Section 15:	MDOT Driveway Permits

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Rita Sawyer

Subject:

FW: Lots 6, 7 & 8 Shared Driveway

From: Fontaine, Anthony [mailto:Anthony.Fontaine@maine.gov]

Sent: Tuesday, May 28, 2019 11:02 AM

To: Greer, Tom

Subject: RE: Lots 6, 7 & 8 Shared Driveway

Permit 26800 (lot 7) is registered for 5 or fewer dwellings; accordingly, this covers the 3 proposed homes and should satisfy the town. I believe you should now have a permit for each driveway that satisfies our requirements as well as the town. If you feel otherwise, let me know.

From: Greer, Tom [mailto:tgreer@walsh-eng.com]

Sent: Tuesday, May 28, 2019 9:27 AM

To: Fontaine, Anthony < Anthony. Fontaine@maine.gov >

Subject: Re: Lots 6, 7 & 8 Shared Driveway

EXTERNAL: This email originated from outside of the State of Maine Mail System. Do not click links or open attachments unless you recognize the sender and know the content is safe.

thanks I appreciate your help.

On Tue, May 28, 2019, 9:09 AM Fontaine, Anthony < Anthony.Fontaine@maine.gov > wrote:

Thanks Tom,

I'll review this week. Was out of the office last week and don't recall the permit details but think I approved each location for 5 or fewer dwellings. If that is the case AND this is corresponds to the location approved the week before, it should be okay. Again, I'll review this week and confirm.

From: Tom Greer [mailto:tgreer@walsh-eng.com]

Sent: Monday, May 20, 2019 11:19 AM

To: Fontaine, Anthony < Anthony. Fontaine@maine.gov>

Cc: Nathan Wadsworth < nwadsworth@wadsworthwoodlands.com >; Ralph Austin < nwa@woodedlaw.com >

Subject: FW: Lots 6, 7 & 8 Shared Driveway

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Hi Tony,

Thanks for following up on lot 8. We had a location in the middle of the lot but it had less than the town required 450 feet. We have added lot 8 to the driveway of lot 7, see attached plan. Does that work for you?

Tom Greer, PE

Walsh Engineering Associates, Inc.

Office: 207.553.9898 tgreer@walsh-eng.com

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Maine Department of Transportation

Driveway/Entrance Permit

Bruce A. Van Note

Commissioner

Permit Number: 26798 - Entrance ID: 1

OWNER

Name: NATESELLSREALESTATE, LLC

Address:

P.O. Box 321

Cornish, ME 04020

Telephone:

Date Printed: May 15, 2019

LOCATION

Route:

0160X, North Road

Municipality:

Parsonsfield York

Tax Map:

County:

R8 Lot Number: 37

Culvert Size:

15 inches

Culvert Type:

plastic 26 feet

Culvert Length:
Date of Permit:

May 15, 2019

Approved Entrance Width: 15 feet

In accordance with rules promulgated under 23 M.R.S.A., Chapter 13, Subchapter I, Section 704, the Maine Department of Transportation (MaineDOT) approves a permit and grants permission to perform the necessary grading to construct, in accordance with sketch or attached plan, a Driveway to Five or fewer single family dwellings at a point 1473 feet East from Hussey Road, subject to the Chapter 299 Highway Driveway and Entrance Rules, standard conditions and special conditions (if any) listed below.

Conditions of Approval:

This Permittee acknowledges and agrees to comply with the Standard Conditions and Approval attached hereto and to any Specific Conditions of Approval shown here.

(G = GPS Location; W = Waiver; S = Special Condition)

G - THE ENTRANCE SHALL BE LOCATED AT GPS COORDINATES: 43.746790N, -70.902900W.

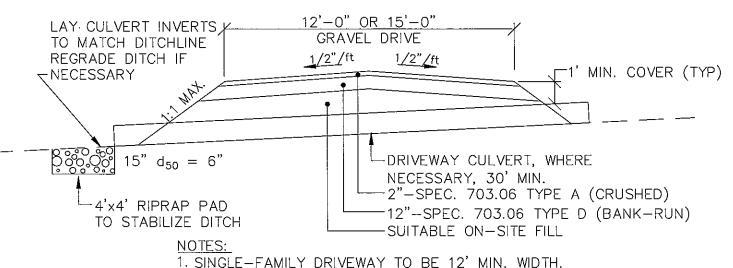
S - In the town of Parsonsfield on the northerly side of Route 160 / North Road, the centerline being approximately 1473 feet easterly of the centerline of Hussey Road and approximately 36 feet westerly of utility pole 105

S - The culvert shall be HDPE smoothbore plastic pipe. Ditching is required, of sufficient length and depth so as to provide gradual water flow into the pipe, leave no standing water on the outlet end of the pipe, and provide sufficient cover over the pipe as recommended by the manufacturer. The Property Owner must contact MaineDOT at (207) 324-5322 prior to driveway and culvert installation to arrange for an inspection.

Approved by:

Date

5/15/2019



1. SINGLE—FAMILY DRIVEWAY TO BE IZ MIN. WIDTH.

 2 COMMON / SHARED DRIVEWAY TO BE 15' MIN. WIDTH.

TYPICAL DRIVEWAY W/ CULVERT DETAIL

NOT TO SCALE

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One Karen Dr., Suite 2A | Westbrook, Maine 04092 ph: 207.553.9898 | www.walsh-eng.com

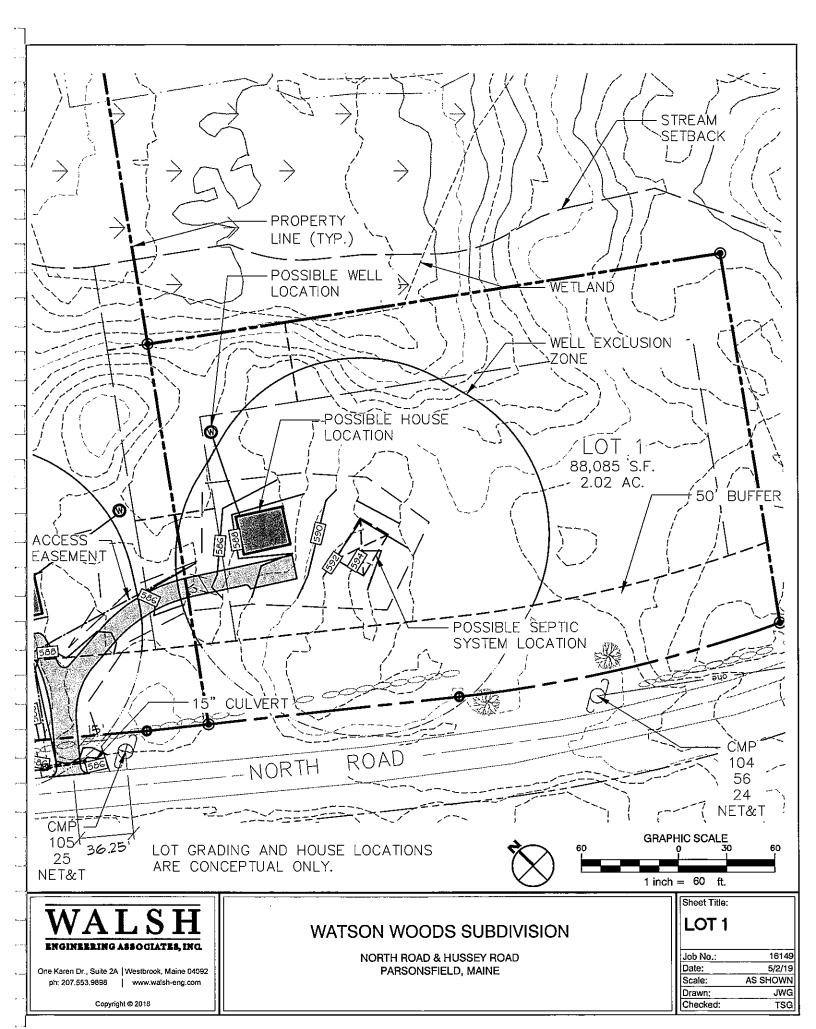
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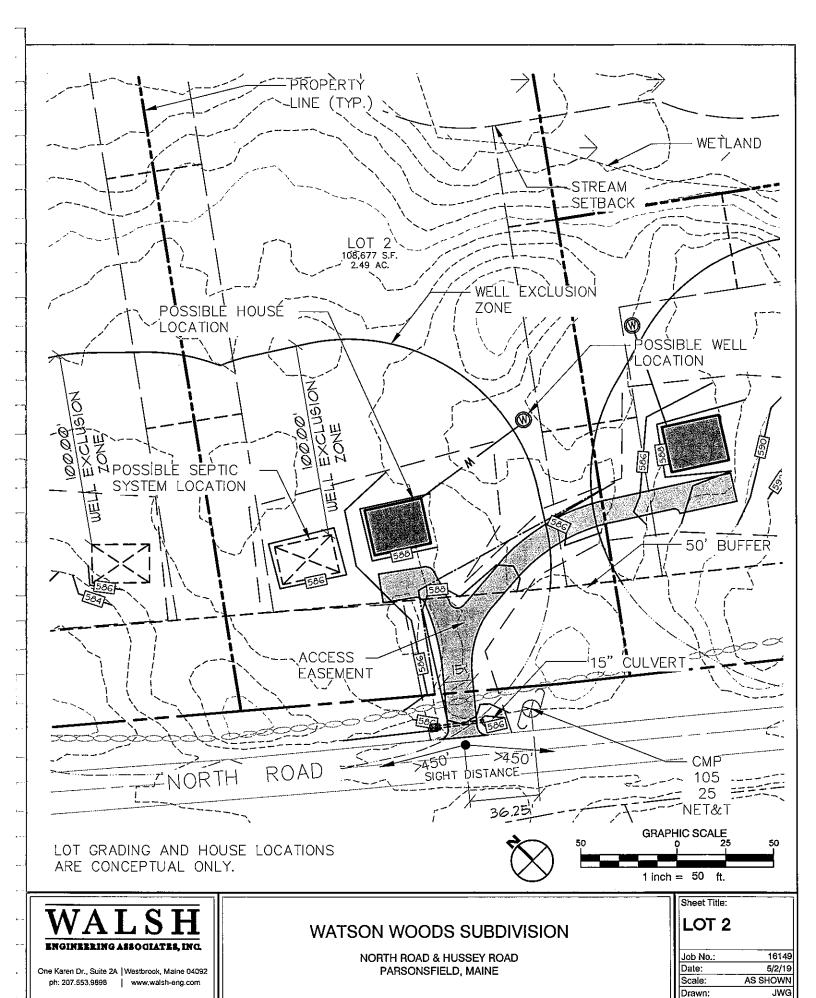
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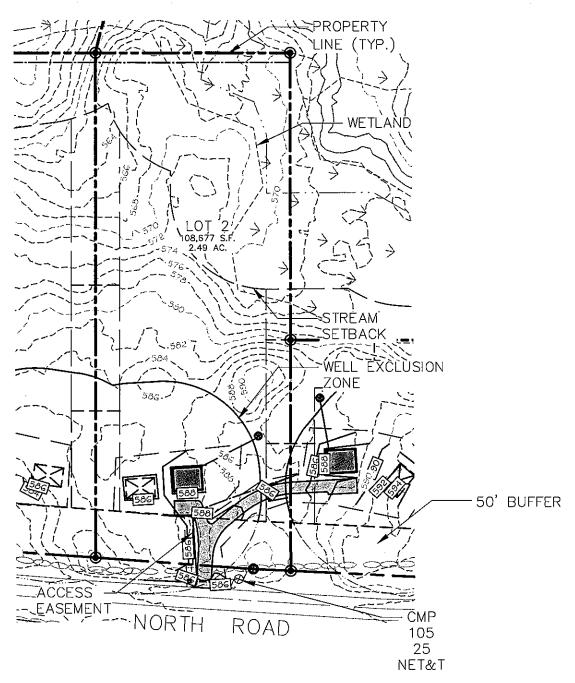
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Maine Department of Transportation

Driveway/Entrance Permit

Bruce A. Van Note

Commissioner

Permit Number: 26799 - Entrance ID: 1

OWNER

Name:

NATESELLSREALESTATE, LLC

Address:

P.O. Box 321

Cornish, ME 04020

Telephone:

Date Printed: May 15, 2019

Route:

0160X, North Road

Municipality:

Parsonsfield

County:

York

LOCATION

Tax Map: Culvert Size: R8 Lot Number: 37 15 inches

Culvert Type:

plastic

Culvert Length:

26 feet

Date of Permit:

May 15, 2019

Approved Entrance Width: 15 feet

In accordance with rules promulgated under 23 M.R.S.A., Chapter 13, Subchapter I, Section 704, the Maine Department of Transportation (MaineDOT) approves a permit and grants permission to perform the necessary grading to construct, in accordance with sketch or attached plan, a Driveway to Five or fewer single family dwellings at a point 1145 feet East from Hussey Road, subject to the Chapter 299 Highway Driveway and Entrance Rules, standard conditions and special conditions (if any) listed below.

Conditions of Approval:

This Permittee acknowledges and agrees to comply with the Standard Conditions and Approval attached hereto and to any Specific Conditions of Approval shown here.

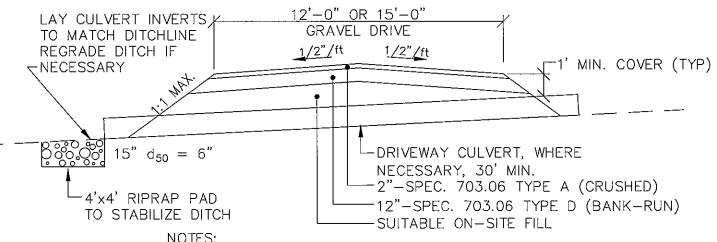
(G = GPS Location; W = Waiver; S = Special Condition)

G - THE ENTRANCE SHALL BE LOCATED AT GPS COORDINATES: 43.747560N, -70.904170W.

