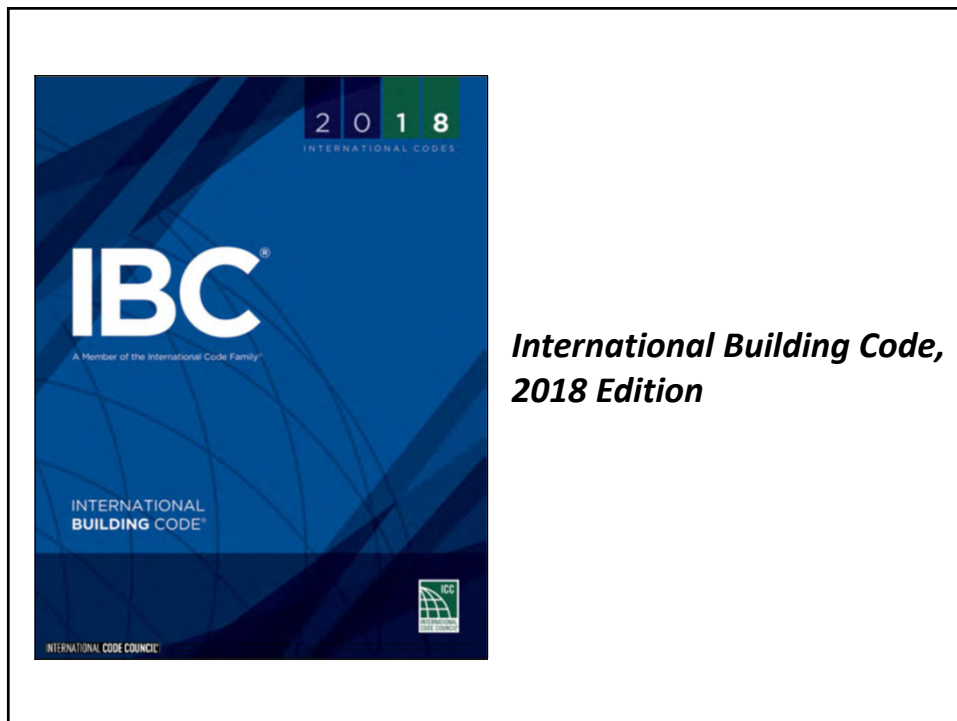
Mark S. Graham

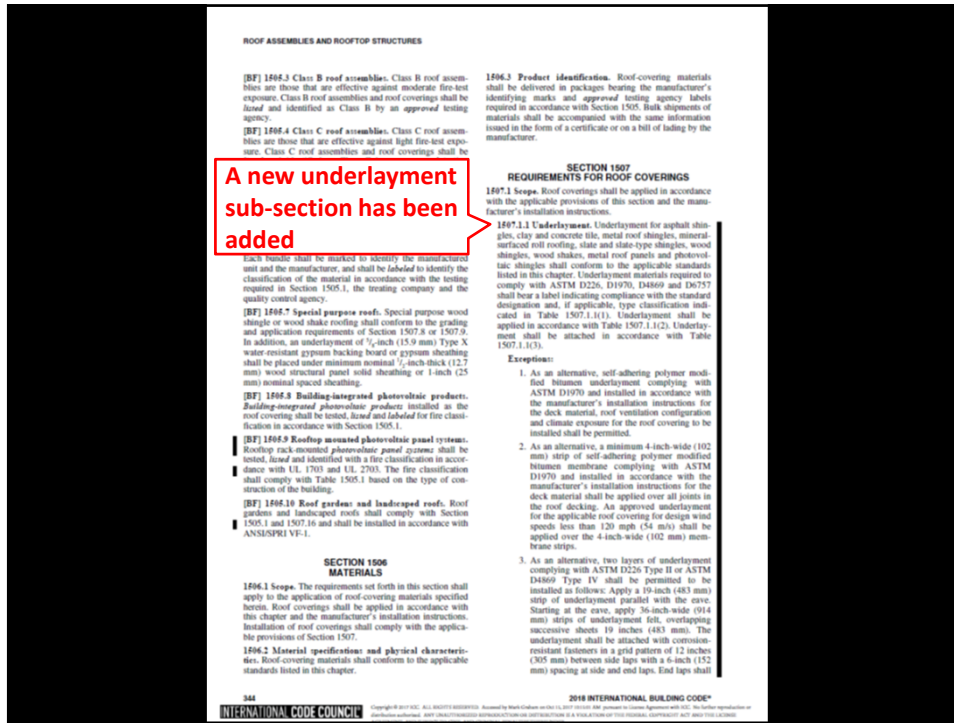
Vice President, Technical Services
National Roofing Contractors Association (NRCA)



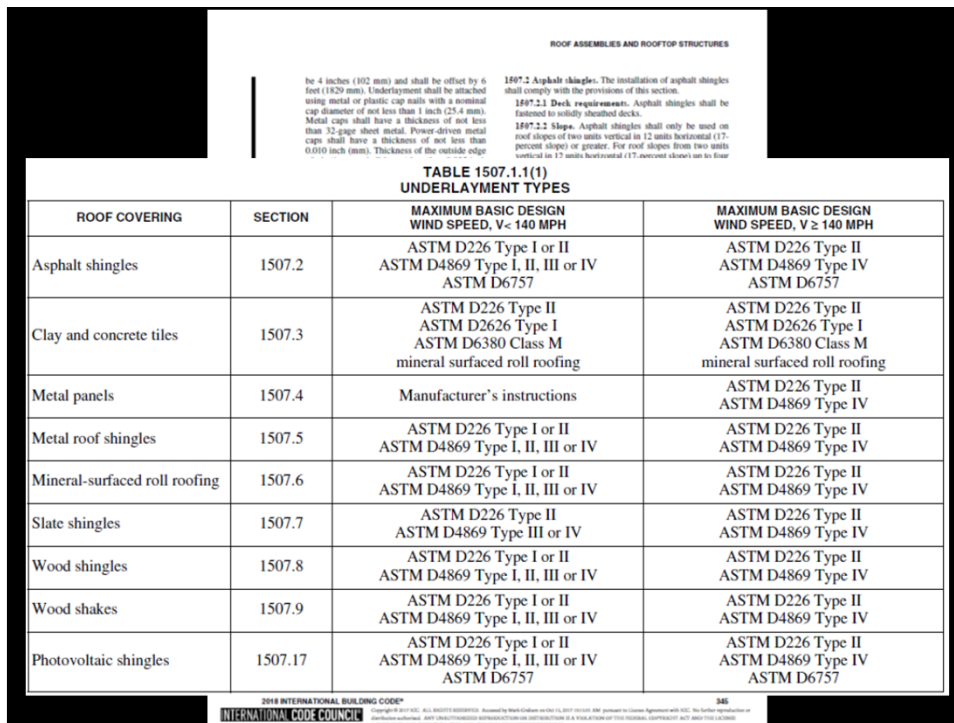
Topics

- 2018 I-code overview
- ASCE 7-16 (wind design)
- NFPA 70 (electrical code) changes
- Roof drain concerns
- Moisture in concrete roof decks
- Steel roof deck concerns
- FM VSH (hail)
- Attic ventilation
- Questions (and answers)





A new underlayment sub-section has been added

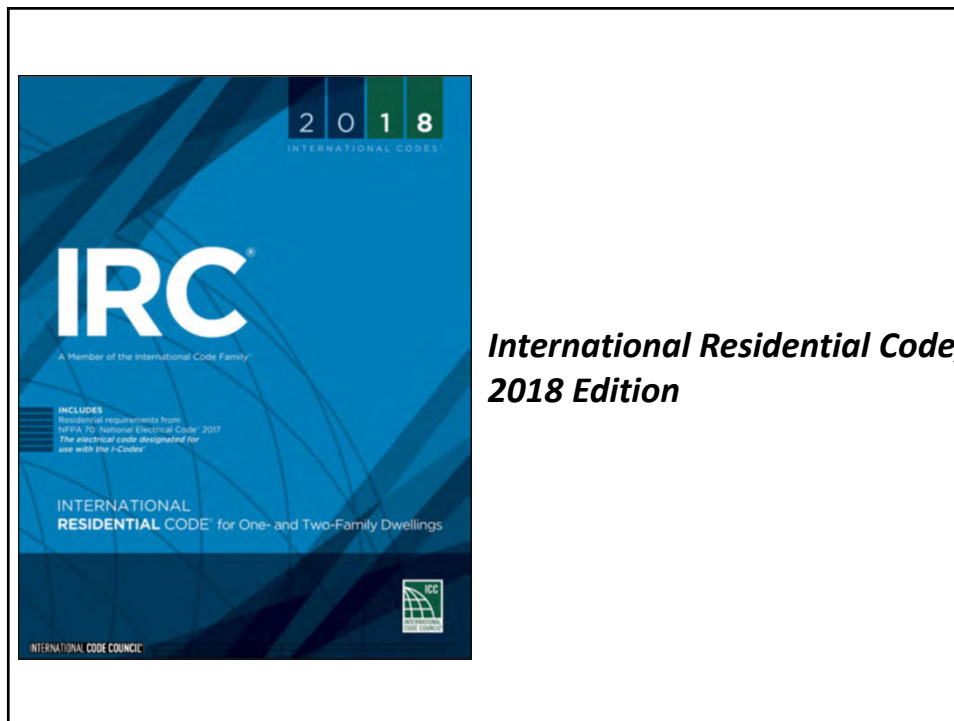
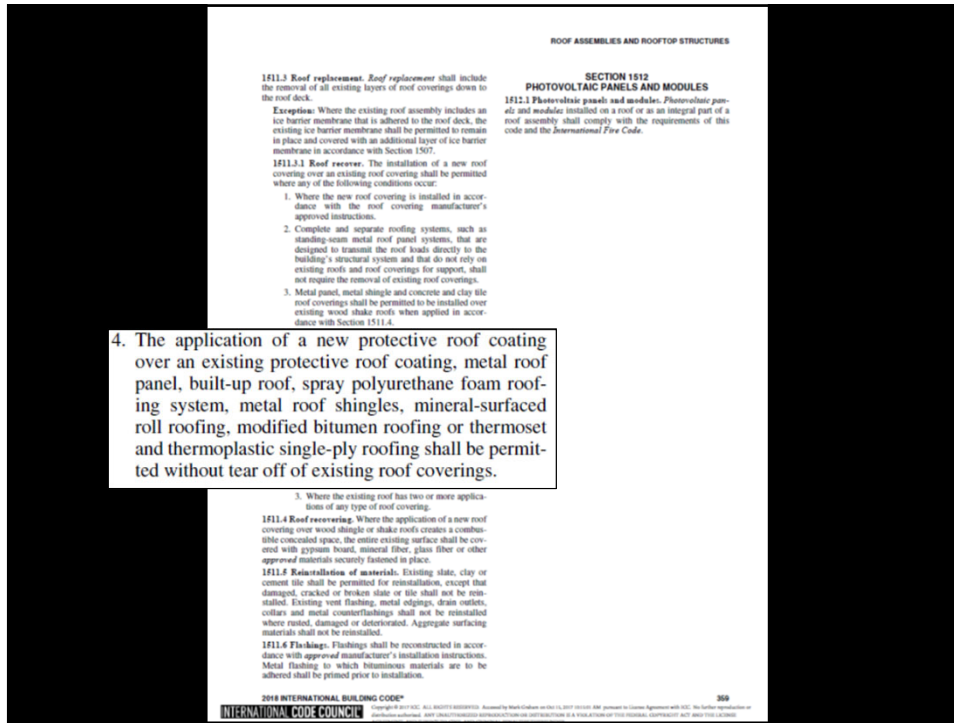