S - In the town of Parsonsfield on the northerly side of Route 160 / North Road, the centerline being approximately 1145 feet easterly of the centerline of Hussey Road and approximately 55 feet westerly of utility pole 106

S - The culvert shall be HDPE smoothbore plastic pipe. Ditching is required, of sufficient length and depth so as to provide gradual water flow into the pipe, leave no standing water on the outlet end of the pipe, and provide sufficient cover over the pipe as recommended by the manufacturer. The Property Owner must contact MaineDOT at (207) 324-5322 prior to driveway and culvert installation to arrange for an inspection

Approved by: Anthony Fortain Date: 5-16-2019



1. SINGLE—FAMILY DRIVEWAY TO BE 12' MIN. WIDTH.

2. COMMON / SHARED DRIVEWAY TO BE 15' MIN. WIDTH.

TYPICAL DRIVEWAY W/ CULVERT DETAIL

NOT TO SCALE

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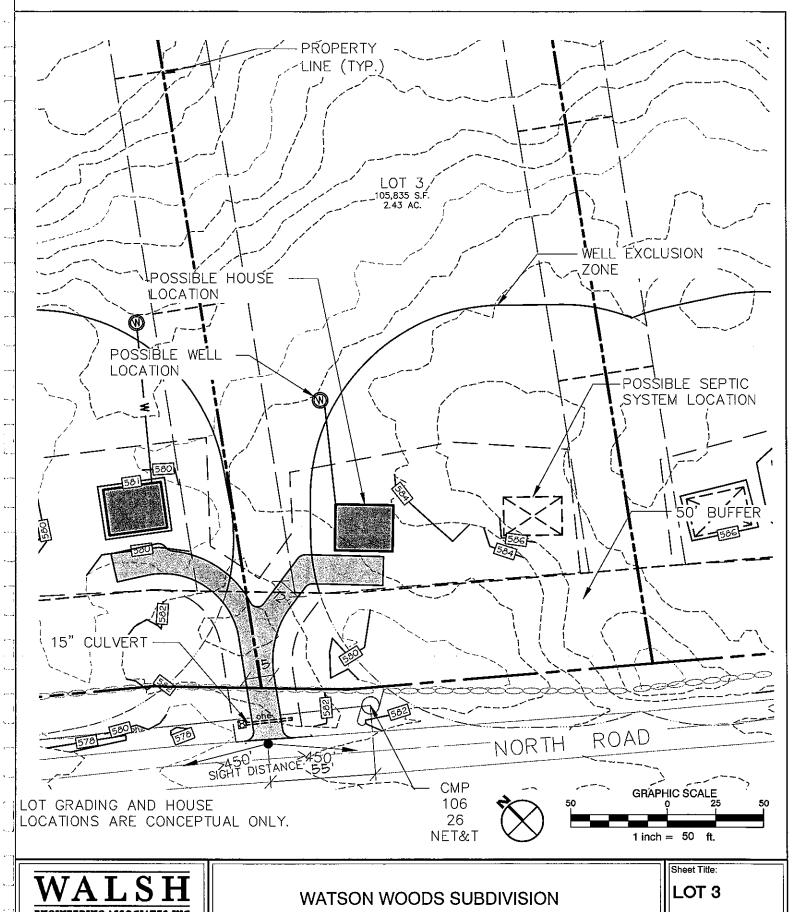
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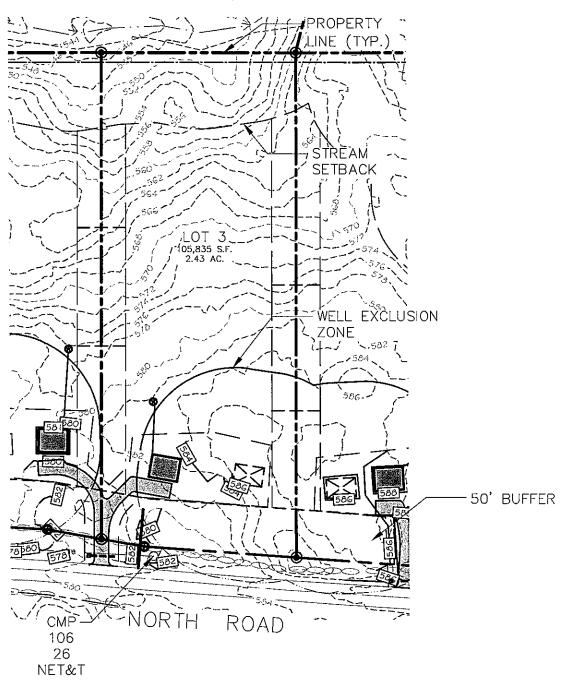


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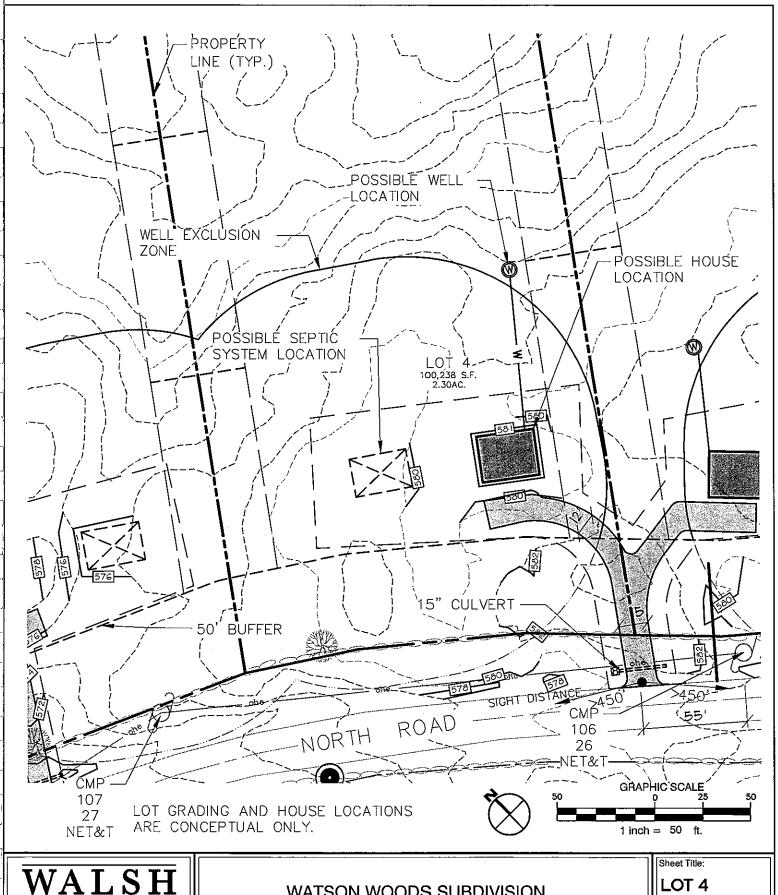
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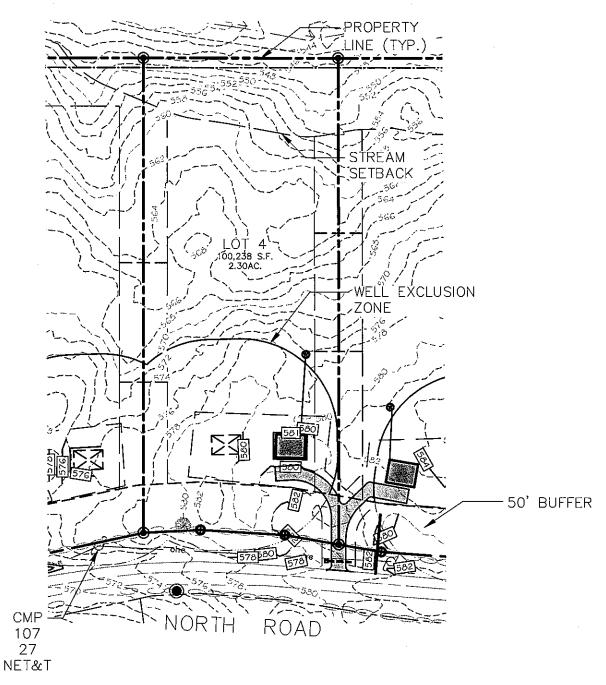
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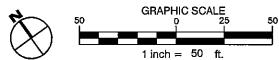
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Maine Department of Transportation

Paul R. LePage Governor

Driveway/Entrance Permit

David Bernhardt, P.E, Commissioner

ermit Number	: 25577 -	- Entrance ID: 1	1
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OWNER

Name:

NATESELLSREALESTATE, LLC

Address:

P.O. Box 321

Cornish, ME 04020

Telephone:

Date Printed: January 25, 2018

LOCATION

Route:

0160X, North Road

Municipality:

Parsonsfield

County:

York

Tax Map:

R08 Lot Number: 37

Culvert Size: Culvert Type: 15 inches plastic

Culvert Length:

24 feet

Date of Permit:

January 25, 2018

Approved Entrance Width: 12 feet

In accordance with rules promulgated under 23 M.R.S.A., Chapter 13, Subchapter I, Section 704, the Maine Department of Transportation (MaineDOT) approves a permit and grants permission to perform the necessary grading to construct, in accordance with sketch or attached plan, a Driveway to Single Family Dwelling at a point 844 feet East from Hussey Road, subject to the Chapter 299 Highway Driveway and Entrance Rules, standard conditions and special conditions (if any) listed below.

Conditions of Approval:

This Permittee acknowledges and agrees to comply with the Standard Conditions and Approval attached hereto and to any Specific Conditions of Approval shown here.

(G = GPS Location; W = Waiver; S = Special Condition)

G - THE ENTRANCE SHALL BE LOCATED AT GPS COORDINATES: 43.747880N, -70.904910W.

S - In the town of Parsonsfield on the northerly side of Route 160 / North Road, the centerline being approximately 844 feet easterly of the centerline of Hussey Road and approximately 70 feet westerly of utility pole 107.

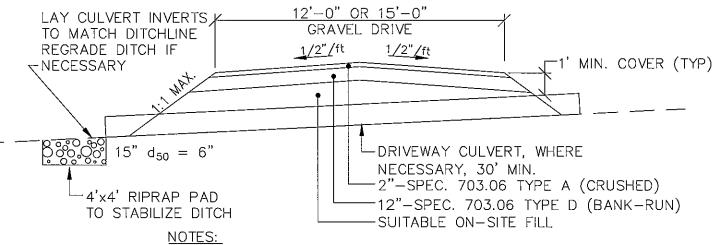
S - The culvert shall be HDPE smoothbore plastic pipe. Ditching on either side of the pipe is likely required but will need to be determined after snow melt. The Property Owner must contact MaineDOT at (207) 324-5322 prior to driveway and culvert installation to discuss ditching requirements and arrange an inspection.

Approved by:

Date

1-25-2018

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1. SINGLE-FAMILY DRIVEWAY TO BE 12' MIN. WIDTH.

2. COMMON / SHARED DRIVEWAY TO BE 15' MIN. WIDTH.

TYPICAL DRIVEWAY W/ CULVERT DETAIL

NOT TO SCALE

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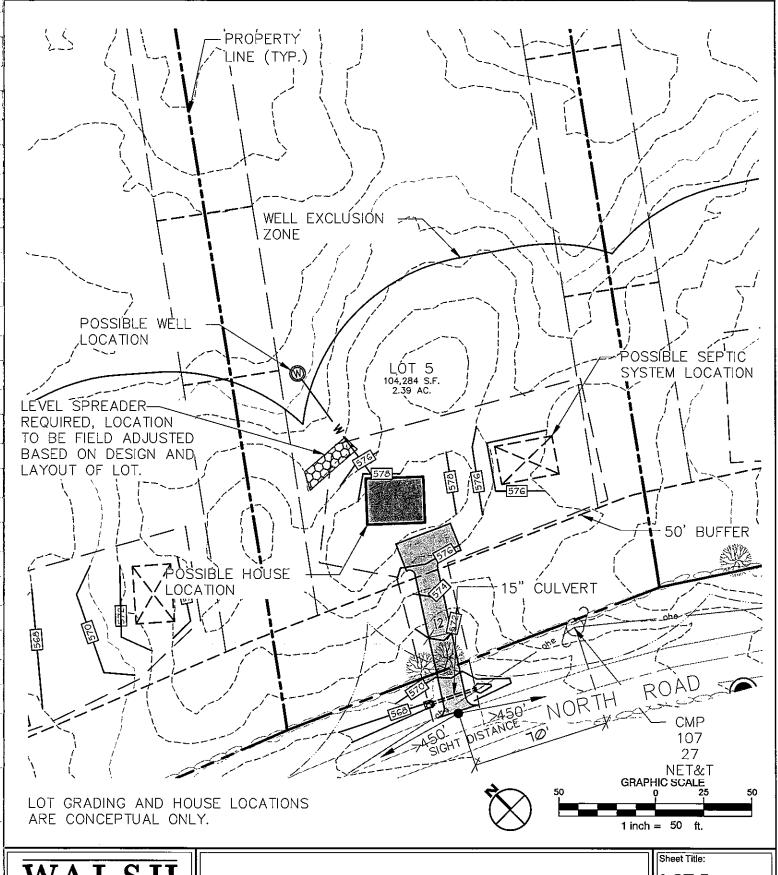
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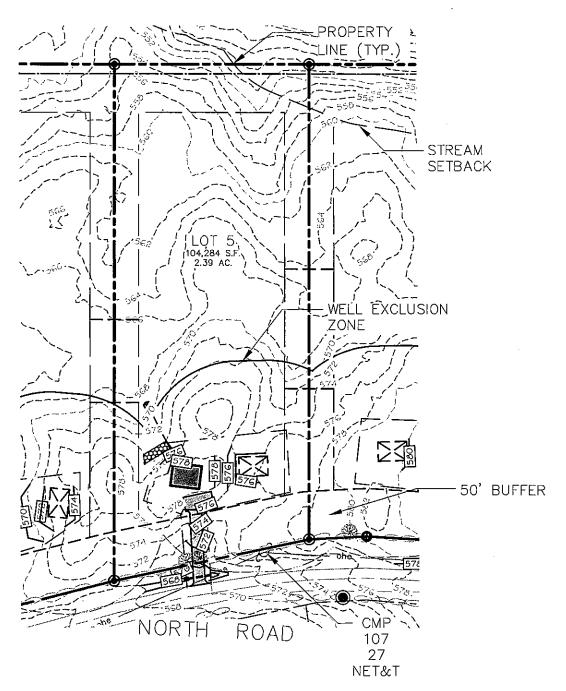
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Maine Department of Transportation

Driveway/Entrance Permit

Bruce A. Van Note Commissioner

Permit Number: 26800 - Entrance ID: 1

OWNER

Name: Address:

NATESELLSREALESTATE, LLC

P.O. Box 321

Cornish, ME 04020

Telephone:

Date Printed: May 16, 2019

Route:

0160X, North Road

Municipality:

Parsonsfield

County:

York

LOCATION

Tax Map:

R8 Lot Number: 37 15 inches

Culvert Size: Culvert Type:

plastic

Culvert Length: Date of Permit:

26 feet May 16, 2019

Approved Entrance Width: 15 feet

In accordance with rules promulgated under 23 M.R.S.A., Chapter 13, Subchapter I, Section 704, the Maine Department of Transportation (MaineDOT) approves a permit and grants permission to perform the necessary

grading to construct, in accordance with sketch or attached plan, a Driveway to Five or fewer single family dwellings at a point 486 feet East from Hussey Road, subject to the Chapter 299 Highway Driveway and

Entrance Rules, standard conditions and special conditions (if any) listed below.

Conditions of Approval:

This Permittee acknowledges and agrees to comply with the Standard Conditions and Approval attached hereto and to any Specific Conditions of Approval shown here.

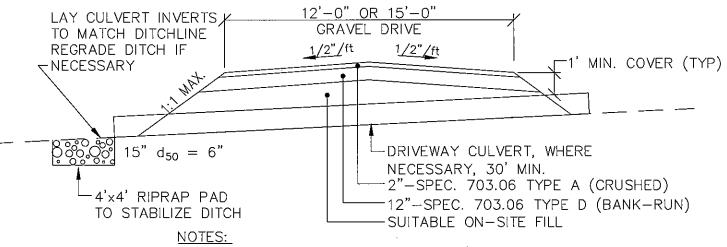
(G = GPS Location; W = Waiver; S = Special Condition)

G - THE ENTRANCE SHALL BE LOCATED AT GPS COORDINATES: 43.748180N, -70.905990W.

S - In the town of Parsonsfield on the northerly side of Route 160 / North Road, the centerline being approximately 486 feet easterly of the centerline of Hussey Road and approximately 164 feet westerly of utility pole 108

S - The culvert shall be HDPE smoothbore plastic pipe. Ditching is required, of sufficient length and depth so as to provide gradual water flow into the pipe, leave no standing water on the outlet end of the pipe, and provide sufficient cover over the pipe as recommended by the manufacturer. The Property Owner must contact MaineDOT at (207) 324-5322 prior to driveway and culvert installation to arrange for an inspection.

Autory Conteins Date: 5-16-2019



1. SINGLE-FAMILY DRIVEWAY TO BE 12' MIN. WIDTH.

2. COMMON / SHARED DRIVEWAY TO BE 15' MIN. WIDTH.

TYPICAL DRIVEWAY W/ CULVERT DETAIL

NOT TO SCALE



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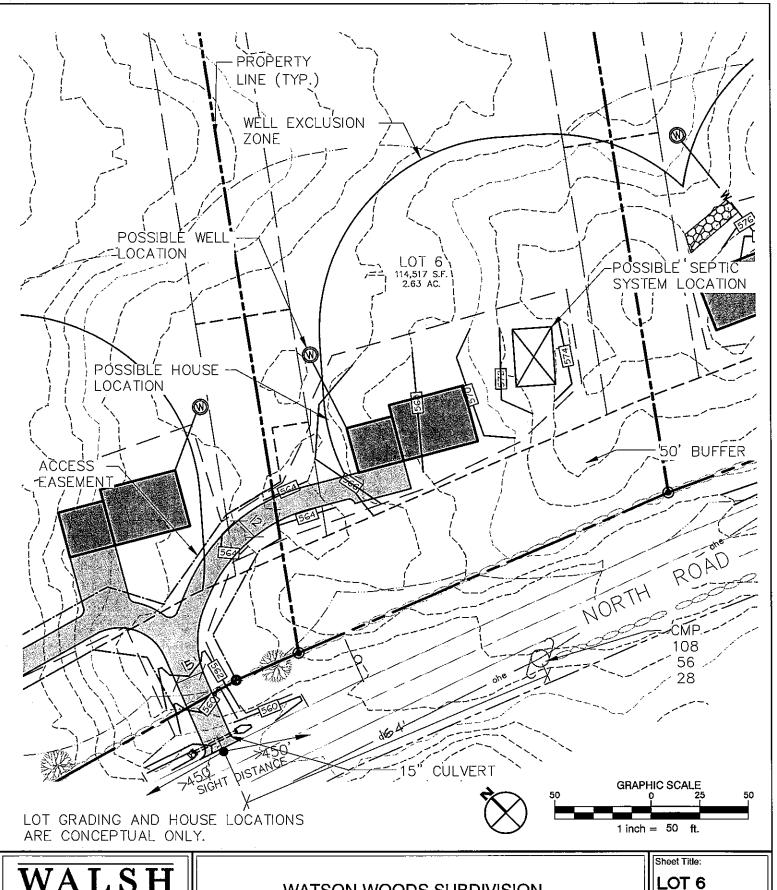
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NORTH ROAD & HUSSEY ROAD PARSONSFIELD, MAINE

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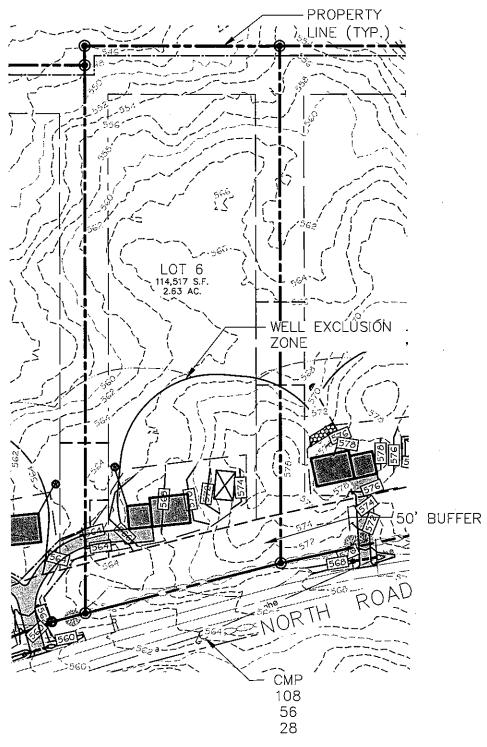
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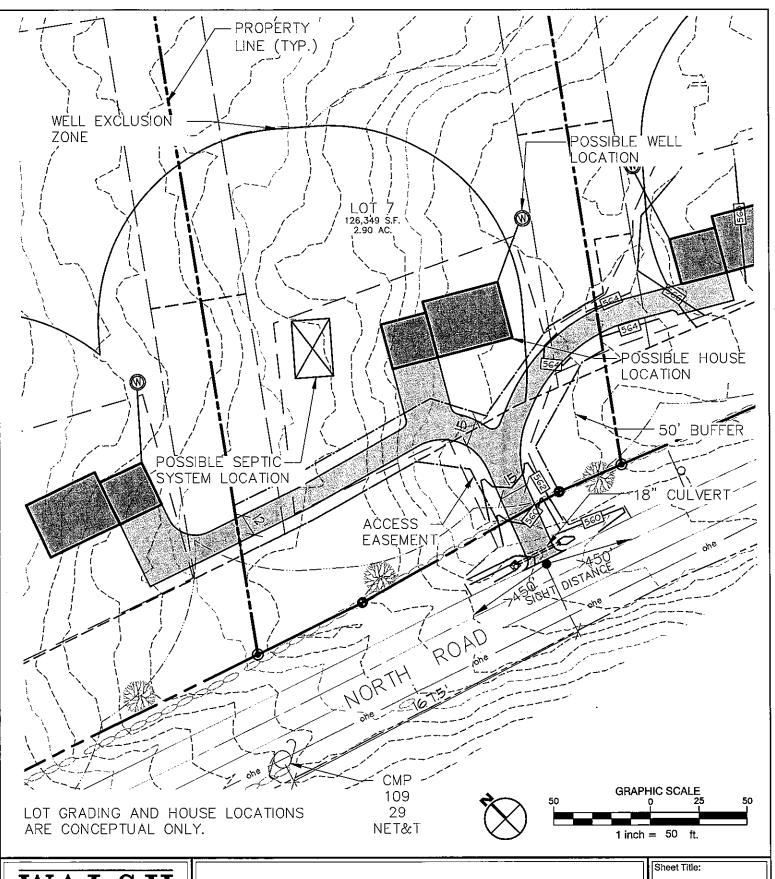
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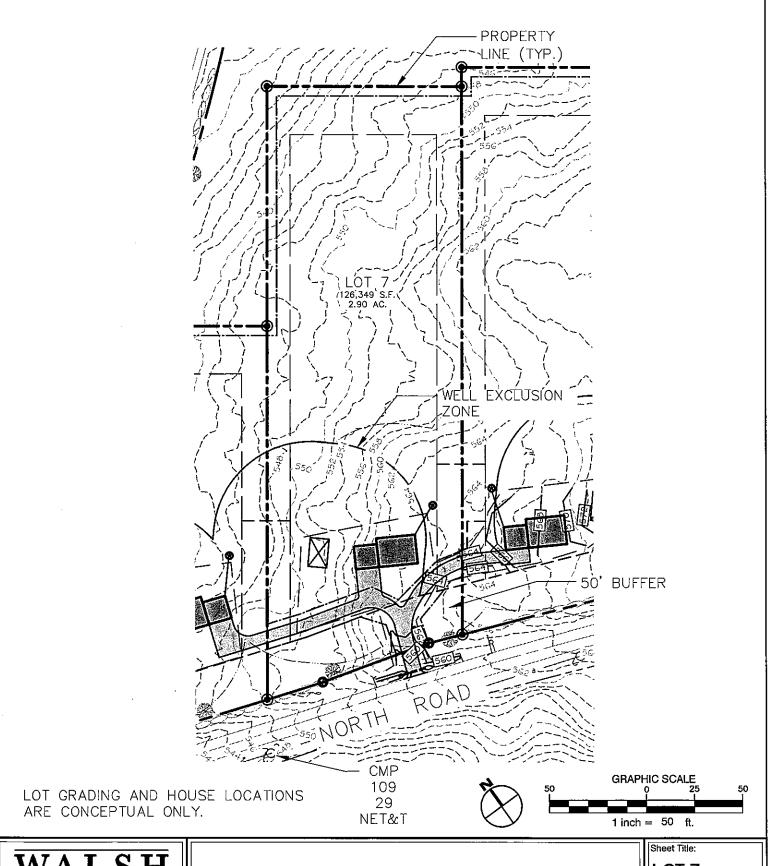
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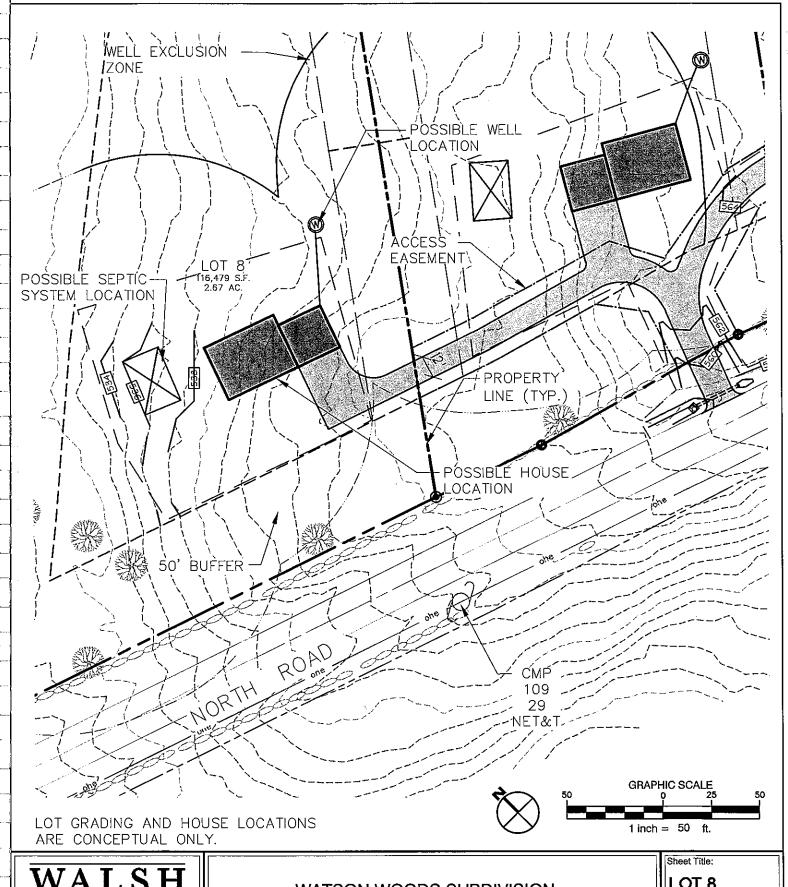
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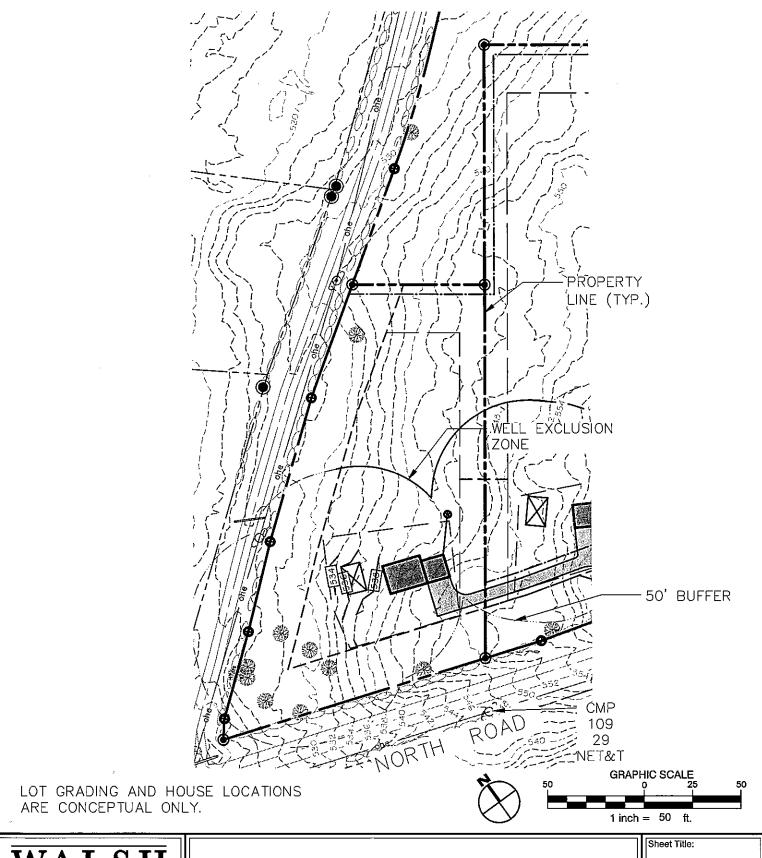
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Stormwater Management Report Watson Woods Parsonsfield, Maine

May 3, 2019

Prepared by:

WALSH ENGINEERING ASSOCIATES, INC.

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STORMWATER MANAGEMENT REPORT WATSON WOODS PARSONSFIELD, MAINE

May 3, 2019

Project Description:

This project, located at the corner of Hussey Road and Route 160, is the division of one (1), 55.76-acre lot into eight (8) single-family residential lots, 8.66 acres of common open space, and 28.06 acres of remaining land, retained by the owner. The lots average 2.38 acres. The lots are accessed from North Road. No roads will be created with this subdivision.

Surface Water:

The watershed consists of 116 acres. About half of that is on neighboring properties. The watershed has been divided into three (3) subcatchments for analysis.

Subcatchment #1 is along North Road from the start of the proposed lot #1 to the 24" culvert that crosses Hussey Road. This culvert is the first Point of Analysis (POA).

Subcatchment #2 is to the north of subcatchment #1 and covers the middle of proposed lots #6-8 and runs 280 feet along Hussey Road. It discharges at the 15" culvert that crosses Hussey Road. This culvert is POA #2.

Subcatchment #3 is the majority of the site and neighboring lots. The offsite area is wooded. The onsite area was recently logged and has been left to revegetate. This area drains to a 30" culvert that crosses Hussey Road. This culvert is POA #3.

Flooding:

The project is <u>not</u> located within the 100-year flood plain, as shown on the attached FEMA Flood Insurance Rate Map Community Panel 230154 0015 B.

Groundcover, Topography and Soils:

The site consists of Skerry, Brayton, and Becket soils. All three fall into the hydrologic soil group C. The ground cover onsite is naturally being revegetated from being logged. The watershed is the western side of an unnamed hill. The hill slopes from elevation 780 to elevation 518 at Hussey Road.

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Alteration of Natural Drainage Ways and Land Cover:

The development of the lots will not alter the natural drainage ways for the site. There is a buffer along the front of the lots that treats stormwater from the project. The natural swale and wetlands down the middle of the site will remain in their existing conditions and will not be impacted. No wetland impacts are anticipated.

Methodology:

HydroCAD version 10.0 developed by HydroCAD Software Solutions LLC of Chocurua, NH is used to model the hydrology and hydraulics of the site and design the hydraulics of stormwater management measures and facilities.

Peak flows for the 2-year (3.3"), 10-year (4.9"), and 25-year (6.2") storm events in a 24-hour period are analyzed for pre- and post-developed conditions. Precipitation values used in the model are taken from the manual: *CHAPTER 500: STORMWATER MANAGEMENT, Appendix H.* The rainfall values used are for York County, Maine.

Times of concentration methods include TR-55 sheet flow and shallow concentrated flow. Runoff curve numbers are selected from Tables 2-2a and 2-2c of the SCS TR-55 manual, which are included in the HydroCAD software. Watershed subcatchments are as delineated on Drawing D1.0 for pre-developed and post-developed conditions. Modeling assumptions made for each subcatchment, culvert, and Best Management Practice (BMP) with the site in its pre-and post-developed conditions are summarized in the HydroCAD reports attached as **Appendices B and C**.