ROOF COVERING		SECTION	MAXIMUM BASIC DESIGN WIND SPEED, $V < 140$ MPH	MAXIMUM BASIC DESIGN WIND SPEED, $V \geq 140$ MPH
Asphalt shingles		1507.2	<p>For roof slopes from two units vertical in 12 units horizontal (2:12), up to four units vertical in 12 units horizontal (4:12), underlayment shall be two layers applied as follows: Apply a 19-inch strip of underlayment felt parallel to and starting at the eaves. Starting at the eave, apply 36-inch-wide sheets of underlayment, overlapping successive sheets 19 inches. End laps shall be 4 inches and shall be offset by 6 feet. Distortions in the underlayment shall not interfere with the ability of the shingles to seal.</p> <p>For roof slopes of four units vertical in 12 units horizontal (4:12) or greater, underlayment shall be one layer applied as follows: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches. Distortions in the underlayment shall not interfere with the ability of the shingles to seal. End laps shall be 4 inches and shall be offset by 6 feet.</p>	<p>Same as Maximum Basic Design Wind Speed, $V < 140$ mph except all laps shall be not less than 4 inches</p>

COVERING	SECTION	MAXIMUM BASIC DESIGN WIND SPEED, $V < 140$ MPH	MAXIMUM BASIC DESIGN WIND SPEED, $V \geq 140$ MPH
Slate shingles	1507.7		
Wood shakes	1507.8		
Wood shingles	1507.9		
Photovoltaic shingles	1507.17	<p>For roof slopes from three units vertical in 12 units horizontal (3:12), up to four units vertical in 12 units horizontal (4:12), underlayment shall be two layers applied as follows: Apply a 19-inch strip of underlayment felt parallel to and starting at the eaves. Starting at the eave, apply 36-inch-wide sheets of underlayment, overlapping successive sheets 19 inches. End laps shall be 4 inches and shall be offset by 6 feet. Distortions in the underlayment shall not interfere with the ability of the shingles to seal.</p> <p>For roof slopes of four units vertical in 12 units horizontal (4:12) or greater, underlayment shall be one layer applied as follows: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches. Distortions in the underlayment shall not interfere with the ability of the shingles to seal. End laps shall be 4 inches and shall be offset by 6 feet.</p>	<p>Same as Maximum Basic Design Wind Speed, $V < 140$ mph except all laps shall be not less than 4 inches</p>

For SL: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s.

ROOF ASSEMBLIES AND ROOFTOP STRUCTURES	
<p>wood complying with Section 2303.2 for exterior installation.</p> <p>3. Where exterior wall covering panels are used, the panels shall have a flame spread index of 25 or less when tested in the minimum and maximum thicknesses intended for use, with each face tested independently in accordance with ASTM E84 or UL 723. The panels shall be tested in the minimum and maximum thicknesses intended for use in accordance with, and shall comply with the acceptance criteria of, NFPA 285 and shall be installed as tested. Where the panels are tested as part of an exterior wall assembly in accordance with NFPA 285, the panels shall be installed on the face of the mechanical equipment screen supporting structure in the same manner as they were installed on the tested exterior wall assembly.</p> <p>[BG] 1510.6.3 Type V construction. The height of mechanical equipment screens located on the roof decks of buildings of Type V construction, as measured from grade plane to the highest point on the mechanical equipment screen, shall be permitted to exceed the maximum building height allowed for the building by other provisions of this code where complying with any one of the following limitations, provided that the fire separation distance is greater than 5 feet (1524 mm):</p> <ol style="list-style-type: none"> 1. Where the fire separation distance is not less than 20 feet (6096 mm), the height above grade plane of the mechanical equipment screen shall not exceed 4 feet (1219 mm) more than the maximum building height allowed. 2. The mechanical equipment screen shall be constructed of noncombustible materials. 3. The mechanical equipment screen shall be constructed of fire-retarded-treated wood complying with Section 2303.2 for exterior installation. 4. Where the fire separation distance is not less than 20 feet (6096 mm), the mechanical equipment screen shall be constructed of materials having a flame spread index of 25 or less when tested in the minimum and maximum thicknesses intended for use with each face tested independently in accordance with ASTM E84 or UL 723. <p>[BG] 1510.7 Photovoltaic panels and modules. Rooftop-mounted photovoltaic panels and modules shall be designed in accordance with this section.</p> <p>[BG] 1510.7.1 Fire classification. Rooftop-mounted photovoltaic panels and modules shall have the fire classification in accordance with Section 1505.9.</p> <p>[BG] 1510.7.2 Photovoltaic panels and modules. Rooftop-mounted photovoltaic panels and modules shall be listed and labeled in accordance with UL 1703 and shall be installed in accordance with the manufacturer's instructions.</p> <p>[BG] 1510.8 Other rooftop structures. Rooftop structures not regulated by Sections 1510.2 through 1510.7 shall comply with Sections 1510.8.1 through 1510.8.5, as applicable.</p>	<p>[BG] 1510.8.1 Aerial supports. Aerial supports shall be constructed of noncombustible materials.</p> <p>Exception: Aerial supports not greater than 12 feet (3658 mm) in height as measured from the roof deck to the highest point on the aerial supports shall be permitted to be constructed of combustible materials.</p> <p>[BG] 1510.8.2 Bulkheads. Bulkheads used for the shelter of mechanical or electrical equipment or vertical shaft openings in the roof assembly shall comply with Section 1510.2 as penoblements. Bulkheads used for any other purpose shall be considered as an additional story of the building.</p> <p>[BG] 1510.8.3 Dormers. Dormers shall be of the same type of construction as required for the roof in which such dormers are located or the exterior walls of the building.</p> <p>[BG] 1510.8.4 Fences. Fences and similar structures shall comply with Section 1510.6 as mechanical equipment screens.</p> <p>[BG] 1510.8.5 Flagpoles. Flagpoles and similar structures shall not be required to be constructed of noncombustible materials and shall not be limited in height or number.</p> <p>[BG] 1510.9 Structural fire resistance. The structural frame and roof construction supporting loads imposed upon the roof by any rooftop structure shall comply with the requirements of Table 601. The fire-resistance reduction permitted by Table 601, Note a, shall not apply to roofs containing rooftop structures.</p> <p>SECTION 1511 REROOFING</p> <p>1511.1 General. Materials and methods of application used for recovering or replacing an existing roof covering shall comply with the requirements of Chapter 15.</p> <p>Exceptions:</p> <ol style="list-style-type: none"> 1. Roof replacement or roof removal of existing low-slope roof coverings shall not be required to meet the minimum design slope requirement of one-quarter unit vertical in 12 units horizontal (2-percent slope) in Section 1507 for roofs that provide positive roof drainage. 2. Recovering or replacing an existing roof covering shall not be required to meet the requirement for secondary (emergency overflow) drains or scuppers in Section 1503.4 for roofs that provide for positive roof drainage. For the purposes of this exception, existing secondary drainage or scupper systems required in accordance with this code shall not be removed unless they are replaced by secondary drains or scuppers designed and installed in accordance with Section 1503.4. <p>1511.2 Structural and construction loads. Structural roof components shall be capable of supporting the roof-covering system and the material and equipment loads that will be encountered during installation of the system.</p>



CHAPTER 9
ROOF ASSEMBLIES

User note:

About this chapter: Chapter 9 addresses the design and construction of roof assemblies. A roof assembly includes the roof deck, substrate or thermal barrier, insulation, vapor retarder and roof covering. This chapter provides the requirement for wind resistance of roof coverings. The types of roof covering materials and installation addressed by Chapter 9 are asphalt shingles, clay and concrete tile, metal roof shingles, manufactured tile roofing, slate and stone-type shingles, wood shakes and shingles, built-up roofs, metal roof panels, modified bitumen roofing, thermoset and thermoplastic single-ply roofing, sprayed polyurethane foam roofing, liquid applied coatings and photovoltaic shingles. Chapter 9 also provides requirements for roof drainage, flashing, above-deck thermal insulation, rooftop-mounted photovoltaic systems and reworking or replacing an existing roof covering.