Proposed BMPs:

Standard erosion and sediment control will be used during the development of the lots. No other BMPs are required for this project. A level spreader will be utilized on Lot 5 to control the peak rate of runoff from the site.

Water Quantity:

The development of this project will not have a significant impact on the peak rate of runoff from the site. See Table 1 for POA runoff values. See attached HydroCAD model for clarification.

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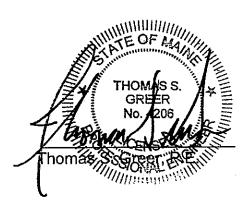


TABLE 1

	PEAK RATE OF RUNOFF (CFS)						
POINT OF	2-YR	10-YR	25-YR	2-YR	10-YR	25-YR	
ANALYSIS	PRE-DEVELOPMENT CONDITIONS			POST-DEVELOPMENT CONDITIONS			
POA # 1	3.81	8.06	11.81	3.16	7.21	10.68	
POA # 2	2.13	4.79	6.66	2.13	4.68	6.57	
POA # 3	25.89	67.48	106.57	25.83	67.30	106.29	

Conclusion:

The HydroCAD model predicts that peak stormwater runoff rates at the points of analyses will be reduced compared to the existing conditions. Therefore, this project will not have a significant impact on adjacent properties or receiving waters.



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APPENDIX H. 24-hour duration rainfalls for various return periods

COUNTY	Storm Type	1-YR	2- YR	5- YR	10- YR	25- YR	50- YR	100- YR	500- YR
ANDROSCOGGIN	III	2.5	3.0	3.7	4.3	5.4	6.4	7.6	11.1
AROOSTOOK C	II	1.9	2.3	2.8	3.2	3.9	4.6	5.3	7.6
(Presque Isle Area)					٠.٠	3.5	1.0	3.3	7.0
AROOSTOOK N (Fort Kent Area)	II	1.9	2.2	2.7	3.1	3.7	4.3	5.0	7.0
AROOSTOOK S									
(Houlton Area)	П	2.1	2.5	3.0	3.4	4.1	4.7	5.4	7.5
CUMBERLAND									
NW	Ш	2.5	3.0	3.7	4.3	5.4	6.3	7.5	10.9
(Bridgton Area)									
CUMBERLAND	777								
SE (N Windham Area)	Ш	2.6	3.1	3.9	4.6	5.8	6.9	8.1	12.1
FRANKLIN	11	2.0	2,4	2.9	2.4	4.0	4.0		0.0
HANCOCK	III	2.5	2.4	3.6	3.4 4.2	4.2 5.2	4.9 6.1	5.7 7.2	8.2 10.5
KENNEBEC	III	2.4	2.8	3.5	4.2	5.2	6.1	7.2 7.2	10.5
KNOX	Ш	2.6	3.2	3.9	4.6	5.7	6.7	7.2 7.9	11.5
LINCOLN	III	2.5	3.1	3.8	4.5	5.5	6.5	7.6	11.1
OXFORD E	II^1	2.3	2.7	3.3	3.9	4.8	5.7	6.7	
(Rumford Area)	**	2.0	2.1	د.د	3.9	-1.0	3.7	0.7	9.7
OXFORD W	II	2.2	2.7	3.4	4.0	4.9	5.8	6.9	10.1
(Gilead Area)							0.0	0.7	10.1
PENOBSCOT N (Millinocket Area)	II	2.2	2.6	3.2	3.8	4.7	5.6	6.5	9.5
PENOBSCOT S									
(Hudson Area)	II	2.3	2.7	3.4	3.9	4.9	5.7	6.7	9.7
PISCATAQUIS N	II	2.0	2.4						
(Chesuncook Area)	11	2.0	2.4	2.9	3.4	4.2	5.0	5.8	8.5
PISCATAQUIS S	II	2.2	2.7	3.3	3.9	4.8	5.7	6.8	10.0
(Monson Area)									10.0
SAGADAHOC	III	2.6	3.2	3.9	4.6	5.7	6.7	7.8	11.4
SOMERSET N (Pittston Farm Area)	H	2.0	2.3	2.8	3.3	4.0	4.7	5.4	7.8
SOMERSET S									
(Solon Area)	II	2.3	2.7	3.4	3.9	4.9	5.7	6.7	9.8
WALDO	Ш	2.4	2.9	3.6	4.2	5.2	6.1	7.2	10.5
WASHINGTON	Ш	2.5	2.8	3.4	3.9	4.8	5.5	6.4	9.0
YORK	111	2.6	3.3	4.1	4.9	6.2	7.3	8.7	13.2

¹ Use Type III rainfall for the towns of Brownfield, Buckfield, Denmark, Hartford, Hebron, Hiram, Oxford, and Porter.

Source: Data extracted by the Maine Department of Environmental Protection from the Northeast Regional Climate Center website (http://precip.eas.cornell.edu), Extreme Precipitation Tables. Data from this website was obtained from the National Oceanic and Atmospheric Administration's Regional Climate Center Program.

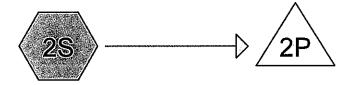
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EXISTING CONDITIONS



SOUTH SIDE ALONG NORTH ROAD CULVERT AT HUSSEY ROAD, POA 1



AREA OF LOT 7+, ALONG HUSSEY ROAD

CULVERT AT HUSSEY ROAD 15", POA 2



CENTRAL SITE PLUS
OFF SITE

CULVERT AT HUSSEY ROAD 32", POA 3









Routing Diagram for 16149 EX DEV 052118

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Type III 24-hr 2-YEAR Rainfall=3.30"

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<u> Page 2</u>

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: SOUTH SIDE ALONG Runoff Area=223,131 sf 10.11% Impervious Runoff Depth>1.05" Flow Length=878' Tc=29.4 min CN=75 Runoff=3.81 cfs 0.449 af

Subcatchment 2S: AREA OF LOT 7+, Runoff Area=140,510 sf 1.49% Impervious Runoff Depth>0.89" Flow Length=875' Tc=25.1 min CN=72 Runoff=2.13 cfs 0.240 af

Subcatchment 3S: CENTRAL SITE PLUS Runoff Area=4,720,717 sf 0.63% Impervious Runoff Depth>0.68" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=26.52 cfs 6.107 af

Pond 1P: CULVERT AT HUSSEY ROAD, POA 1 Peak Elev=515.34' Storage=79 cf Inflow=3.81 cfs 0.449 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0383 '/' Outflow=3.81 cfs 0.449 af

Pond 2P: CULVERT AT HUSSEY ROAD 15", Peak Elev=524.93' Storage=84 cf Inflow=2.13 cfs 0.240 af 15.0" Round Culvert n=0.013 L=30.0' S=0.0133 '/' Outflow=2.13 cfs 0.240 af

Pond 3P: CULVERT AT HUSSEY ROAD 32", Peak Elev=520.72' Storage=8,332 cf Inflow=26.52 cfs 6.107 af Outflow=25.89 cfs 6.079 af

Total Runoff Area = 116.721 ac Runoff Volume = 6.797 af Average Runoff Depth = 0.70" 98.93% Pervious = 115.473 ac 1.07% Impervious = 1.248 ac

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Summary for Subcatchment 1S: SOUTH SIDE ALONG NORTH ROAD

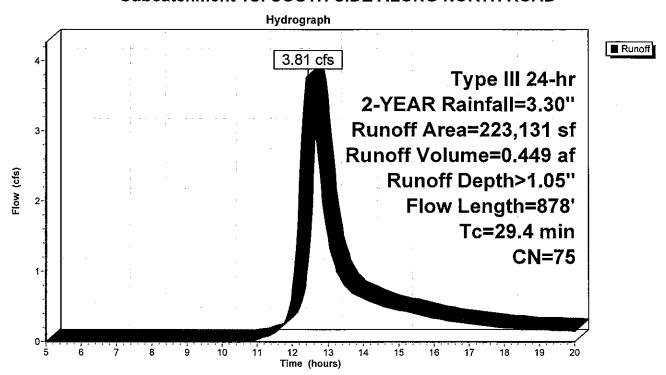
Runoff = 3.81 cfs @ 12.44 hrs, Volume=

0.449 af, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

	Α	rea (sf)	CN E	escription		
*		22,558	98 F	aved road	, HSG C	
*	2	00,573	72 V	<u>Voods, HS</u>	G C	
	223,131 200,573			Veighted A 9.89% Per	verage vious Area	
		22,558	1	0.11% lmp	ervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	21.1	100	0.0800	0.08		Sheet Flow, WEST IN WOODS Woods: Dense underbrush n= 0,800 P2= 3,30"
	2.7	157	0.0380	0.97		Shallow Concentrated Flow, IN WOODS Woodland Kv= 5.0 fps
	4.2	391	0.0970	1.56		Shallow Concentrated Flow, IN WOODS Woodland Kv= 5.0 fps
	1.4	230	0.0350	2.81		Shallow Concentrated Flow, IN ROAD DITCH Grassed Waterway Kv= 15.0 fps
	29.4	878	Total			

Subcatchment 1S: SOUTH SIDE ALONG NORTH ROAD



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Summary for Subcatchment 2S: AREA OF LOT 7+, ALONG HUSSEY ROAD

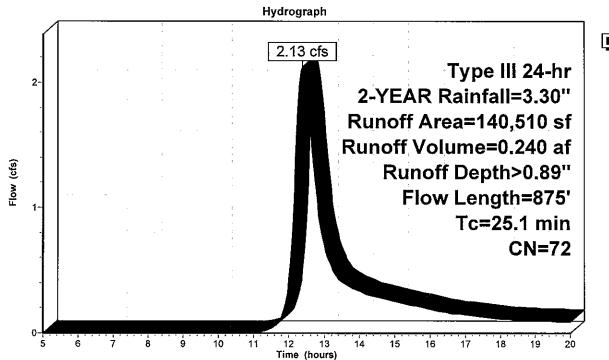
Runoff = 2.13 cfs @ 12.39 hrs, Volume=

0.240 af, Depth> 0.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

	Α	rea (sf)	CN	Description		
*		2,095	98	Paved road	, HSG C	
*	1	38,415	72	Woods, HS	GC	
	140,510 72 Weighted Average 138,415 98.51% Pervious Area 2,095 1.49% Impervious Area				vious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	14.7	100	0.0500	0.11		Sheet Flow, NORTH IN WOODS Woods: Light underbrush n= 0,400 P2= 3,30"
	8.2	625	0.0640	1.26		Shallow Concentrated Flow, NORTH WEST THROUGH WOOD Woodland Kv= 5.0 fps
_	2.2	150	0.0130	1.14		Shallow Concentrated Flow, SOUTH WEST IN ROAD DITCH Nearly Bare & Untilled Kv= 10.0 fps
	25.1	875	Total			

Subcatchment 2S: AREA OF LOT 7+, ALONG HUSSEY ROAD



Runoff

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Summary for Subcatchment 3S: CENTRAL SITE PLUS OFF SITE

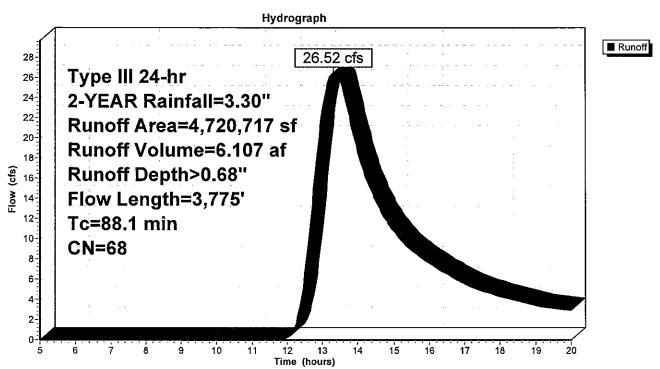
Runoff = 26.52 cfs @ 13.32 hrs, Volume=

6.107 af, Depth> 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

	Α	rea (sf)	CN E	escription		
4	•	29,709 91,008	98 F	Paved road Voods, HS	s, HSG C	
-	4,720,717 68 Weighted Average 4,691,008 99.37% Pervious Area 29,709 0.63% Impervious Area				verage vious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	31.6	150	0.0660	0.08	,	Sheet Flow, SOUTH WEST THROUGH WOODS Woods: Dense underbrush n= 0.800 P2= 3.30"
	52.7	1,580	0.0400	0.50		Shallow Concentrated Flow, WEST THROUGH WOODS Forest w/Heavy Litter Kv= 2.5 fps
_	3.8	2,045	0.0420	9.03	162.47	Trap/Vee/Rect Channel Flow, NORTH WEST IN STREAM Bot.W=5.00' D=2.00' Z= 2.0'/' Top.W=13.00' n= 0.040 Winding stream, pools & shoals
_	88.1	3.775	Total			

Subcatchment 3S: CENTRAL SITE PLUS OFF SITE



Type III 24-hr 2-YEAR Rainfall=3.30"

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Summary for Pond 1P: CULVERT AT HUSSEY ROAD, POA 1

Inflow Area = 5.122 ac, 10.11% Impervious, Inflow Depth > 1.05" for 2-YEAR event

Inflow = 3.81 cfs @ 12.44 hrs, Volume= 0.449 af

Outflow = 3.81 cfs @ 12.44 hrs, Volume= 0.449 af, Atten= 0%, Lag= 0.2 min

Primary = 3.81 cfs @ 12.44 hrs, Volume= 0.449 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 515.34' @ 12.44 hrs Surf.Area= 92 sf Storage= 79 cf

Plug-Flow detention time= 0.9 min calculated for 0.447 af (100% of inflow)

Center-of-Mass det. time= 0.4 min (832.8 - 832.3)

<u>Volume</u>	Invert	Avail.Storage	Storage Description	
#1	514.00'	520 cf	Custom Stage Data (Prismatic)Listed below (Recalc)	_
Elevation	Surf.A	Area Inc	nc.Store Cum.Store	

.Store c-feet)
0
150
520

Device	Routing	Invert	Outlet Devices	
#1	Primary	514.40'	24.0" Round Culvert	

L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 514.40' / 512.10' S= 0.0383 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

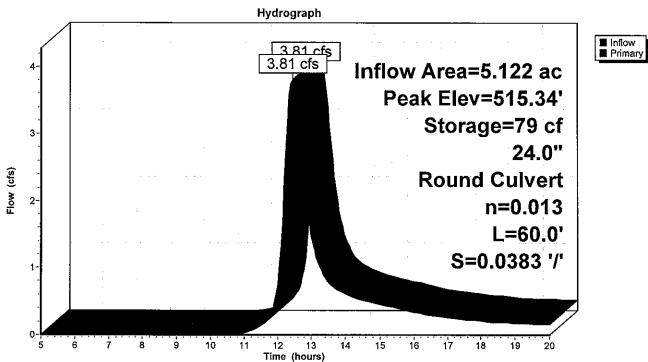
Primary OutFlow Max=3.80 cfs @ 12.44 hrs HW=515.34' (Free Discharge) 1=Culvert (Inlet Controls 3.80 cfs @ 2.61 fps)

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Pond 1P: CULVERT AT HUSSEY ROAD, POA 1





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Summary for Pond 2P: CULVERT AT HUSSEY ROAD 15", POA 2

Inflow Area = 3.226 ac, 1.49% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow = 2.13 cfs @ 12.39 hrs, Volume= 0.240 af

Outflow = 2.13 cfs @ 12.40 hrs, Volume= 0.240 af, Atten= 0%, Lag= 0.7 min

Primary = 2.13 cfs @ 12.40 hrs, Volume= 0.240 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 524.93' @ 12.40 hrs Surf.Area= 155 sf Storage= 84 cf

Plug-Flow detention time= 1.0 min calculated for 0.239 af (100% of inflow)

Center-of-Mass det. time= 0.6 min (836.7 - 836.1)

Volume	Invert	Avail.Storage	Storage	e Description
#1	524.00'	3,549 cf	Custom	n Stage Data (Prismatic)Listed below (Recalc)
Elevation (feet)	Surf.Aı (sq		:.Store c-feet)	Cum.Store (cubic-feet)
524 00		25	ń	0

524.00	25	0	0
526.00	304	329	329
528.00	2,916	3,220	3,549

Device Routing Invert Outlet Devices

#1 Primary 524.10' 15.0" Round Culvert

L= 30.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 524.10' / 523.70' S= 0.0133 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

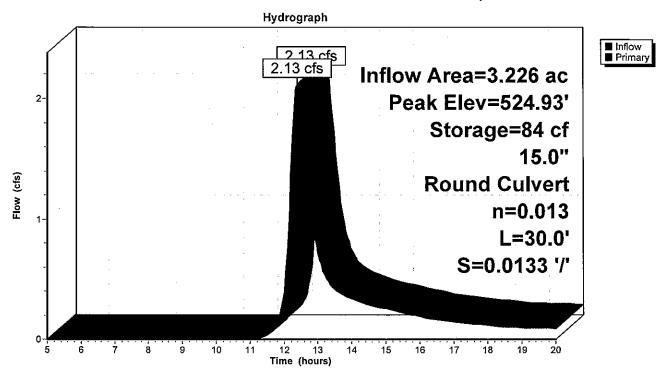
Primary OutFlow Max=2.12 cfs @ 12.40 hrs HW=524.93' (Free Discharge) 1=Culvert (Inlet Controls 2.12 cfs @ 2.45 fps)

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Pond 2P: CULVERT AT HUSSEY ROAD 15", POA 2



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Summary for Pond 3P: CULVERT AT HUSSEY ROAD 32", POA 3

Inflow Area = 108.373 ac, 0.63% Impervious, Inflow Depth > 0.68" for 2-YEAR event

Inflow = 26.52 cfs @ 13.32 hrs, Volume= 6.107 af

Outflow = 25.89 cfs @ 13.47 hrs, Volume= 6.079 af, Atten= 2%, Lag= 9.3 min

Primary = 25.89 cfs @ 13.47 hrs, Volume= 6.079 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 520.72' @ 13.47 hrs Surf.Area= 5,216 sf Storage= 8,332 cf

Plug-Flow detention time= 5.2 min calculated for 6.079 af (100% of inflow)

Center-of-Mass det. time= 3.7 min (895.8 - 892.0)

Volume	Inv	ert Avail.Sto	orage Storage	e Description			
#1	517.	50' 66,9	75 cf Custon	n Stage Data (Pr	ismatic)Listed below (Recalc)		
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
517.5	50	450	0	0			
518.00		925	344	344			
520.0	00	3,810	4,735	5,079			
522.0	00	7,710	11,520	16,599			
524.0	00	12,683	20,393	36,992			
526.0	00	17,300	29,983	66,975			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	517.90'	32.0" Round	d Culvert L= 31.	0' Ke= 0.900		
""			n= 0.010, Flo	ow Area= 5.59 sf	517.30' S= 0.0194'/' Cc= 0.900		
#2 Primary 522.00' ⁻		10.0' long x	10.0' long x 16.0' breadth Broad-Crested Rectangular Weir				

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63

Primary OutFlow Max=25.88 cfs @ 13.47 hrs HW=520.72' (Free Discharge)

1=Culvert (Inlet Controls 25.88 cfs @ 4.63 fps)

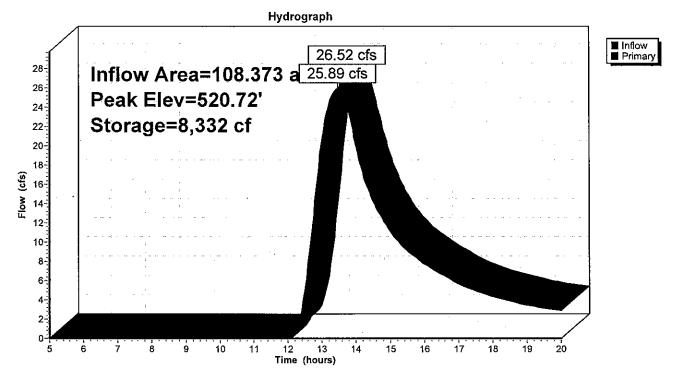
⁻²⁼Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Pond 3P: CULVERT AT HUSSEY ROAD 32", POA 3



Type III 24-hr 10-YEAR Rainfall=4.90"

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Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: SOUTH SIDE ALONG Runoff Area=223,131 sf 10.11% Impervious Runoff Depth>2.18" Flow Length=878' Tc=29.4 min CN=75 Runoff=8.06 cfs 0.929 af

Subcatchment 2S: AREA OF LOT 7+, Runoff Area=140,510 sf 1.49% Impervious Runoff Depth>1.94" Flow Length=875' Tc=25.1 min CN=72 Runoff=4.83 cfs 0.522 af

Subcatchment3S: CENTRAL SITE PLUS Runoff Area=4,720,717 sf 0.63% Impervious Runoff Depth>1.60" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=67.94 cfs 14.425 af

Pond 1P: CULVERT AT HUSSEY ROAD, POA 1 Peak Elev=515.87' Storage=134 cf Inflow=8.06 cfs 0.929 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0383 '/' Outflow=8.06 cfs 0.929 af

Pond 2P: CULVERT AT HUSSEY ROAD 15", Peak Elev=525.78' Storage=265 cf Inflow=4.83 cfs 0.522 af 15.0" Round Culvert n=0.013 L=30.0' S=0.0133 '/' Outflow=4.79 cfs 0.522 af

Pond 3P: CULVERT AT HUSSEY ROAD Peak Elev=523.00' Storage=25,539 cf Inflow=67.94 cfs 14.425 af Outflow=67.48 cfs 14.383 af

Total Runoff Area = 116.721 ac Runoff Volume = 15.877 af Average Runoff Depth = 1.63" 98.93% Pervious = 115.473 ac 1.07% Impervious = 1.248 ac

Type III 24-hr 25-YEAR Rainfall=6.20" Printed 5/21/2018

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: SOUTH SIDE ALONG Runoff Area=223,131 sf 10.11% Impervious Runoff Depth>3.20" Flow Length=878' Tc=29.4 min CN=75 Runoff=11.82 cfs 1.364 af

Subcatchment 2S: AREA OF LOT 7+, Runoff Area=140,510 sf 1.49% Impervious Runoff Depth>2.91" Flow Length=875' Tc=25.1 min CN=72 Runoff=7.28 cfs 0.783 af

Subcatchment3S: CENTRAL SITE PLUS Runoff Area=4,720,717 sf 0.63% Impervious Runoff Depth>2.48" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=107.26 cfs 22.391 af

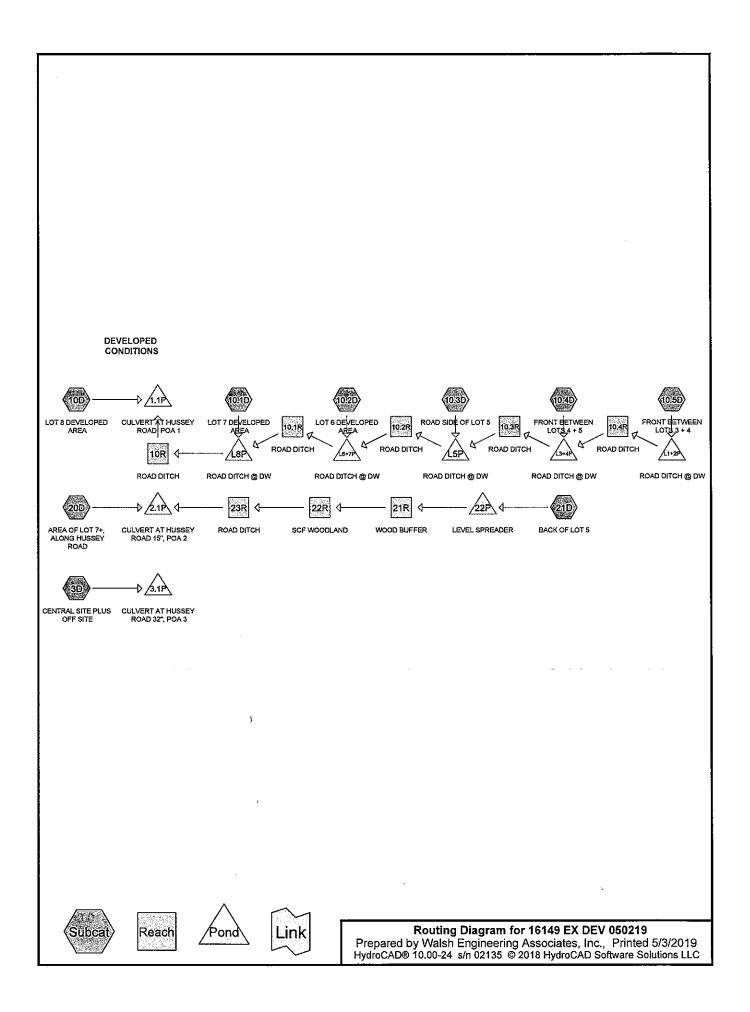
Pond 1P: CULVERT AT HUSSEY ROAD, POA Peak Elev=516.37' Storage=200 cf Inflow=11.82 cfs 1.364 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0383 '/' Outflow=11.81 cfs 1.363 af

Pond 2P: CULVERT AT HUSSEY ROAD 15", Peak Elev=526.76' Storage=942 cf Inflow=7.28 cfs 0.783 af 15.0" Round Culvert n=0.013 L=30.0' S=0.0133 '/' Outflow=6.66 cfs 0.783 af

Pond 3P: CULVERT AT HUSSEY ROAD Peak Elev=523.76' Storage=34,015 cf Inflow=107.26 cfs 22.391 af Outflow=106.57 cfs 22.335 af

Total Runoff Area = 116.721 ac Runoff Volume = 24.538 af Average Runoff Depth = 2.52" 98.93% Pervious = 115.473 ac 1.07% Impervious = 1.248 ac

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3D: CENTRAL SITE PLUS Runoff Area=4,708,093 sf 0.79% Impervious Runoff Depth>0.68" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=26.44 cfs 6.091 af

Subcatchment 10.1D: LOT 7 DEVELOPED Runoff Area=17,600 sf 15.05% Impervious Runoff Depth>1.23" Flow Length=135' Tc=21.4 min CN=78 Runoff=0.41 cfs 0.042 af

Subcatchment 10.2D: LOT 6 DEVELOPED Runoff Area=39,406 sf 13.07% Impervious Runoff Depth>1.18" Flow Length=312' Tc=15.1 min CN=77 Runoff=0.99 cfs 0.089 af

Subcatchment 10.3D: ROAD SIDE OF LOT Runoff Area=36,777 sf 12.09% Impervious Runoff Depth>1.18" Flow Length=223' Tc=15.2 min CN=77 Runoff=0.93 cfs 0.083 af

Subcatchment 10.4D: FRONT BETWEEN Runoff Area=13,940 sf 22.87% Impervious Runoff Depth>1.37" Flow Length=180' Tc=7.8 min CN=80 Runoff=0.51 cfs 0.036 af

Subcatchment 10.5D: FRONT BETWEEN Runoff Area=29,490 sf 17.37% Impervious Runoff Depth>1.17" Flow Length=380' Tc=19.5 min CN=77 Runoff=0.67 cfs 0.066 af

Subcatchment 10D: LOT 8 DEVELOPED Runoff Area=98,423 sf 9.25% Impervious Runoff Depth>1.10"
Flow Length=1,135' Tc=48.8 min CN=76 Runoff=1.38 cfs 0.207 af

Subcatchment 20D: AREA OF LOT 7+, Runoff Area=136,683 sf 1.53% Impervious Runoff Depth>0.95" Flow Length=875' Tc=27.3 min CN=73 Runoff=2.13 cfs 0.247 af

Subcatchment 21D: BACK OF LOT 5 Runoff Area=3,938 sf 22.55% Impervious Runoff Depth>1.30"

Tc=6.0 min CN=79 Runoff=0.15 cfs 0.010 af

Reach 10.1R: ROAD DITCH

Avg. Flow Depth=0.12' Max Vel=2.98 fps Inflow=1.99 cfs 0.110 af n=0.035 L=140.0' S=0.0929 '/' Capacity=381.91 cfs Outflow=1.97 cfs 0.110 af

Reach 10.2R: ROAD DITCH

Avg. Flow Depth=0.13' Max Vel=1.74 fps Inflow=1.27 cfs 0.071 af n=0.035 L=310.0' S=0.0287 '/' Capacity=212.36 cfs Outflow=1.24 cfs 0.071 af

Reach 10.3R: ROAD DITCH

Avg. Flow Depth=0.09' Max Vel=1.41 fps Inflow=0.66 cfs 0.034 af n=0.035 L=286.0' S=0.0311 '/' Capacity=221.09 cfs Outflow=0.65 cfs 0.034 af

Reach 10.4R: ROAD DITCH

Avg. Flow Depth=0.09' Max Vel=1.20 fps Inflow=0.57 cfs 0.026 af n=0.035 L=300.0' S=0.0230 '/' Capacity=190.07 cfs Outflow=0.55 cfs 0.026 af

Reach 10R: ROAD DITCH

Avg. Flow Depth=0.13' Max Vel=3.22 fps Inflow=2.27 cfs 0.120 af n=0.035 L=72.0' S=0.0958 '/' Capacity=334.91 cfs Outflow=2.26 cfs 0.120 af

Reach 21R: WOOD BUFFER

Avg. Flow Depth=0.03' Max Vel=0.07 fps Inflow=0.16 cfs 0.007 af n=0.800 L=50.0' S=0.1200 '/' Capacity=1.35 cfs Outflow=0.06 cfs 0.007 af

Reach 22R: SCF WOODLAND Avg. Flow Depth=0.02' Max Vel=0.30 fps Inflow=0.06 cfs 0.007 af n=0.100 L=596.0' S=0.0671 '/' Capacity=29.12 cfs Outflow=0.03 cfs 0.007 af