IRC 2018 Ch. 9 changes are similar to those of IBC 2018 Ch. 15 except:

- ASCE 7-10's wind maps apply
- Some rooftop PV reformatting:
 - New Sec. R324-Solar Energy Systems
- New Sec. R905.17 (BIPV applied directly to the roof deck)

Impregnation with chemicals by the full-cell vacuum-pressure process, in accordance with AWPA C1. Each bundle shall be marked to identify the manufacturer and the manufacturer, and shall be labeled to identify the classification of the material in accordance with the testing required in Section R902.1, the testing company and the quality control agency.

R902.3 Building-integrated photovoltaic product. Building-integrated photovoltaic products installed as the roof covering shall be tested, listed and labeled for fire classification in accordance with Section R902.1.

R902.4 Rooftop-mounted photovoltaic panel systems. Rooftop-mounted photovoltaic panel systems installed on or

R903.2.2 Crickets and saddles. A cricket or saddle shall be installed on the ridge side of any chimney or penetration more than 30 inches (762 mm) wide as measured perpendicular to the slope. Cricket or saddle coverings shall be sheet metal or of the same material as the roof covering.

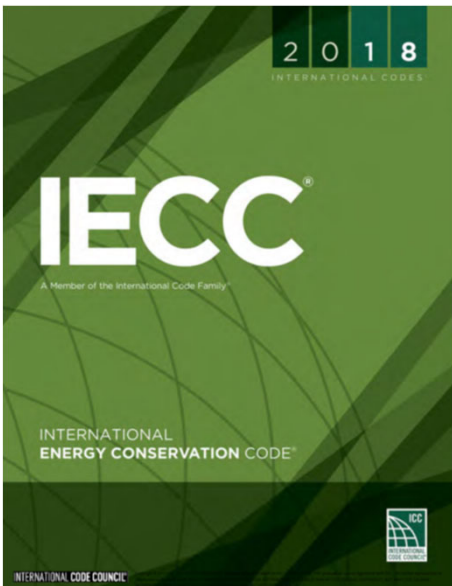
Exception: Unit skylights installed in accordance with Section R308.6 and flashed in accordance with the manufacturer's instructions shall be permitted to be installed without a cricket or saddle.

R903.3 Coping. Parapet walls shall be properly coped with noncombustible, weatherproof materials of a width not less than the thickness of the parapet wall.

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2018 INTERNATIONAL RESIDENTIAL CODE®

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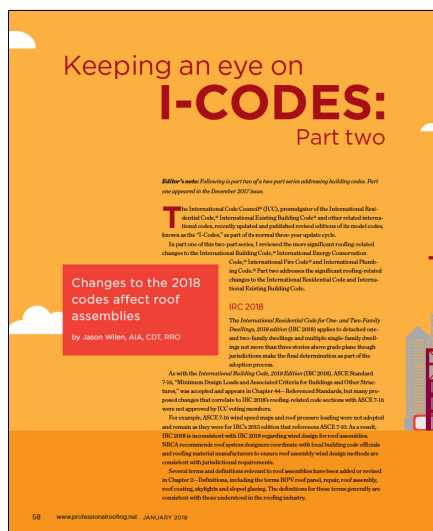
International Energy Conservation Code, 2018 Edition

IECC 2018's roofing-related requirements

- No substantive changes from IECC 2015
 - R-value
 - Roof reflectivity and emissivity
 - Air barriers
- ASHRAE 90.1-16 alternative
 - ASHRAE 90.1-12 referenced in IECC 2015

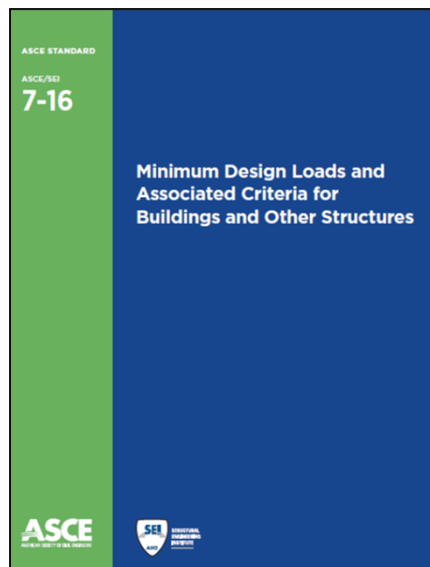


Professional Roofing, December 2017
[Link to access this article](#)



Professional Roofing, January 2018
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ASCE 7-16
Design wind uplift



American Society of Civil Engineers Standard 7, “Minimum Design Loads and Associated Criteria for Buildings and Other Structures” (ASCE 7-16)

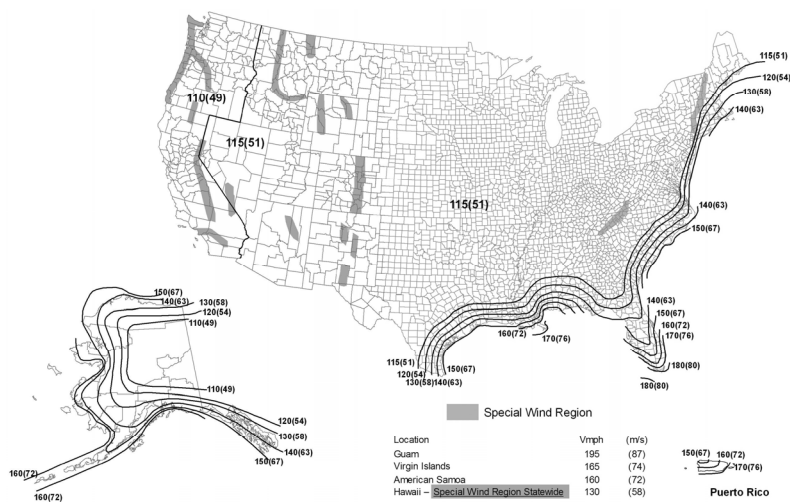
Noteworthy changes in ASCE 7-16

Compared to ASCE 7-10

- Revised basic wind speed map
- Changes (and new) pressure coefficients
- Revised perimeter and corner zones

ASCE 7-10 basic wind speed map

Fig. 1607A-- V_{ult} for Risk Category II Buildings



ASCE 7-16 basic wind speed map

Risk Category II Buildings (MRI = 700 years)



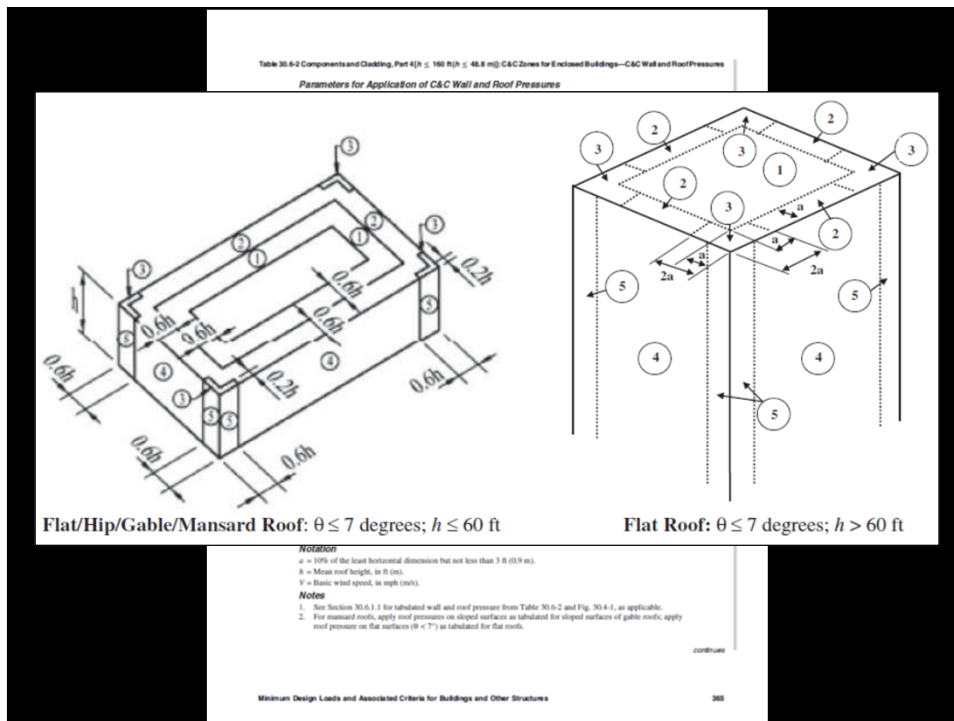
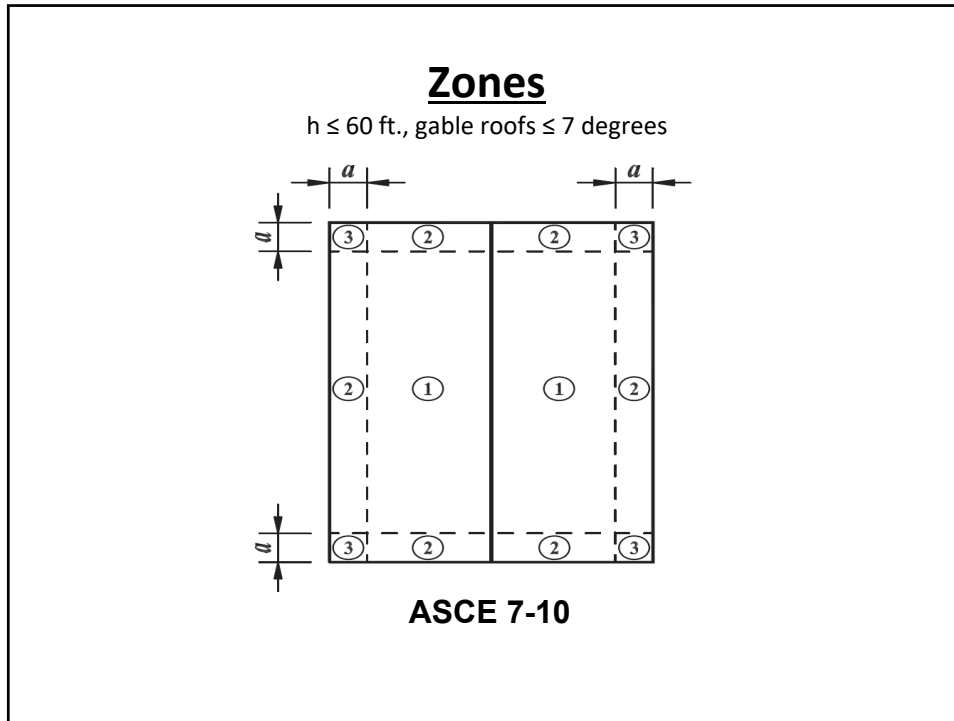
MRI		
Risk Category	ASCE 7-10	ASCE 7-16
I (Low)	300 yrs.	300 yrs.
II (not I, II or IV)	700 yrs.	700 yrs.
Category III (High risk)	1,700 yrs.	1,700 yrs.
Category IV (Essential)	1,700 yrs.	3,000 yrs.

Selection of the correct Risk Category/map (i.e., wind speed) is essential

Comparing GC_p pressure coefficients

h ≤ 60 ft., gable roofs ≤ 7 degrees

Zone	ASCE 7-10	ASCE 7-16	Change
1'	n/a	0.9	-10%
1 (field)	-1.0	-1.7	+70%
2 (perimeter)	-1.8	-2.3	+28%
3 (corners)	-2.8	-3.2	+14%



Noteworthy changes in ASCE 7-16

Compared to ASCE 7-10

- Revised basic wind speed map
- Changes (and new) pressure coefficients
- Revised perimeter and corner zones

While center field pressures may be slightly lower, field, perimeter and corner uplift pressures will generally be greater

How the roofing industry will adapt to ASCE 7-16 remains to be seen....

FM Global has indicated they will update their FM 1-28 to be based on ASCE 7-16 (with modifications) in mid-2019.

Comparing ASCE 7-05, ASCE 7-10 and ASCE 7-16

Example: A office building (Risk Category II) is located in Dallas, Texas. The building is an enclosed structure with a mean roof height of 40 ft. The building is located in an open terrain area that can be categorized as Exposure Category C. An adhered, membrane roof systems is to be installed.

Document	Basic wind speed (mph)	Design wind pressure (psf)			
		Zone 1' (Center)	Zone 1 (Field)	Zone 2 (Perimeter)	Zone 3 (Corners)
ASCE 7-05	90	--	21.8	36.4	54.8
ASCE 7-10 Ult.	115	--	35.5	59.5	89.5
ASCE 7-10 ASD	89	--	21.3	35.7	53.4
ASCE 7-16 Ult.	110	29.7	51.7	68.1	92.8
ASCE 7-16 ASD	85	17.8	31.8	40.9	55.7

This comparison illustrates why it is important for Designers to include wind design loads in their Construction Documents (per IBC Sec. 1603.1)...

...It also illustrates why specifying a wind warrantee can create an uneven playing field. Unless the Designer indicates the wind design loads, which design method will the manufacturer use (e.g., in a competitive environment)?


TECH TODAY

Specifying wind design

Many roof system designers inadequately address wind loads in contract documents
by Mark S. Graham

NRCA is receiving an increasing number of requests indicating proper drawings and specifications incompletely, inadequately or inaccurately address proper wind design for low-slope residential roof systems. Some designers, according to requests, only include a specification requirement for the roof system manufacturer to provide a wind warranty.

But there are minimum requirements for proper wind design of low-slope residential roof systems.

Code requirements

Building codes typically provide specific requirements for specifying design loads, including wind loads, in contract documents.

The International Building Code, 2012 Edition (IBC, 2012), Chapter 16-Structural Design, Section 1603-Contract Documents, indicates contract documents need to include a roof system low wind zone load data, wind design data and any special loads.

Required wind design data includes identifying the ultimate design wind speed, nominal design wind speed, risk category, wind exposure and applicable annual pressure coefficients. For component and cladding systems that are not specifically designed by a registered design professional, design wind pressure in terms of psf (pounds per square foot) also are required. Roof systems typically are considered component and cladding systems. Design wind pressure in the field, perimeter and corner regions

of roof areas should be noted in contract documents.

IBC's previous editions include similar contract document requirements.

For new construction projects, design loads most accurately will be identified on structural drawings in the project drawing set. For projects without specific structural drawings, design loads may be provided on architectural drawings or drawing notes or in project specifications.

ANSI/SPRI ES-1

ANSI/SPRI ES-1, "Wind Design Standard for Edge Systems Used with Low-Slope Roofing Systems," which is referenced in IBC, 2012, includes two primary document determinations of design wind loads at roof edges (finite, support and testing for minimum loads of coping and fascia).

Designers should not simply specify compliance with ANSI/SPRI ES-1 in project specifications; they should determine and clearly include design wind loads at roof edges in contract documents.

IBC 2012 indicates in Section 1905.5-Edge System for Low-Slope Roofs design wind loads should be determined using the ultimate design wind speed and IBC, 2012's Chapter 16, which is based on ASCE 7-05, "Minimum Design Loads for Buildings and Other Structures."

IBC 2012 references ANSI/SPRI ES-1-03, ANSI/SPRI ES-1-03 is based upon ASCE 7-02, which is not an ultimate design wind speed based method. Therefore, the design wind load determination method contained in ANSI/SPRI ES-1 does not verify IBC, 2012's requirements for design wind loads at roof edges.

Design wind loads at roof edges should be

16 www.professionaldrafting.net MARCH 2014

[Link](#)

Professional Roofing
March 2014



roofwinddesigner.com

ASCE 7-05, ASCE 7-10 and ASCE 7-16

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Roof Wind Designer is intended to provide users with an easy-to-use means for determining roof systems' design wind loads for many commonly encountered building types that are subject to building code compliance.

Roof Wind Designer has been updated based upon ASCE 7-16:

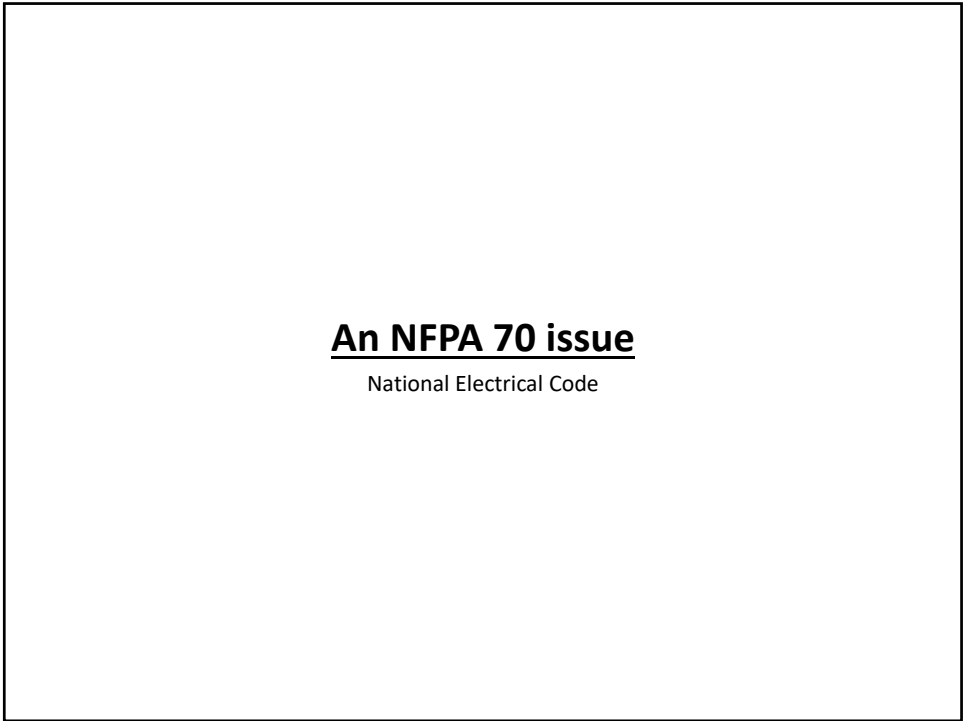
- Part 2: Low-rise Buildings (Simplified) [h ≤ 60 ft.]
- Part 4: Buildings with 60 ft. < h ≤ 160 ft. (Simplified)*

* Does not include hip and gable roofs h > 60 ft. and all roof slopes over 7 degrees (about 1.5:12)

To register for a new account [click here](#). If you already have an account, [click here](#) to login.