16149 EX DEV 050219 Prepared by Walsh Engineering Associates, Inc. HydroCAD® 10.00-24 s/n 02135 © 2018 HydroCAD Software Solutions	Type III 24-hr 2-YEAR Rainfall=3.30" Printed 5/3/2019 s LLC Page 3
	Max Vel=0.79 fps Inflow=0.03 cfs 0.007 af acity=215.22 cfs Outflow=0.03 cfs 0.007 af
Pond 1.1P: CULVERT AT HUSSEY ROAD, POA Peak Elev=515.2 24.0" Round Culvert n=0.013 L=60	25' Storage=70 cf Inflow=3.14 cfs 0.328 af 0.0' S=0.0383 '/' Outflow=3.14 cfs 0.327 af
Pond 2.1P: CULVERT AT HUSSEY ROAD 15", Peak Elev=524.9 15.0" Round Culvert n=0.013 L=30	03' Storage=84 cf Inflow=2.13 cfs 0.254 af 0.0' S=0.0133 '/' Outflow=2.13 cfs 0.254 af
Pond 3.1P: CULVERT AT HUSSEY ROAD Peak Elev=520.71' S	Storage=8,293 cf Inflow=26.44 cfs 6.091 af Outflow=25.83 cfs 6.062 af
Pond 22P: LEVEL SPREADER Peak Elev=574.02	2' Storage=107 cf Inflow=0.15 cfs 0.010 af Outflow=0.16 cfs 0.007 af
	60' Storage=11 cf Inflow=0.67 cfs 0.066 af 57 cfs 0.026 af Outflow=0.67 cfs 0.066 af
	64' Storage=21 cf Inflow=0.76 cfs 0.063 af 66 cfs 0.034 af Outflow=0.76 cfs 0.063 af
	79' Storage=30 cf Inflow=1.37 cfs 0.117 af 27 cfs 0.071 af Outflow=1.37 cfs 0.117 af
	92' Storage=86 cf Inflow=2.09 cfs 0.159 af 99 cfs 0.110 af Outflow=2.09 cfs 0.159 af
	.79' Storage=8 cf Inflow=2.37 cfs 0.152 af 27 cfs 0.120 af Outflow=2.37 cfs 0.152 af
Total Runoff Area = 116.721 ac Runoff Volume = 98.62% Pervious = 118	6.870 af Average Runoff Depth = 0.71" 5.115 ac

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Summary for Subcatchment 3D: CENTRAL SITE PLUS OFF SITE

Runoff = 26.44 cfs @ 13.32 hrs, Volume=

6.091 af, Depth> 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

	Α	rea (sf)	CN	Description					
*		29,709	98	Paved roads, HSG C					
*	4,6	59,503	68	Woods, HS	GC				
*		7,598	98	Roofs & Dri	iveways, HS	SG C			
_		11,283	74	>75% Gras	s cover, Go	ood, HSG C			
	4,708,093 68 Weighted Average								
	4,670,786 99.21% Pervious Area								
		37,307		0.79% Impe	ervious Area	а			
	_								
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)		(cfs)	·			
	31.6	150	0.0660	0.08		Sheet Flow, SOUTH WEST THROUGH WOODS			
						Woods: Dense underbrush n= 0.800 P2= 3.30"			
	52.7	1,580	0.0400	0.50		Shallow Concentrated Flow, WEST THROUGH WOODS			
						Forest w/Heavy Litter Kv= 2.5 fps			
	3.8	2,045	0.0420	9.03	162.47	Trap/Vee/Rect Channel Flow, NORTH WEST IN STREAM			
						Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'			
_						n= 0.040 Winding stream, pools & shoals			
	88.1	3,775	Total						

Summary for Subcatchment 10.1D: LOT 7 DEVELOPED AREA

Runoff = 0.41 cfs @ 12.31 hrs, Volume=

CM Description

Aron (of)

0.042 af, Depth> 1.23"

	P	\rea (st)	CN	<u>Description</u>							
*		1,761	98	Existing Paved roads, HSG C							
		1,382	96	Gravel surfa	Gravel surface, HSG C						
*		10,480	72	Woods, HS	GC						
*		888	98	Roofs, HSG	Roofs, HSG C						
		3,089	74	75% Grass cover, Good, HSG C							
		17,600	78								
		14,951		84.95% Pei	rvious Area						
		2,649		15.05% Imp	pervious Ar	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
(<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	20.8	85	0.0150	0.07		Sheet Flow, WEST IN WOODS					
						Woods: Light underbrush n= 0.400 P2= 3.30"					
	0.6	50	0.0100	1.50		Shallow Concentrated Flow, DW DITCH					
						Grassed Waterway Kv= 15.0 fps					
	21.4	135	Total			<u> </u>					

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Summary for Subcatchment 10.2D: LOT 6 DEVELOPED AREA

Runoff = 0.99 cfs @ 12.22 hrs, Volume=

0.089 af, Depth> 1.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

_	Α	rea (sf)	CN [Description					
*		3,661	98 E	Existing Paved roads, HSG C					
		2,156			ace, HSG C				
*		27,701		Woods, HSG C					
*		1,488	98 F	Roofs, HSG C					
		4,400	74 >	>75% Grass cover, Good, HSG C					
		39,406	77 V	Weighted Average					
		34,257		86.93% Pervious Area					
		5,149	1	3.07% lmp	ervious Ar	ea			
		,		•					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	12.1	100	0.0800	0.14		Sheet Flow, IN WOODS			
						Woods: Light underbrush n= 0.400 P2= 3.30"			
	3.0	212	0.0280	1.17		Shallow Concentrated Flow, ROAD DITCH			
_						Short Grass Pasture Kv= 7.0 fps			
	15.1	312	Total						

Summary for Subcatchment 10.3D: ROAD SIDE OF LOT 5

Runoff = 0.93 cfs @ 12.22 hrs, Volume=

0.083 af, Depth> 1.18"

	. A	rea (sf)	CN E	Description					
*		3,269	98 E	Existing Paved roads, HSG C					
		1,948	96 (Gravel surface, HSG C					
*		24,846	72 V	Woods, HSG C					
*		1,176	98 F	Roofs, HSG C					
		5,538	74 >	>75% Grass cover, Good, HSG C					
		36,777	77 V	77 Weighted Average					
		32,332	8	7.91% Per	vious Area				
		4,445	1	2.09% Imp	ervious Are	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	13.6	100	0.0600	0.12		Sheet Flow, WEST IN WOODS			
						Woods: Light underbrush n= 0.400 P2= 3.30"			
	1.6	123	0.0325	1.26		Shallow Concentrated Flow, ROAD DITCH			
_						Short Grass Pasture Kv= 7.0 fps			
	15.2	223	Total						

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Summary for Subcatchment 10.4D: FRONT BETWEEN LOTS 4 + 5

Runoff = 0.51 cfs @ 12.12 hrs, Volume=

0.036 af, Depth> 1.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

_	Α	rea (sf)	CN E	Description						
*		3,188	98 E	98 Existing Paved road, HSG C						
		883	96 (Gravel surfa	ace, HSG C	;				
*		8,832	72 \	Voods, HS	GĊ					
_		1,037	74 >	>75% Grass cover, Good, HSG C						
		13,940	80 V	80 Weighted Average						
		10,752	7	7.13% Per	vious Area					
		3,188	2	22.87% Imp	pervious Ar	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	4.6	30	0.0800	0.11		Sheet Flow, WEST IN WOODS				
						Woods: Light underbrush n= 0.400 P2= 3.30"				
	3.2	150	0.0240	0.77		Shallow Concentrated Flow, IN WOODS				
_						Woodland Kv= 5.0 fps				
	7.8	180	Total							

Summary for Subcatchment 10.5D: FRONT BETWEEN LOTS 3 + 4

Runoff =

0.67 cfs @ 12.29 hrs, Volume=

0.066 af, Depth> 1.17"

	A	\rea (sf)	CN I	Description							
*		5,121	98 Paved road, HSG C								
*		22,925	72	· · · · · · · · · · · · · · · · · · ·							
		611	96	Gravel surfa							
		833	74 :	>75% Gras	s cover, Go	ood, HSG C					
		29,490	۲7 ۱	77 Weighted Average							
		24,369	;	32.63% Pei	rviouš Area						
		5,121	17.37% Impervious Area								
				•							
	Тс	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	16.9	100	0.0350	0.10		Sheet Flow, WEST IN WOODS					
						Woods: Light underbrush n= 0.400 P2= 3.30"					
	2.6	280	0.0323	1.80		Shallow Concentrated Flow, TO ROAD DITCH					
			Nearly Bare & Untilled Kv= 10.0 fps								
_	19.5	380	Total								

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Summary for Subcatchment 10D: LOT 8 DEVELOPED AREA

Runoff =

1.38 cfs @ 12.71 hrs, Volume=

0.207 af, Depth> 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

			ON F							
_		\rea (sf)	CN Description							
*		5,557	98 Existing Paved roads, HSG C							
		2,265	96 0	, ,						
*		59,479		·						
*		3,552		· ·						
		27,570		•		ood, HSG C				
_						7,000,11000				
		98,423		Veighted A						
	89,314 90.75% Pervious Area									
		9,109	9).25% Impe	ervious Area	a				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•				
	31.6	150	0.0660	0.08	-	Sheet Flow, WEST IN WOODS				
						Woods: Dense underbrush n= 0.800 P2= 3.30"				
	13.1	755	0.0370	0.96		Shallow Concentrated Flow, IN WOODS				
	١٠,١	700	0.0070	0.30		Woodland Kv= 5.0 fps				
	4 4	222	0.0050	0.04						
	4.1	230	0.0350	0.94		Shallow Concentrated Flow, ROAD DITCH				
						Woodland Kv= 5.0 fps				

Summary for Subcatchment 20D: AREA OF LOT 7+, ALONG HUSSEY ROAD

Runoff

=

Area (sf)

CN

2.13 cfs @ 12.42 hrs, Volume=

Description

0.247 af, Depth> 0.95"

	(/ _ / _				
*	2,095	98	Paved road	, HSG C	
*	124,158	72	Woods, HS	GC	
	10,430	74	>75% Ġras	s cover, Go	ood, HSG C
	136,683 134,588 2,095		Weighted A 98.47% Per 1.53% Impe	rvious Area	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
15.4	100	0.0440	0.11		Sheet Flow, NORTH IN WOODS Woods: Light underbrush n= 0.400 P2= 3.30"
8.8	625	0.0560	1.18		Shallow Concentrated Flow, NORTH WEST THROUGH WO Woodland Kv= 5.0 fps
3.1	150	0.0130	0.80		Shallow Concentrated Flow, SOUTH WEST IN ROAD DITCH Short Grass Pasture Kv= 7.0 fps
27.3	875	Total			

Type III 24-hr 2-YEAR Rainfall=3.30"

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Summary for Subcatchment 21D: BACK OF LOT 5

Runoff = 0.15 cfs @ 12.10 hrs, Volume=

0.010 af, Depth> 1.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.30"

_	Α	rea (sf)	CN	Description							
4	•	888	98	Roofs & Driveways, HSG C							
_		3,050			% Grass cover, Good, HSG C						
3,938 79 Weighted Average											
		3,050		77.45% Pervious Area							
		888		22.55% lmp							
	Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	Description					
-		(leet)	(IVIL) (WSec)	(CIS)		t				
	6.0					Direct Entry.					

Summary for Reach 10.1R: ROAD DITCH

Inflow Area = 2.746 ac, 14.97% Impervious, Inflow Depth > 0.48" for 2-YEAR event

Inflow = 1.99 cfs @ 12.32 hrs, Volume= 0.110 af

Outflow = 1.97 cfs @ 12.35 hrs, Volume= 0.110 af, Atten= 1%, Lag= 1.7 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.98 fps, Min. Travel Time= 0.8 min Avg. Velocity = 1.11 fps, Avg. Travel Time= 2.1 min

Peak Storage= 93 cf @ 12.33 hrs
Average Depth at Peak Storage= 0.12'

Bank-Full Depth= 2.00' Flow Area= 26.0 sf, Capacity= 381.91 cfs

5.00' x 2.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 4.0 '/' Top Width= 21.00'

Length= 140.0' Slope= 0.0929 '/'

Inlet Invert= 557.00'. Outlet Invert= 544.00'



Summary for Reach 10.2R: ROAD DITCH

Inflow Area = 1.841 ac, 15.90% Impervious, Inflow Depth > 0.46" for 2-YEAR event

Inflow = 1.27 cfs @ 12.26 hrs, Volume= 0.071 af

Outflow = 1.24 cfs @ 12.35 hrs, Volume= 0.071 af, Atten= 2%, Lag= 5.8 min

Type III 24-hr 2-YEAR Rainfall≈3.30"

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Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.74 fps, Min. Travel Time= 3.0 min

Avg. Velocity = 0.63 fps, Avg. Travel Time= 8.3 min

Peak Storage= 222 cf @ 12.30 hrs

Average Depth at Peak Storage= 0.13'
Bank-Full Depth= 2.00' Flow Area= 26.0 sf, Capacity= 212.36 cfs

5.00' x 2.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 4.0 '/' Top Width= 21.00'

Length= 310.0' Slope= 0.0287 '/'

Inlet Invert= 566.90', Outlet Invert= 558.00'



Summary for Reach 10.3R: ROAD DITCH

Inflow Area = 0.997 ac, 19.13% Impervious, Inflow Depth > 0.41" for 2-YEAR event

Inflow = 0.66 cfs @ 12.39 hrs, Volume= 0.034 af

Outflow = 0.65 cfs @ 12.48 hrs, Volume= 0.034 af, Atten= 2%, Lag= 5.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.41 fps, Min. Travel Time= 3.4 min

Avg. Velocity = 0.60 fps, Avg. Travel Time= 7.9 min

Peak Storage= 132 cf @ 12.42 hrs

Average Depth at Peak Storage= 0.09'

Bank-Full Depth= 2.00' Flow Area= 26.0 sf, Capacity= 221.09 cfs

5.00' x 2.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 4.0 '/' Top Width= 21.00'

Length= 286.0' Slope= 0.0311 '/'

Inlet Invert= 576.90'. Outlet Invert= 568.00'



Type III 24-hr 2-YEAR Rainfall=3.30"

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Summary for Reach 10.4R: ROAD DITCH

Inflow Area = 0.677 ac, 17.37% Impervious, Inflow Depth > 0.47" for 2-YEAR event

Inflow = 0.57 cfs @ 12.29 hrs, Volume= 0.026 af

Outflow = 0.55 cfs @ 12.41 hrs, Volume= 0.026 af, Atten= 4%, Lag= 7.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.20 fps, Min. Travel Time= 4.2 min Avg. Velocity = 0.52 fps, Avg. Travel Time= 9.7 min

Peak Storage= 137 cf @ 12.34 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 2.00' Flow Area= 26.0 sf, Capacity= 190.07 cfs

5.00' x 2.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 4.0 '/' Top Width= 21.00'

Length= 300.0' Slope= 0.0230 '/'

#

Inlet Invert= 584.90', Outlet Invert= 578.00'



Summary for Reach 10R: ROAD DITCH

Inflow Area = 3.150 ac, 14.98% Impervious, Inflow Depth > 0.46" for 2-YEAR event

Inflow = 2.27 cfs @ 12.34 hrs, Volume= 0.120 af

Outflow = 2.26 cfs @ 12.36 hrs, Volume= 0.120 af, Atten= 0%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity = 3.22 fps, Min. Travel Time = 0.4 min Avg. Velocity = 1.14 fps, Avg. Travel Time = 1.1 min

Peak Storage= 51 cf @ 12.35 hrs

Average Depth at Peak Storage= 0.13'

Bank-Full Depth= 2.00' Flow Area= 22.0 sf, Capacity= 334.91 cfs

5.00' x 2.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 3.0 '/' Top Width= 17.00'

Length= 72.0' Slope= 0.0958 '/'

Inlet Invert= 524.90', Outlet Invert= 518.00'



Type III 24-hr 2-YEAR Rainfall=3.30"

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Summary for Reach 21R: WOOD BUFFER

Inflow Area =

0.090 ac, 22.55% Impervious, Inflow Depth > 0.98" for 2-YEAR event

Inflow =

0.16 cfs @ 12.15 hrs, Volume=

0.007 af

Outflow =

0.06 cfs @ 12.55 hrs, Volume=

0.007 af, Atten= 61%, Lag= 24.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.07 fps, Min. Travel Time= 12.7 min

Avg. Velocity = 0.03 fps, Avg. Travel Time= 28.3 min

Peak Storage= 50 cf @ 12.32 hrs

Average Depth at Peak Storage= 0.03'