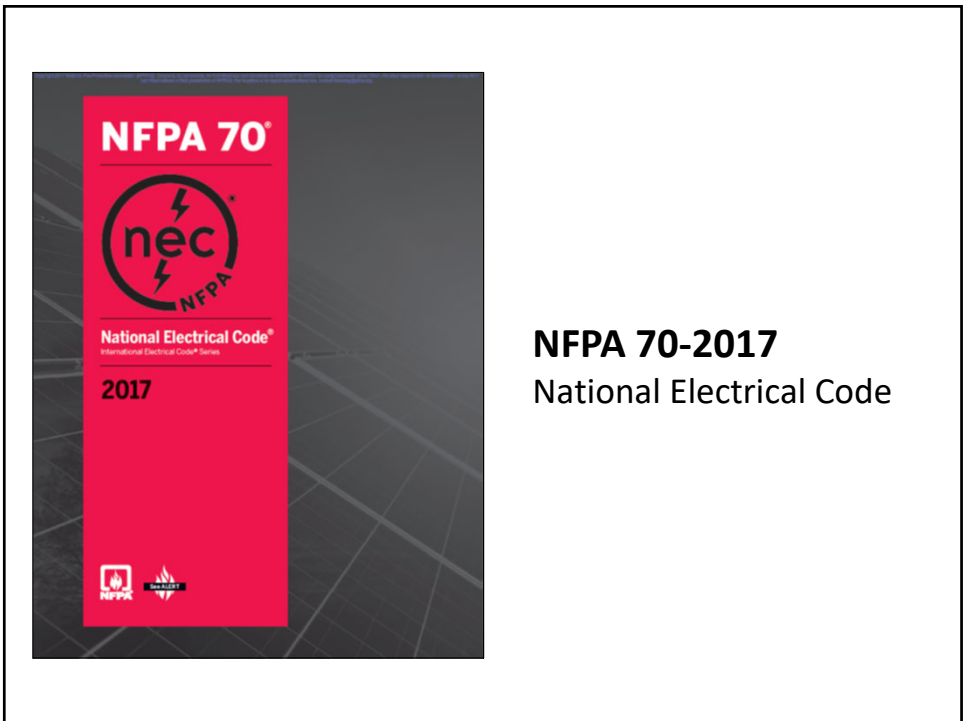


National Roofing Contractors Association

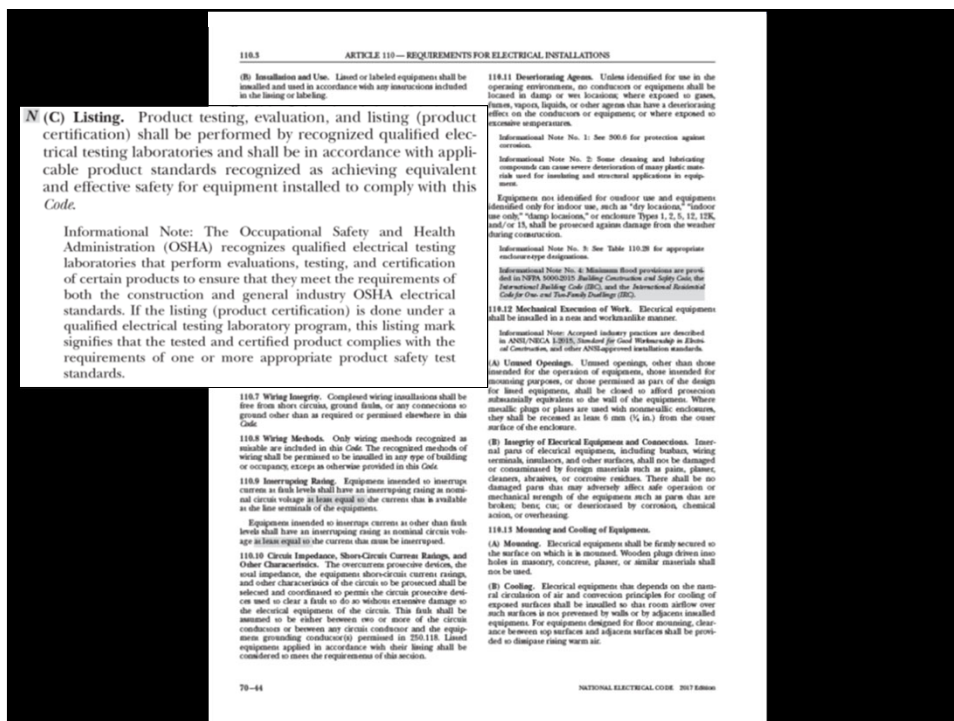
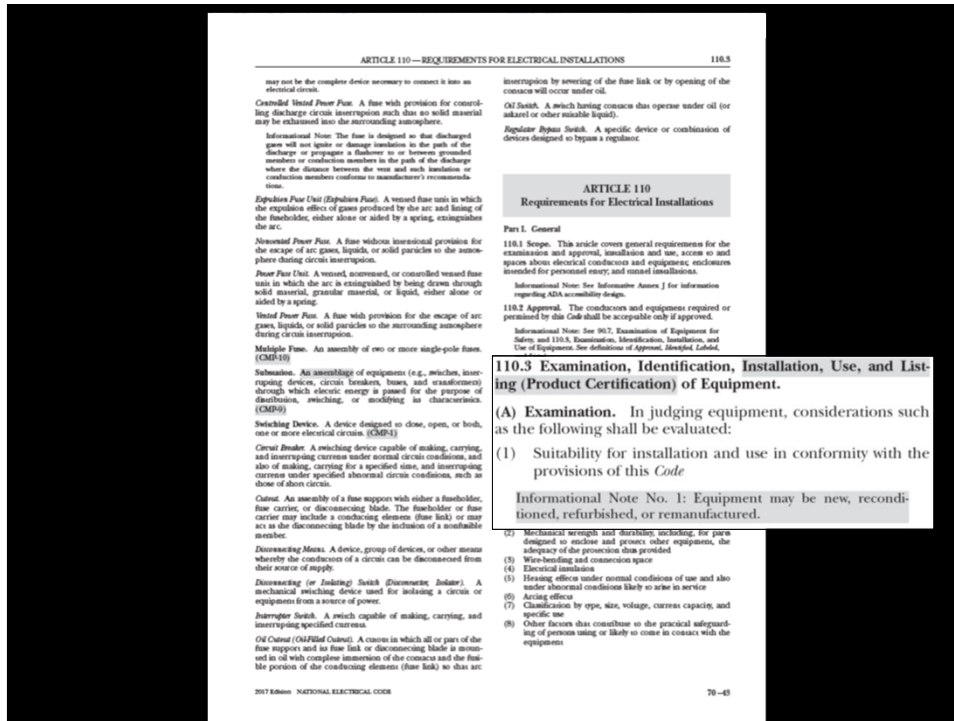


An NFPA 70 issue

National Electrical Code



NFPA 70-2017
National Electrical Code



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NRLRC News

OSHA Issues memorandum outlining enforcement of silica standard

Contract provision to provide roofing contractor with opportunity to appeal dispute resolution decision issued by "decision maker" designated by contract

[More news]

Contract provision obligating manufacturer and seller of equipment to roofing contractor to furnish equipment that is code-compliant

When purchasing a new piece of equipment, roofing contractors should beware of signing a seller's or manufacturer's standard purchase order agreement or agreeing to a seller's or manufacturer's standard terms and conditions. This agreement could include an express disclaimer of Uniform Commercial Code warranties of merchantability and fitness and will seek to limit the liability of the seller and the remedies available to the buyer in the event of a defect or problem with the product. Prior to making a purchase, the roofing contractor should obtain written assurance the equipment or product the contractor is purchasing complies with all codes, standards and regulations applicable to that equipment or product and its installation. Roofing contractors should be certain to include a provision to that effect in the purchase agreement.

For example, if your roofing company is in the market to purchase a sheet metal folding machine, it's important the sales agreement contain a provision such as the one above that obligates the seller to furnish a machine that will comply with all applicable codes and standards pertaining to the machine in the locality where you intend to install the machine. Such a provision is especially critical considering the 2017 edition of the NFPA 70, National Electrical Code (NEC), which jurisdictions could adopt as of Jan. 1. Article 110 of the 2017 NEC contains a new provision that has been interpreted as requiring all electrical equipment installed or used in a building undergo product testing, evaluation and listing (product certification) by a recognized qualified electrical testing laboratory in accordance with applicable product safety standards recognized by the NEC. If your business is in one of the states, cities, counties or towns throughout the U.S. that has adopted the 2017 edition of the NEC, in the absence of proof your new sheet metal folding machine complies with the NEC, code officials may not permit the newly purchased machine to be used. To

Equipment and product purchase agreement: The Seller and Manufacturer warrant to the Roofing Contractor that the equipment and product manufactured by Manufacturer and sold by Seller to Roofing Contractor will comply with all codes, standards and regulations applicable to the equipment and product in the jurisdiction where the equipment and product are delivered and intended for use, including the applicable electrical code and OSHA standards. No disclaimer or limitation of warranties of merchantability or fitness or other warranties by Seller or Manufacturer and no term or condition in the sales agreement shall cause or be interpreted to void, disclaim or reduce the obligation of the Seller and Manufacturer to furnish equipment and products that are in compliance with applicable codes, standards and regulations.

7/31/2018 [Link](#)

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Field Evaluations of Electrical Equipment

Are you struggling with the local authority? Has the local electrical inspector, or other Authority Having Jurisdiction (AHJ) "red-tagged" your equipment without a mark of compliance to electrical safety standards? Intertek's experts can be onsite within 24 hours in response to a red-tag event. What's more, our experts are on hand to answer your questions.

- Need an Intertek expert on site within 24 hours? Call 1-800 WORLD LAB and ask for Field Labeling help.
- Need advice on how to handle a red-tag event? Call 1-800 WORLD LAB and ask for a Field Labeling expert.
- Want to learn about the fastest Field Labeling team in North America? [Download a free copy of our fact sheet.](#)

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Roof drain concerns

Roof drainage

SECTION 1502 ROOF DRAINAGE

[P] 1502.1 General. Design and installation of roof drainage systems shall comply with Section 1502 of this code and Sections 1106 and 1108, as applicable, of the *International Plumbing Code*.

[P] 1502.2 Secondary (emergency overflow) drains or scuppers. Where roof drains are required, secondary (emergency overflow) roof drains or scuppers shall be provided where the roof perimeter construction extends above the roof in such a manner that water will be entrapped if the primary drains allow buildup for any reason. The installation and sizing of secondary emergency overflow drains, leaders and conductors shall comply with Sections 1106 and 1108, as applicable, of the *International Plumbing Code*.

1502.3 Scuppers. Where scuppers are used for secondary (emergency overflow) roof drainage, the quantity, size, location and inlet elevation of the scuppers shall be sized to prevent the depth of ponding water from exceeding that for which the roof was designed as determined by Section 1611.1. Scuppers shall not have an opening dimension of less than 4 inches (102 mm). The flow through the primary system shall not be considered when locating and sizing scuppers.

1502.4 Gutters. Gutters and leaders placed on the outside of buildings, other than Group R-3, private garages and buildings of Type V construction, shall be of noncombustible material or not less than Schedule 40 plastic pipe.

CHAPTER 11 STORM DRAINAGE

Where scuppers, scupper roofs, buildings must be removed and drained to a location that can accommodate storm water. Chapter 11 specifies the design method used for the geographic area and provides sizing methods for piping and public systems to convey the storm water away from the building. Included in this chapter are regulations for piping materials and related drainage systems.

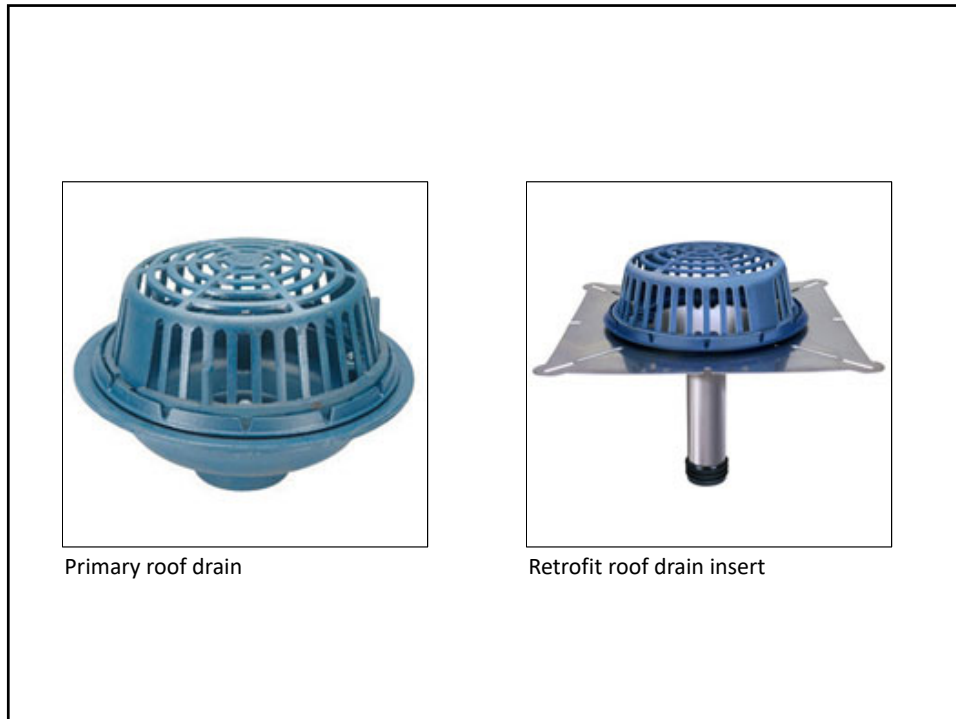
SECTION 1105 ROOF DRAINS

1105.1 General. Roof drains shall be installed in accordance with the manufacturer's instructions. The inside opening for the roof drain shall not be obstructed by the roofing membrane material.

1105.2 Roof drain flow rate. The published roof drain flow rate, based on the head of water above the roof drain, shall be used to size the storm drainage system in accordance with Section 1106. The flow rate used for sizing the storm drainage piping shall be based on the maximum anticipated ponding at the roof drain.

SECTION 1106 SIZE OF CONDUCTORS, LEADERS AND STORM DRAINS

1106.1 General. The size of the vertical conductors and leaders, building *storm drains*, building *storm sewers* and any horizontal branches of such drains or *sewers* shall be based on the 100-year hourly rainfall rate indicated in Figure 1106.1 or on other rainfall rates determined from approved local weather data.



Primary roof drain

Retrofit roof drain insert

NRCA recommendations

Roof drainage concerns

- Be cautious of roof drain issues, particularly in reroofing situations
 - IBC 2009 adds secondary drainage
 - IBC 2015 provides exception
 - IPC 2015 and IPC 2018 changes
- Assure membrane opening is larger than drain outlet/piping opening
- Be cautious of retrofit drain inserts
- Consider proposal/contract language

Moisture in concrete roof decks


Concrete Floors and Moisture, 2nd Edition

Howard M. Kanare, CTL Group

75% internal RH can be achieved:

- Normal weight structural concrete
 - Less than 90 days
- Lightweight structural concrete
 - Almost 6 months

NRCA Industry Issue Update, August 2013



INDUSTRY ISSUE UPDATE

NRCA Member Benefit

Moisture in Lightweight Structural Concrete Roof Decks

Concrete Moisture Presents Challenges for Roofing Contractors

NRCA's Technical Services Section is receiving an increasing number of inquiries relating to the application of roof systems over concrete roof decks. These inquiries can be separated into two general questions: When is a concrete roof deck dry enough to apply a roof covering? And why is a roof system applied over a concrete roof deck showing signs of moisture infiltration when the roof covering isn't leaking?

CONCRETE BASICS
There are three general types of concrete: normal-weight structural concrete, lightweight structural concrete and lightweight insulating concrete.

Normal-weight structural concrete is what most people think of as concrete; it has a density of about 150 pounds per cubic foot (pcf). Lightweight structural concrete has structural load-carrying capabilities similar to normal-weight structural concrete; it has a density in the range of 85 to 120 pcf. Lightweight insulating concrete, which many roofing professionals are familiar with as an insulating, slope-to-drain deck topping, typically has a density in the range from 20 to 40 pcf.

Structural concrete—normal-weight structural concrete and lightweight structural concrete—is produced by mixing large and small aggregates, Portland cement, water and, in some instances, admixtures such as fly ash or various chemical additives. Admixtures can add entrained air to the concrete, accelerate concrete's setting, retain concrete's excess moisture and/or lengthen concrete's finishing time. Use of admixtures typically is not visually identifiable in the field; microscopic analysis usually is needed for post-application identification of admixtures.

The primary difference in the composition of normal-weight structural concrete and lightweight structural concrete is the large aggregate type. Normal-weight structural concrete contains normal-weight aggregates such as stone or crushed gravel, which are dense and typically will absorb no more moisture than about 2 percent by weight. Lightweight structural concrete uses lightweight,

porous aggregates such as expanded shale, which will absorb about 5 to 25 percent moisture by weight. Lightweight aggregate needs to be saturated with moisture—it often soaks in ponds—before mixing. As a result, lightweight structural concrete inherently contains much more water than normal-weight structural concrete.

Lightweight structural concrete is used in roofing-related applications for cast-in-place concrete roof decks using removable form composite roof decks where a metal form deck remains in place and as a deck topping material, such as a concrete topping surface over precast concrete planks or slabs.

Once poured, lightweight structural concrete typically cannot be easily distinguished from normal-weight structural concrete.

Visual identification is possible using magnification, typically a microscope used by a trained technician.

REPORTED PROBLEMS
The problems reported to NRCA associated with lightweight structural concrete roof decks include the following:

- **Moisture accumulation.** Excessive moisture from a concrete deck can be pressure-differential driven into and condensed within a roof system.
- **Adhesive del.** The presence of moisture can result in deterioration of moisture-sensitive roofing materials and adhesive bond loss between adhered material layers.
- **Adhesive resin softening and delamination.** Excessive moisture can affect adhesive curing and drying rate. Also, moisture can result in adhesive "rewetting," resulting in bond strength loss.
- **Metal fastener corrosion.** Excessive moisture can contribute to and accelerate metal component corrosion, including fastener corrosion.
- **Insulation R-value del.** The accumulation and presence of moisture in most insulation products will result in reduced thermal performance (lower effective R-value).
- **Microbial growth.** The presence of prolonged high-moisture

[Link](#)

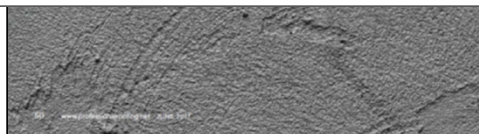
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June 2017



ASTM E96 calculated perm					
		Lightweight structural concrete		Normal weight concrete	
Age	Wet cup	Dry cup	Wet cup	Dry cup	
28 days	1.48	0.78	3.42	1.05	
60 days	1.45	0.47	2.03	1.13	

The figure shows results of ASTM E96 water vapor transmission testing. Note the lightweight structural concrete has about half of the permeability of regular weight concrete. Considering lightweight structural concrete arrives with more than twice the evaporable water of regular weight concrete, this explains why lightweight structural concrete retains moisture for so long.