Bank-Full Depth= 0.20' Flow Area= 6.4 sf, Capacity= 1.35 cfs

30.00' x 0.20' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Side Slope Z-value= 10.0 '/' Top Width= 34.00'

Length= 50.0' Slope= 0.1200 '/'

Inlet Invert= 574.00', Outlet Invert= 568.00'

Summary for Reach 22R: SCF WOODLAND

Inflow Area =

Outflow

‡

‡

0.090 ac, 22.55% Impervious, Inflow Depth > 0.95" for 2-YEAR event

Inflow =

0.06 cfs @ 12.55 hrs, Volume= 0.03 cfs @ 13.46 hrs, Volume=

0.007 af 0.007 af, Atten= 48%, Lag= 54.7 min

Routing by Stor-Ind+Trans method. Time Span= 5.00-20.00 hrs. dt= 0.05 hrs

Max. Velocity= 0.30 fps, Min. Travel Time= 32.9 min

Avg. Velocity = 0.20 fps, Avg. Travel Time= 49.5 min

Peak Storage= 67 cf @ 12.91 hrs

Average Depth at Peak Storage= 0.02'

Bank-Full Depth= 1.00' Flow Area= 10.0 sf, Capacity= 29.12 cfs

5.00' x 1.00' deep channel, n= 0.100 Heavy timber, flow below branches

Side Slope Z-value= 5.0 '/' Top Width= 15.00'

Length= 596.0' Slope= 0.0671 '/'

Inlet Invert= 568.00', Outlet Invert= 528.00'

Type III 24-hr 2-YEAR Rainfall=3.30"

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Summary for Reach 23R: ROAD DITCH

Inflow Area =

0.090 ac, 22.55% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow

0.03 cfs @ 13.46 hrs. Volume=

0.007 af

Outflow

0.03 cfs @ 13.57 hrs, Volume=

0.007 af, Atten= 1%, Lag= 6.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.79 fps, Min. Travel Time= 3.2 min

Avg. Velocity = 0.79 fps, Avg. Travel Time= 3.2 min

Peak Storage= 6 cf @ 13.52 hrs

Average Depth at Peak Storage= 0.01'

Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 215.22 cfs

3.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 15.00'

Length= 150.0' Slope= 0.0260 '/'

Inlet Invert= 528.00', Outlet Invert= 524.10'



Summary for Pond 1.1P: CULVERT AT HUSSEY ROAD, POA 1

Inflow Area =

5.409 ac, 12.59% Impervious, Inflow Depth > 0.73" for 2-YEAR event

Inflow

3.14 cfs @ 12.42 hrs, Volume=

Outflow

3.14 cfs @ 12.42 hrs, Volume= 0.327 af, Atten= 0%, Lag= 0.2 min

0.328 af

3.14 cfs @ 12.42 hrs, Volume=

0.327 af

Primary

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 515.25' @ 12.42 hrs Surf.Area= 87 sf Storage= 70 cf

Plug-Flow detention time= 1.0 min calculated for 0.326 af (100% of inflow)

Center-of-Mass det. time= 0.5 min (811.7 - 811.2)

Volume	Invert	Avail.Storage	Storage	Description	
#1	514.00'	520 cf	Custom	Stage Data (Pri	smatic)Listed below (Recalc)
Elevation (feet)	Surf.A (sc		:.Store c-feet)	Cum.Store (cubic-feet)	
514.00 516.00 518.00		25 125 245	0 150 370	0 150 520	

Device Routing Invert Outlet Devices

514.40'

#1 Primary 24.0" Round Culvert

L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 514.40' / 512.10' S= 0.0383 '/' Cc= 0.900

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n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.13 cfs @ 12.42 hrs HW=515.25' (Free Discharge) 1=Culvert (Inlet Controls 3.13 cfs @ 2.47 fps)

Summary for Pond 2.1P: CULVERT AT HUSSEY ROAD 15", POA 2

Inflow Area = 3.228 ac, 2.12% Impervious, Inflow Depth > 0.94" for 2-YEAR event

Inflow = 2.13 cfs @ 12.42 hrs, Volume= 0.254 af

Outflow = 2.13 cfs @ 12.43 hrs, Volume= 0.254 af, Atten= 0%, Lag= 0.6 min

Primary = 2.13 cfs @ 12.43 hrs, Volume= 0.254 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 524.93' @ 12.43 hrs Surf.Area= 155 sf Storage= 84 cf

Plug-Flow detention time= 1.0 min calculated for 0.253 af (100% of inflow)

Center-of-Mass det. time= 0.6 min (838.4 - 837.7)

Volume	Invert	Avail.Storage	Storage Description
#1	524.00'	3,549 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Surf.Area (sg-ft)	Inc.Store (cubic-feet)	(cubic-feet)
25	0	0
304	329	329
2,916	3,220	3,549
	(sq-ft) 25 304	(sq-ft) (cubic-feet) 25 0 304 329

Device Routing Invert Outlet Devices

#1 Primary 524.10' 15.0" Round Culvert

L= 30.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 524.10' / 523.70' S= 0.0133 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.12 cfs @ 12.43 hrs HW=524.93' (Free Discharge)
1=Culvert (Inlet Controls 2.12 cfs @ 2.45 fps)

Summary for Pond 3.1P: CULVERT AT HUSSEY ROAD 32", POA 3

Inflow Area = 108.083 ac, 0.79% Impervious, Inflow Depth > 0.68" for 2-YEAR event

Inflow = 26.44 cfs @ 13.32 hrs, Volume= 6.091 af

Outflow = 25.83 cfs @ 13.47 hrs, Volume= 6.062 af, Atten= 2%, Lag= 9.3 min

Primary = 25.83 cfs @ 13.47 hrs, Volume= 6.062 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 520.71' @ 13.47 hrs Surf.Area= 5,201 sf Storage= 8,293 cf

Plug-Flow detention time= 5.2 min calculated for 6.062 af (100% of inflow)

Center-of-Mass det. time= 3.7 min (895.8 - 892.0)

Type III 24-hr 2-YEAR Rainfall=3.30"

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Volume	lnv	ert Avail.Sto	rage Storage D	Description	
#1	517.	50' 66,9	75 cf Custom	Stage Data (Pr	rismatic)Listed below (Recalc)
Elevation		Surf.Area	Inc.Store	Cum.Store	
(fee	<u>≥t)</u>	(sq-ft)	(cubic-feet)	(cubic-feet)	
517.5	_	450	0	0	
518.0	00	925	344	344	
520.0	00	3,810	4,735	5,079	
522.0	00	7,710	11,520	16,599	
524.0	00	12,683	20,393	36,992	
526.0	00	17,300	29,983	66,975	
Device	Routing	Invert	Outlet Devices		
#1 #2	Primary Primary	517.90' 522.00'	n= 0.010, Flow 10.0' long x 1 Head (feet) 0.3	vert= 517.90' / v Area= 5.59 sf 6.0' breadth B 20 0.40 0.60	517.30' S= 0.0194 '/' Cc= 0.900

Primary OutFlow Max=25.81 cfs @ 13.47 hrs HW=520.71' (Free Discharge)

-1=Culvert (Inlet Controls 25.81 cfs @ 4.62 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 22P: LEVEL SPREADER

Inflow Area = 0.090 ac, 22.55% Impervious, Inflow Depth > 1.30" for 2-YEAR event

Inflow = 0.15 cfs @ 12.10 hrs, Volume= 0.010 af

Outflow = 0.16 cfs @ 12.15 hrs, Volume= 0.007 af, Atten= 0%, Lag= 3.0 min

Primary = 0.16 cfs @ 12.15 hrs, Volume= 0.007 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 574.02' @ 12.15 hrs Surf.Area= 151 sf Storage= 107 cf

Plug-Flow detention time= 94.1 min calculated for 0.007 af (75% of inflow)

Center-of-Mass det. time= 32.7 min (837.7 - 804.9)

<u>Volume</u>	Inv	<u>ert Avail.Sto</u>	<u>rage Storage</u> D	escription	
#1	573.0	00' 2	90 cf Custom S	Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation (feet		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
573.00 574.00 575.00	0	60 150 220	0 105 185	0 105 290	
Device	Routing	Invert	Outlet Devices		
#1	Primary	574.00'	Head (feet) 0.2 2.50 3.00 3.50	20 0.40 0.60) 4.00 4.50 5	0.80 1.00 1.20 1.40 1.60 1.80 2.00 .00 5.50 69 2.68 2.67 2.67 2.65 2.66 2.66

Volume

Invert

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2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Primary OutFlow Max=0.14 cfs @ 12.15 hrs HW=574.02' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 0.14 cfs @ 0.30 fps)

Summary for Pond L1+2P: ROAD DITCH @ DW

Inflow Area = 0.677 ac, 17.37% Impervious, Inflow Depth > 1.17" for 2-YEAR event
Inflow = 0.67 cfs @ 12.29 hrs, Volume= 0.066 af
Outflow = 0.67 cfs @ 12.29 hrs, Volume= 0.066 af, Atten= 0%, Lag= 0.3 min
Discarded = 0.57 cfs @ 12.29 hrs, Volume= 0.040 af
Primary = 0.57 cfs @ 12.29 hrs, Volume= 0.026 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 585.50' @ 12.29 hrs Surf.Area= 32 sf Storage= 11 cf

Plug-Flow detention time= 0.2 min calculated for 0.066 af (100% of inflow) Center-of-Mass det. time= 0.2 min (820.3 - 820.1)

Avail.Storage Storage Description

#1	585.00'	488 cf Custom	Stage Data (Pris	matic)Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
585.00	10	0	0	
586.00	55	33	33	
588.00	400	455	488	

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	585.00'	12.0" Round Culvert
			L= 30.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 585.00' / 584.90' S= 0.0033 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	587.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#3	Discarded	585.00'	0.10 cfs Exfiltration at all elevations

Discarded OutFlow Max=0.10 cfs @ 11.85 hrs HW=585.04' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.57 cfs @ 12.29 hrs HW=585.50' (Free Discharge)

1=Culvert (Barrel Controls 0.57 cfs @ 2.14 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond L3+4P: ROAD DITCH @ DW

Invert

Volume

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Inflow Area = 0.997 ac, 19.13% Impervious, Inflow Depth > 0.75" for 2-YEAR event
Inflow = 0.76 cfs @ 12.38 hrs, Volume= 0.063 af
Outflow = 0.76 cfs @ 12.39 hrs, Volume= 0.063 af, Atten= 0%, Lag= 0.4 min
Discarded = 0.10 cfs @ 11.80 hrs, Volume= 0.029 af
Primary = 0.66 cfs @ 12.39 hrs, Volume= 0.034 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 577.54' @ 12.39 hrs Surf.Area= 47 sf Storage= 21 cf

Plug-Flow detention time= 0.4 min calculated for 0.063 af (100% of inflow) Center-of-Mass det. time= 0.4 min (784.8 - 784.4)

Avail.Storage Storage Description

VOIGITIC	7110	Cit /waii.Otc	nage Clorage	2 Description		
#1	577.0	00' 3	07 cf Custor	n Stage Data (P	rismatic)Listed below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
577.0		30	0	0		
578.0	30	61	46	46		
580.0	00 .	100	161	207		
580.5	50	300	100	307		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	577.00'	12.0" Roun	d Culvert		
	•		Inlet / Outlet	Invert= 577.00' /	headwall, Ke= 0.900 576.90' S= 0.0033 '/' Cc= 0.900 ooth interior, Flow Area= 0.79 sf	
#2	Primary	580.00'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64			
#3	Discarde	ed 577.00'		iltration at all ele		

Discarded OutFlow Max=0.10 cfs @ 11.80 hrs HW=577.04' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.66 cfs @ 12.39 hrs HW=577.54' (Free Discharge)

-1=Culvert (Barrel Controls 0.66 cfs @ 2.22 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond L5P: ROAD DITCH @ DW

Inflow Area =	1.841 ac, 1	5.90% Impervious, Int	flow Depth > 0.76"	for 2-YEAR event
Inflow =	1.37 cfs @	12.25 hrs, Volume=	0.117 af	
Outflow =	1.37 cfs @	12.26 hrs, Volume=	0.117 af, At	ten= 0%, Lag= 0.3 min
Discarded =	0.10 cfs @	11.75 hrs, Volume=	0.046 af	
Primary =	1.27 cfs @	12.26 hrs. Volume=	0.071 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 567.79' @ 12.26 hrs Surf.Area= 46 sf Storage= 30 cf

Plug-Flow detention time= 0.3 min calculated for 0.117 af (100% of inflow) Center-of-Mass det. time= 0.3 min (798.9 - 798.5)

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Volume	Inve	ert Avail.Sto	rage	Storage D	Description	w
#1	567.0	00' 3	61 cf	Custom 9	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
567.0	00	30	·	0	0	
568.0	00	50		40	40	
570.0	00	137		187	227	
570.5	50	400		134	361	
Device	Routing	Invert	Outle	et Devices		
#1	Primary	567.00'	12.0	" Round (Culvert	
	·		Inlet n= 0	/ Outlet In	vert= 567.00' / ugated PE, sm	headwall, Ke= 0.900 566.90' S= 0.0033 '/' Cc= 0.900 booth interior, Flow Area= 0.79 sf
#2	Primary	570.00'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64			
#3	Discarde	ed 567.00'			ration at all el	

Discarded OutFlow Max=0.10 cfs @ 11.75 hrs HW=567.04' (Free Discharge) 3=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=1.27 cfs @ 12.26 hrs HW=567.78' (Free Discharge)

1=Culvert (Barrel Controls 1.27 cfs @ 2.64 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond L6+7P: ROAD DITCH @ DW

Inflow Area =	2.746 ac, 14.97% Impervious, Inflo	w Depth > 0.70" for 2-YEAR event
Inflow ≔	2.09 cfs @ 12.31 hrs, Volume=	0.159 af
Outflow =	2.09 cfs @ 12.32 hrs, Volume=	0.159 af, Atten= 0%, Lag= 0.7 min
Discarded =	0.10 cfs @ 11.75 hrs, Volume=	0.049 af
Primary =	1.99 cfs @ 12.32 hrs. Volume=	0.110 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 558.92' @ 12.32 hrs Surf.Area= 162 sf Storage= 86 cf

Plug-Flow detention time= 0.6 min calculated for 0.159 af (100% of inflow) Center-of-Mass det. time= 0.5 min (790.6 - 790.1)

<u>Volume</u>	Invert	Avail.Storage	Storage Des	cription
#1	558.00'	681 cf	Custom Sta	ge Data (Prismatic)Listed below (Recalc)
Claustian	Ct V	raa laa	Chara	Com Store

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
558.00	25	0	0
560.00	325	350	350
560.50	1,000	331	681

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Device	Routing	Invert	Outlet Devices
#1	Primary	558.00'	15.0" Round Culvert
			L= 30.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 558.00' / 557.90' S= 0.0033 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Primary	560.00'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	Ť		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#3	Discarded	558.00'	0.10 cfs Exfiltration at all elevations

Discarded OutFlow Max=0.10 cfs @ 11.75 hrs HW=558.05' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=1.97 cfs @ 12.32 hrs HW=558.91' (Free Discharge)

1=Culvert (Barrel Controls 1.97 cfs @ 2.87 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond L8P: ROAD DITCH @ DW

Inflow Area	=	3.150 ac, 1	14.98% Impe	ervious, I	Inflow E	Depth >	0.58"	for 2-Y	EAR event	t
Inflow :	=	2.37 cfs @	12.34 hrs,	Volume=	=	0.152	af			
Outflow :		2.37 cfs @	12.34 hrs,	Volume=	=	0.152	af, Att	ten= 0%,	Lag= 0.1 r	nin
Discarded :	=	0.10 cfs @	11.90 hrs,	Volume=	=	0.032	af		•	
Primary :	=	2.27 cfs @	12.34 hrs,	Volume=	=	0.120	af			