[Link](#)

Moisture on concrete roof decks



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Sept. 2017

[Link](#)

Moisture vapor reduction admixtures (MVRAs)

Some examples:

- Barrier One
- ISE Logik MVRA 9000
- SPG VaporLock

NRCA has still not seen an MVRA perform successfully in concrete roof deck applications

*The roofing industry needs to re-think
the concept of concrete roof deck “acceptance”*

Steel roof deck concerns

Steel roof deck design

- SDI Design Manual
- AISI S100, “Specifications for the Design of Cold-formed Steel structural Members”
- ANSI/SDI RD1.0-2006, “Standard for Steel Roof Deck”
- ANSI/SDI RD-2010, “Standard for Steel Roof Deck”
- *SDI Roof Deck Design Manual, First Edition* (Nov. 2012)


Steel roof deck design

Wind uplift resistance

- Minimum 30 psf uplift (uniform loading)
- Minimum 45 psf uplift (uniform loading) at roof overhangs

SDI bulletin

2009



STEEL DECK INSTITUTE
Position Statement

ATTACHMENT OF ROOFING MEMBRANES TO STEEL DECK

This document has been published by the Steel Deck Institute (SDI) as a position paper in response to discussions taking place in the roofing community about the screw attachment of roofing membranes to steel deck following line patterns with large spacing. The impetus for this paper is in response to testing carried out by the Special Interest Group for Dynamic Evaluation of Roofing Systems (SDIGERS) at the Institute for Research in Construction, National Research Council of Canada. The mandate of the SDIGERS joint research program is to carry out generic, joint competitive research on the performance of flat roofing systems subjected to dynamic wind loading. The objective is to develop improved roofing systems and design methods.

The SDIGERS research is looking at roofing systems that incorporate wide membrane sheets attached to the steel deck following line patterns spaced at up to 12 ft (3.66 m). While the membrane itself has the performance characteristics to accommodate this size of tributary loading, the existing design methods for steel deck under wind uplift are typically based on the uniform application of the wind suction to the deck. The large majority of the steel roof deck used for commercial buildings in North America is profiled with 1 1/4" (31.8 mm) flutes, with the structural supports usually spaced between 5' 0" (1.52 m) and 6' 0" (1.83 m). Under uplift conditions, the attachment of the roofing membrane along lines with large spacing could produce localized loads that can exceed the capacity of the deck, whereas those same loads applied uniformly on the surface of the deck would be acceptable.

The strength of screwed connection between the membrane and the steel deck, as well as the strength of screwed, nailed, or welded attachment of the steel deck to the structural supports can be computed according to the North American Specification for the Design of Cold-Formed Steel Structural Members. These design values are based on the specified minimum mechanical properties (i.e. base steel thickness and yield strength) specified for the steel sheet roof deck, and should be lower than the strength determined by field testing. The use of field test results for properties such as the pull-out strength of a screw into a steel deck needs to recognize that the properties of the steel deck can be higher than the minimum limits required by the steel specifications. Therefore, field testing results must be adjusted accordingly to account for the difference between the actual properties of the deck and the minimum properties of the steel according to the material specification used in design.

The screw fastening of wide roofing membranes (up to 12 ft) and the corresponding spacing of the lines of screws holding the membrane on the deck, will have a very different effect on the deck and structural supports than a membrane that is adhered over its entire surface. The screws will produce a line load along the deck instead of a uniform load of the entire deck surface. The line loads can be perpendicular or parallel to the deck flutes depending on the orientation of the membrane each condition can have different implications of the loading that is applied to the deck.

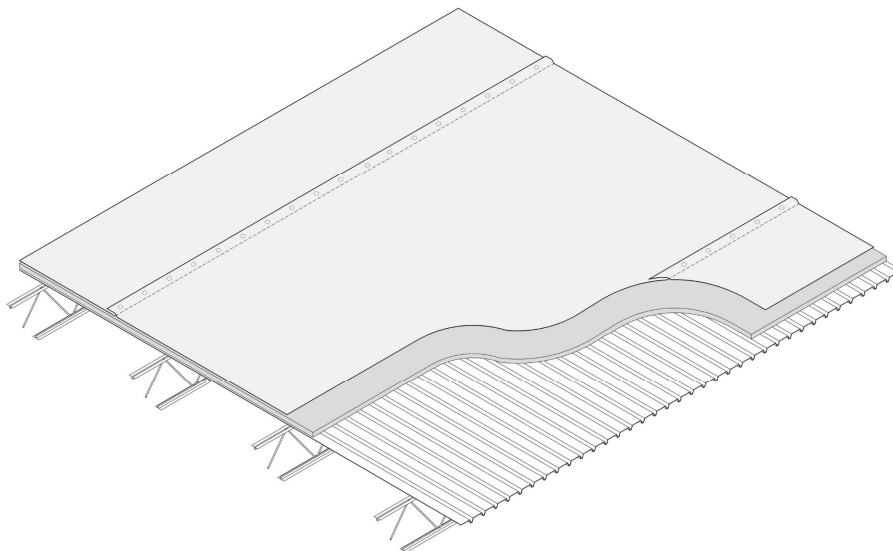
If the roofing membrane seam is perpendicular to the flutes of the deck, as illustrated in Figure 1, there are two special conditions that need to be considered.

1. If the membrane seam occurs at the mid-span of the steel deck; and
2. If the membrane seam occurs at the structural support (joist).

1

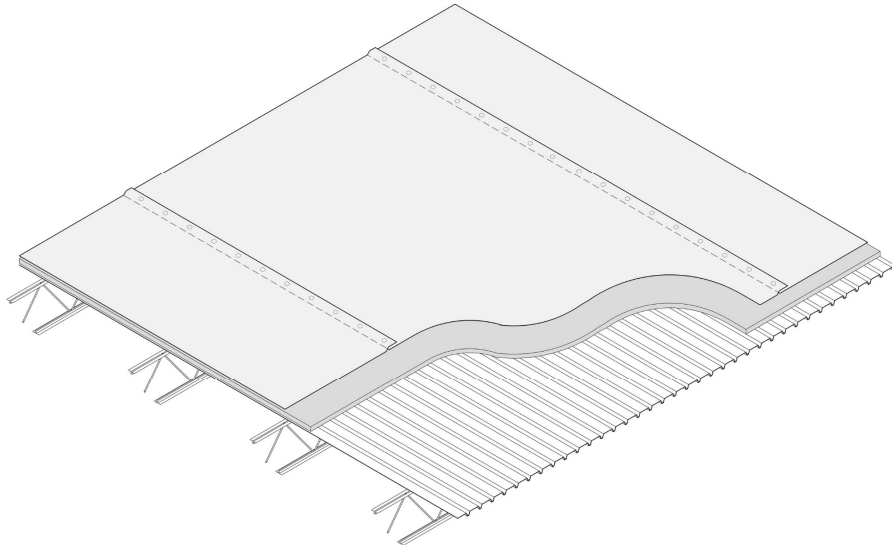
- Decks designed for joist spacing between 5' and 6' 8" o.c.
- Decks designed for uniform loading
- Seam-fastened single-ply membranes are a concern

Membrane seams across deck flutes



SDI: 3.8 X moment (deck); 2 X load (joists)

Membrane seams in deck flute direction



SDI: 12 X bending moment and shear (deck)

SDI bulletin -- Conclusion

“...SDI does not recommend the use of roofing membranes attached to the steel deck using line patterns with large spacing unless a structural engineer has reviewed the adequacy of the steel deck and the structural supports to resist to wind uplift loads transmitted along the lines of attachment. Those lines of attachment shall only be perpendicular to the flutes of the deck.”

FM 1-29 updated

www.fmglobaldatasheets.com

New criteria for steel roof deck uplift:

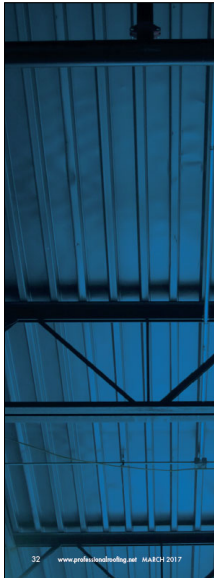
- Uniformly-distributed loading
- Concentrated loading

An example

Hypothetical analysis using FM 1-29

- Adhered (uniform loading) roof system:
 - 6 ft. joist spacing → Class 165
- Seam-fastened (nonuniform, linear load) roof system:
 - 6 ft. seam spacing → Class 90 (33 ksi steel deck)
 - 9.5 ft. seam spacing → Class 90 (80 ksi steel deck)
 - 6 ft. seam spacing → Class 165 (80 ksi steel deck)

Seam spacing wider than joist spacing is problematic



THE SITUATION WITH STEEL DECKS

Steel roof deck design can affect roof system selection and design

by Mark S. Graham

Professional Roofing

March 2017
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CONSTRUCTION ISSUES


discussion of construction issues and techniques

Are Your Roof Members Overstressed?

By James M. Fisher, Ph.D., F.E., Dux, M.ASCE and Thomas Spun, Ph.D., F.E., S.E., F.AISC

James M. Fisher is Vice President Director, Commercial Structural Design, Milwaukee, WI, and Consulting Engineer at the Steel Institute. He may be reached at jfisher@steelinstitute.com.

Thomas Spun is President of Spun and Lammers Engineering, LLC, Columbus, PA, and Technical Director of the Steel Deck Institute. He may be reached at tspin@steeldeck.com.



Membrane roof systems installed on steel deck traditionally result in a uniform transfer of wind uplift loads from the roof membrane to the steel roof deck and underlying supporting structure (e.g., steel joists). For example, in a built-up membrane roof system—which has been used commonly in the U.S. roofing industry for more than 125 years—the built-up membrane is continuously adhered to rigid board insulation. The rigid board insulation, which is used to span the steel deck's ribs, is mechanically attached to the steel roof deck in a closely-spaced pattern (e.g., 1 fastener per every 2 square feet), resulting in a near uniform uplift load path. Polymer-modified bitumen roof systems and adhered single-ply membrane roof systems are installed in similar configurations and result in a similar uniform uplift load path.

In the 1960s, single-ply membrane roof systems were first introduced into the U.S. roofing market. By the late 1970s, the seam-fastened, mechanically attached method of installation was first introduced. With this installation method, the single-ply membrane sheet is mechanically attached along its raised edges into the roof deck, which results in a larger tributary uplift load per fastener and placement of fasteners in lines, non-uniform loading configurations of the roof deck and underlying supporting structure. When first introduced, membrane sheet widths in seam-fastened single-ply membrane roof systems typically were five feet wide, resulting in rows of mechanical fasteners spaced at four on-center. Since the early 2000s, single-ply membrane sheet widths have become wider, with 16-foot-wide sheets now commonplace—resulting in rows of mechanical fasteners spaced at 10-foot on-center.

Currently, single-ply membrane roof systems have closely mimicked conventional built-up and polymer-modified bitumen membrane systems in market share. The seam-fastened, mechanically attached method of installation also has overlaid traditionally adhered methods of application. The National Roofing Contractors Association (NRCA) annual market survey shows seam-fastened, mechanically attached single-ply membrane roof systems make up the majority of all membrane roof systems currently installed.

With the present emphasis on wind resistance in design, a closer look at how seam-fastened mechanically attached single-ply membrane roof systems interact with steel roof deck and joint construction is in order.

A common method of single-ply membrane sheet layout is shown in Figure 1. A common placement of mechanical fasteners is shown in Figure 2. These concentrated line loads can severely overstress the steel deck and may also cause the steel joint before the deck to be overstressed under uplift loading. The behavior of such fastening systems, when the roof system is subjected to uplift loadings, is shown in Figure 3. The current trend in construction is for the membrane installer to mechanically fasten the membrane to the deck only along the edge of the sheet with its spread by the roof installation, thereby lowering installation costs. Unfortunately, the Structural Engineer of Record, and the steel deck and joint supplier, are usually unaware of the concentrated load pattern of the roof membrane attachment. In fact, the architect of record may not be aware of the modifications of such attachments. The Architectural roofing specifications may simply state that the roof membrane shall be installed per manufacturers' recommendations. The roofing installer however is the one who generally decides on the exact layout of the membrane sheets on the roof. This decision is made based on what layout can be installed in the fastest and least expensive

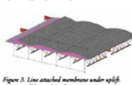


Figure 1. Line attached membrane under uplift. Courtesy of the Steel Deck Institute

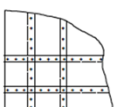


Figure 2. Typical fastener layout at corner seams.

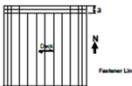


Figure 3. Typical membrane layout by rafters.

Structure magazine

March 2017
www.structuremag.org

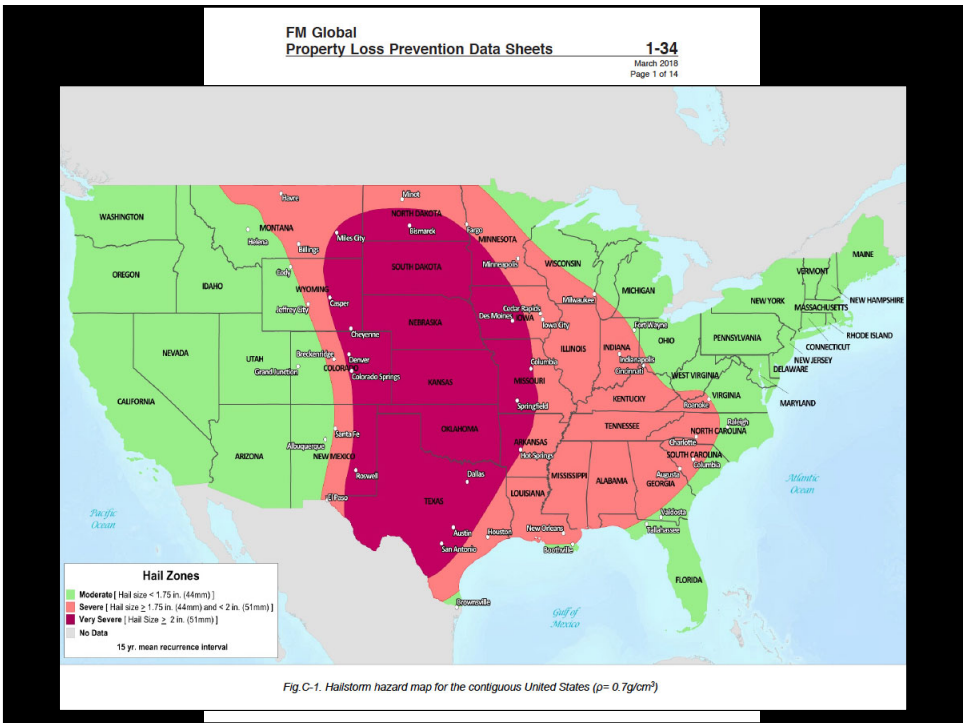
Fastener pull-out tests...

There is little correlation between fastener pull-out resistance and a steel roof deck's yield strength and uplift (bending) strength

Although roofing contractors sometimes are given the responsibility of inspecting and accepting steel roof decks to receive a new roof system, determining a roof deck's design adequacy is beyond the expertise of most roofing contractors.

This determination is best made during a project's design phase.

FM's very severe hail (VSH) classifications



Of the 892,903 roof assemblies in FM’s RoofNav,
only 301 have a VSH classification

As of Oct. 11, 2018

RESEARCH+TECH



Understanding FM VSH
FM has implemented a new impact-
resistance classification

by Mark S. Graham

To access FM Global Data Sheets,
including FM 1-36—“Hail Damage,”
go to www.professionalroofing.net.

22 www.professionalroofing.net DECEMBER 2017

Professional Roofing, December 2017

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RESEARCH+TECH



Designing for hail
resistance

Did you know FM Global has updated its
hail design guidance?

by Mark S. Graham

In March, property and building loss insurer FM Global updated its Property Loss Prevention Data Sheet 1-36, “Hail Damage” (FM 1-36). If you work on buildings insured by FM Global, you should be aware of its latest hail-resistance guidelines and the effects they may have on roof system selection and design.

FM 1-36

FM 1-36 provides new prescriptive guidelines to maintain the potential for hail damage to buildings, roof-mounted equipment and other outdoor equipment. FM Global intends FM 1-36 and its other Property Loss Prevention Data Sheets to apply to its insured buildings. However, some designers use the Property Loss Prevention Data Sheets as design guidelines for buildings (and/or equipment) other than those insured by FM Global.

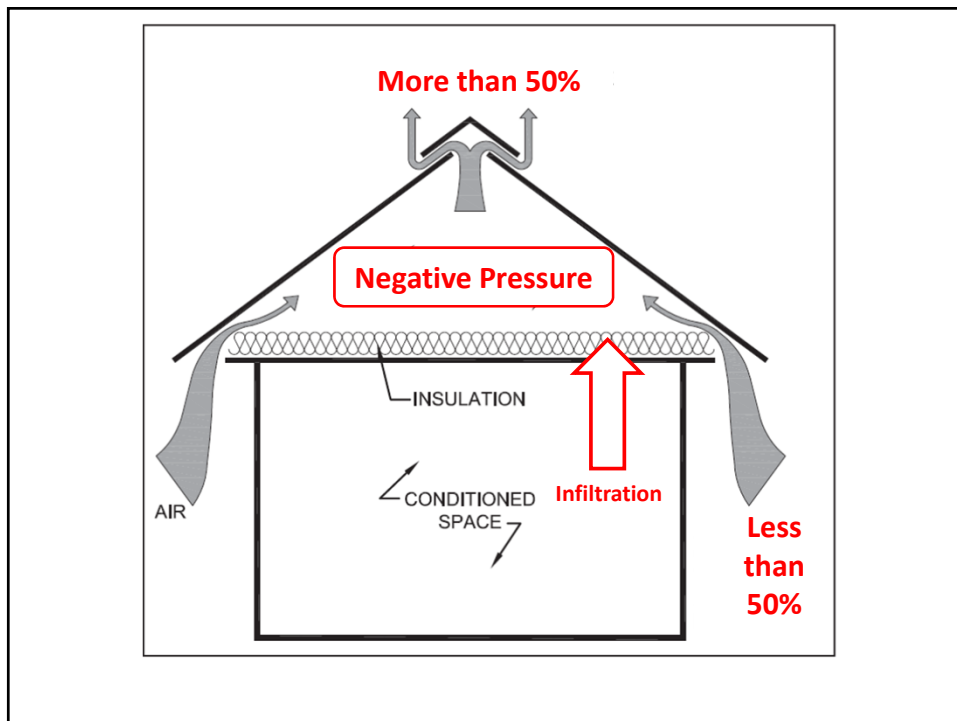
FM Global contends hailstorms are an widespread hazard affecting many areas of the world that can severely damage building roof systems, roofing HVAC units and skylights. Cooling towers and exposed glass and plastic components of outdoor equipment also can be

20 www.professionalroofing.net MAY 2018


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Attic ventilation



Be careful not to install excess amounts of ridge vents.... It can have undesirable consequences



RESEARCH+TECH

Professional Roofing

July 2018

Clearing the air

Considerations for attic ventilation
by Mark S. Graham

Proper attic ventilation can be an important consideration when designing high-performance, energy-saving roof assemblies. For example, with some asphalt shingle products, proper attic ventilation may be a necessary requirement. Following is a review of code requirements and NRCA's guidelines for attic ventilation in steep-slope roof assemblies.

Code requirements

Code requirements for attic ventilation have varied between the International Building Code® (IBC) and International Residential Code® (IRC) and have changed with each edition up to the 2018 editions.

Beginning with the IBC's and IRC's 2015 editions, both codes require the net free ventilating area (NFVA) to be at least a 1:150 ratio of the space being vented. Any blocking or bridging in attic must not interfere with air movement, and an air space of at least 1 inch must be provided between the bottom of the roof deck and any attic insulation. These openings must protect against the entrance of rain and snow and be installed according to manufacturer's installation instructions.

20 www.professionalroofing.net JULY 2018

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