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 544.79' @ 12.34 hrs Surf.Area= 15 sf Storage= 8 cf

Plug-Flow detention time= 0.1 min calculated for 0.152 af (100% of inflow) Center-of-Mass det. time= 0.1 min (771.8 - 771.7)

Volume	Invert	Avail	Storage	Storage	e Description	
#1	544.00'		690 cf	Custon	n Stage Data (Pri	ismatic)Listed below (Recalc)
Elevation (feet)		.Area sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
544.00		5		0	0	
546.00		30		35	35	
548.00		300		330	365	
548.50		1,000		325	690	

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	544.00'	18.0" Round Culvert
	•		L= 30.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 544.00' / 543.50' S= 0.0167 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Primary	548.00'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#3	Discarded	544.00'	0.10 cfs Exfiltration at all elevations

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Discarded OutFlow Max=0.10 cfs @ 11.90 hrs HW=544.11' (Free Discharge) 3=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=2.26 cfs @ 12.34 hrs HW=544.79' (Free Discharge)

1=Culvert (Inlet Controls 2.26 cfs @ 2.39 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment3D: CENTRAL SITE PLUS Runoff Area=4,708,093 sf 0.79% Impervious Runoff Depth>1.60" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=67.76 cfs 14.387 af

Subcatchment 10.1D: LOT 7 DEVELOPED Runoff Area=17,600 sf 15.05% Impervious Runoff Depth>2.43"
Flow Length=135' Tc=21.4 min CN=78 Runoff=0.81 cfs 0.082 af

Subcatchment 10.2D: LOT 6 DEVELOPED Runoff Area=39,406 sf 13.07% Impervious Runoff Depth>2.35" Flow Length=312' Tc=15.1 min CN=77 Runoff=2.02 cfs 0.177 af

Subcatchment 10.3D: ROAD SIDE OF LOT Runoff Area=36,777 sf 12.09% Impervious Runoff Depth>2.35" Flow Length=223' Tc=15.2 min CN=77 Runoff=1.88 cfs 0.166 af

Subcatchment 10.4D: FRONT BETWEEN Runoff Area=13,940 sf 22.87% Impervious Runoff Depth>2.62" Flow Length=180' Tc=7.8 min CN=80 Runoff=0.98 cfs 0.070 af

Subcatchment 10.5D: FRONT BETWEEN Runoff Area=29,490 sf 17.37% Impervious Runoff Depth>2.35" Flow Length=380' Tc=19.5 min CN=77 Runoff=1.37 cfs 0.133 af

Subcatchment 10D: LOT 8 DEVELOPED Runoff Area=98,423 sf 9.25% Impervious Runoff Depth>2.24" Flow Length=1,135' Tc=48.8 min CN=76 Runoff=2.87 cfs 0.422 af

Subcatchment 20D: AREA OF LOT 7+, Runoff Area=136,683 sf 1.53% Impervious Runoff Depth>2.02" Flow Length=875' Tc=27.3 min CN=73 Runoff=4.72 cfs 0.528 af

Subcatchment 21D: BACK OF LOT 5 Runoff Area=3,938 sf 22.55% Impervious Runoff Depth>2.53"

Tc=6.0 min CN=79 Runoff=0.28 cfs 0.019 af

Reach 10.1R: ROAD DITCH

Avg. Flow Depth=0.20' Max Vel=4.04 fps Inflow=4.73 cfs 0.301 af n=0.035 L=140.0' S=0.0929'/ Capacity=381.91 cfs Outflow=4.66 cfs 0.301 af

Reach 10.2R: ROAD DITCH

Avg. Flow Depth=0.22' Max Vei=2.37 fps Inflow=3.08 cfs 0.194 af n=0.035 L=310.0' S=0.0287 '/' Capacity=212.36 cfs Outflow=3.06 cfs 0.194 af

Reach 10.3R: ROAD DITCH

Avg. Flow Depth=0.15' Max Vel=1.93 fps Inflow=1.58 cfs 0.095 af n=0.035 L=286.0' S=0.0311 '/' Capacity=221.09 cfs Outflow=1.56 cfs 0.095 af

Reach 10.4R: ROAD DITCH

Avg. Flow Depth=0.14' Max Vel=1.61 fps Inflow=1.26 cfs 0.072 af

n=0.035 L=300.0' S=0.0230 '/' Capacity=190.07 cfs Outflow=1.24 cfs 0.072 af

Reach 10R: ROAD DITCH

Avg. Flow Depth=0.22' Max Vel=4.37 fps Inflow=5.36 cfs 0.331 af n=0.035 L=72.0' S=0.0958 '/' Capacity=334.91 cfs Outflow=5.35 cfs 0.331 af

Reach 21R: WOOD BUFFER

Avg. Flow Depth=0.07' Max Vel=0.11 fps Inflow=0.29 cfs 0.017 af n=0.800 L=50.0' S=0.1200 '/' Capacity=1.35 cfs Outflow=0.22 cfs 0.016 af

Reach 22R: SCF WOODLAND Avg. Flow Depth=0.05' Max Vel=0.50 fps Inflow=0.22 cfs 0.016 af n=0.100 L=596.0' S=0.0671 '/' Capacity=29.12 cfs Outflow=0.13 cfs 0.016 af

16149 EX DEV 050219Type III 24-hr 10-YEAR Rainfall≈4.Prepared by Walsh Engineering Associates, Inc.Printed 5/3/2HydroCAD® 10.00-24 s/n 02135 © 2018 HydroCAD Software Solutions LLCPage 10.00-2	
Reach 23R: ROAD DITCH Avg. Flow Depth=0.04' Max Vel=1.18 fps Inflow=0.13 cfs 0.01 n=0.022 L=150.0' S=0.0260'/' Capacity=215.22 cfs Outflow=0.13 cfs 0.01	
Pond 1.1P: CULVERT AT HUSSEY ROAD, Peak Elev=515.77' Storage=122 cf Inflow=7.18 cfs 0.75 24.0" Round Culvert n=0.013 L=60.0' S=0.0383 '/' Outflow=7.19 cfs 0.75	
Pond 2.1P: CULVERT AT HUSSEY ROAD 15", Peak Elev=525.73' Storage=253 cf Inflow=4.72 cfs 0.54 15.0" Round Culvert n=0.013 L=30.0' S=0.0133 '/ Outflow=4.68 cfs 0.54	l4 af l3 af
Pond 3.1P: CULVERT AT HUSSEY ROAD Peak Elev=522.99' Storage=25,498 cf Inflow=67.76 cfs 14.38 Outflow=67.30 cfs 14.34	
Pond 22P: LEVEL SPREADER Peak Elev=574.02' Storage=109 cf Inflow=0.28 cfs 0.01 Outflow=0.29 cfs 0.01	
Pond L1+2P: ROAD DITCH @ DW Peak Elev=585.78' Storage=22 cf Inflow=1.37 cfs 0.13 Discarded=0.10 cfs 0.061 af Primary=1.26 cfs 0.072 af Outflow=1.36 cfs 0.13	
Pond L3+4P: ROAD DITCH @ DW Peak Elev=577.90' Storage=40 cf Inflow=1.68 cfs 0.14 Discarded=0.10 cfs 0.046 af Primary=1.58 cfs 0.095 af Outflow=1.68 cfs 0.14	
Pond L5P: ROAD DITCH @ DW Peak Elev=568.58' Storage=76 cf Inflow=3.19 cfs 0.26 Discarded=0.10 cfs 0.068 af Primary=3.08 cfs 0.194 af Outflow=3.18 cfs 0.26	
Pond L6+7P: ROAD DITCH @ DW Peak Elev=559.75' Storage=274 cf Inflow=4.84 cfs 0.37 Discarded=0.10 cfs 0.070 af Primary=4.73 cfs 0.301 af Outflow=4.83 cfs 0.37	
Pond L8P: ROAD DITCH @ DW Peak Elev=545.38' Storage=19 cf Inflow=5.48 cfs 0.38 Discarded=0.10 cfs 0.051 af Primary=5.36 cfs 0.331 af Outflow=5.46 cfs 0.38	
Total Runoff Area = 116.721 ac Runoff Volume = 15.984 af Average Runoff Depth = 98.62% Pervious = 115.115 ac 1.38% Impervious = 1.6	

<u> Page 3</u>

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment3D: CENTRAL SITE PLUS Runoff Area=4,708,093 sf 0.79% Impervious Runoff Depth>2.48" Flow Length=3,775' Tc=88.1 min CN=68 Runoff=106.98 cfs 22.331 af

Subcatchment 10.1D: LOT 7 DEVELOPED Runoff Area=17,600 sf 15.05% Impervious Runoff Depth>3.50" Flow Length=135' Tc=21.4 min CN=78 Runoff=1.16 cfs 0.118 af

Subcatchment 10.2D: LOT 6 DEVELOPED Runoff Area=39,406 sf 13.07% Impervious Runoff Depth>3.41" Flow Length=312' Tc=15.1 min CN=77 Runoff=2.91 cfs 0.257 af

Subcatchment 10.3D: ROAD SIDE OF LOT Runoff Area=36,777 sf 12.09% Impervious Runoff Depth>3.41" Flow Length=223' Tc=15.2 min CN=77 Runoff=2.71 cfs 0.240 af

Subcatchment 10.4D: FRONT BETWEEN Runoff Area=13,940 sf 22.87% Impervious Runoff Depth>3.71" Flow Length=180' Tc=7.8 min CN=80 Runoff=1.37 cfs 0.099 af

Subcatchment 10.5D: FRONT BETWEEN Runoff Area=29,490 sf 17.37% Impervious Runoff Depth>3.40" Flow Length=380' Tc=19.5 min CN=77 Runoff=1.97 cfs 0.192 af

Subcatchment 10D: LOT 8 DEVELOPED Runoff Area=98,423 sf 9.25% Impervious Runoff Depth>3.27"
Flow Length=1,135' Tc=48.8 min CN=76 Runoff=4.17 cfs 0.616 af

Subcatchment 20D: AREA OF LOT 7+, Runoff Area=136,683 sf 1.53% Impervious Runoff Depth>3.01" Flow Length=875′ Tc=27.3 min CN=73 Runoff=7.04 cfs 0.786 af

Subcatchment 21D: BACK OF LOT 5 Runoff Area=3,938 sf 22.55% Impervious Runoff Depth>3.62"

Tc=6.0 min CN=79 Runoff=0.40 cfs 0.027 af

Reach 10.1R: ROAD DITCH

Avg. Flow Depth=0.25' Max Vel=4.62 fps Inflow=7.10 cfs 0.491 af n=0.035 L=140.0' S=0.0929 '/' Capacity=381.91 cfs Outflow=6.97 cfs 0.491 af

Reach 10.2R: ROAD DITCH

Avg. Flow Depth=0.28' Max Vel=2.71 fps Inflow=4.61 cfs 0.318 af n=0.035 L=310.0' S=0.0287 '/' Capacity=212.36 cfs Outflow=4.55 cfs 0.318 af

Reach 10.3R: ROAD DITCH

Avg. Flow Depth=0.18' Max Vel=2.22 fps Inflow=2.36 cfs 0.159 af n=0.035 L=286.0' S=0.0311 '/' Capacity=221.09 cfs Outflow=2.33 cfs 0.159 af

Reach 10.4R: ROAD DITCH

Avg. Flow Depth=0.17' Max Vel=1.86 fps Inflow=1.87 cfs 0.118 af n=0.035 L=300.0' S=0.0230 '/' Capacity=190.07 cfs Outflow=1.84 cfs 0.118 af

Reach 10R: ROAD DITCH

Avg. Flow Depth=0.27' Max Vei=5.04 fps Inflow=8.04 cfs 0.544 af n=0.035 L=72.0' S=0.0958 '/' Capacity=334.91 cfs Outflow=8.04 cfs 0.544 af

Reach 21R: WOOD BUFFER

Avg. Flow Depth=0.09' Max Vel=0.12 fps Inflow=0.40 cfs 0.025 af n=0.800 L=50.0' S=0.1200 '/' Capacity=1.35 cfs Outflow=0.33 cfs 0.024 af

Reach 22R: SCF WOODLAND Avg. Flow Depth=0.07' Max Vel=0.61 fps Inflow=0.33 cfs 0.024 af n=0.100 L=596.0' S=0.0671 '/' Capacity=29.12 cfs Outflow=0.22 cfs 0.024 af

16149 EX DEV 050219 Prepared by Walsh Engineering Associates, Inc.	Type III 24-hr 25-YEAR Rainfall=6.20" Printed 5/3/2019
HydroCAD® 10.00-24 s/n 02135 © 2018 HydroCAD Software Solu	utions LLC Page 4
· · · · · · · · · · · · · · · · · · ·	05' Max Vel=1.44 fps Inflow=0.22 cfs 0.024 af Capacity=215.22 cfs Outflow=0.22 cfs 0.024 af
11-0.022 E-100.0 G-0.0200 /	Oapaoky-210.22 013 Outilow-0.22 013 0.024 at
Pond 1.1P: CULVERT AT HUSSEY ROAD, Peak Elev=510 24.0" Round Culvert n=0.013 L	6.18' Storage=173 cf Inflow=10.60 cfs 1.160 af =60.0' S=0.0383 '/' Outflow=10.59 cfs 1.159 af
Pond 2.1P: CULVERT AT HUSSEY ROAD 15", Peak Elev=57 15.0" Round Culvert n=0.013	26.71' Storage=871 cf Inflow=7.07 cfs 0.810 af L=30.0' S=0.0133 '/' Outflow=6.57 cfs 0.809 af
Pond 3.1P: CULVERT AT HUSSEY Peak Elev=523.75'	Storage=33,955 cf Inflow=106.98 cfs 22.331 af Outflow=106.29 cfs 22.275 af
Pond 22P: LEVEL SPREADER Peak Elev=5	74.03' Storage=110 cf Inflow=0.40 cfs 0.027 af Outflow=0.40 cfs 0.025 af
	586.01' Storage=33 cf Inflow=1.97 cfs 0.192 af ry=1.87 cfs 0.118 af Outflow=1.97 cfs 0.192 af
	578.22' Storage=59 cf Inflow=2.46 cfs 0.217 af ry=2.36 cfs 0.159 af Outflow=2.46 cfs 0.217 af

 Pond L6+7P: ROAD DITCH @ DW
 Peak Elev=560.14' Storage=411 cf
 Inflow=7.11 cfs
 0.574 af

 Discarded=0.10 cfs
 0.083 af
 Primary=7.10 cfs
 0.491 af
 Outflow=7.20 cfs
 0.574 af

 Pond L8P: ROAD DITCH @ DW
 Peak Elev=546.18' Storage=43 cf
 Inflow=8.13 cfs
 0.609 af

Pond L5P: ROAD DITCH@DW

Total Runoff Area = 116.721 ac Runoff Volume = 24.665 af Average Runoff Depth = 2.54" 98.62% Pervious = 115.115 ac 1.38% Impervious = 1.606 ac

Peak Elev=569.88' Storage=211 cf Inflow=4.80 cfs 0.398 af

Discarded=0.10 cfs 0.081 af Primary=4.61 cfs 0.318 af Outflow=4.71 cfs 0.398 af

Discarded=0.10 cfs 0.065 af Primary=8.04 cfs 0.544 af Outflow=8.14 cfs 0.609 af

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