

ABBREVIATIONS	
ACI	AMERICAN CONCRETE INSTITUTE
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AOR	ARCHITECT OF RECORD
APA	AMERICAN PLYWOOD ASSOCIATION
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BM	BEAM
BO	BOTTOM OF
BOS	BOTTOM OF STEEL
BRG	BEARING
CL	CENTERLINE
CLR	CLEAR
COL	COLUMN
CONC	CONCRETE
CONT	CONTINUOUS
DIA	DIAMETER
EL	ELEVATION
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ	EQUAL
EW	EACH WAY
FND	FOUNDATION
FF	FINISH FLOOR
FOW	FACE OF WALL
FS	FAR SIDE
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE
GC	GENERAL CONTRACTOR
GYP	GYPSON BOARD
H	HEIGHT
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HSS	HOLLOW STRUCTURAL STEEL
INFO	INFORMATION
ISO	ISOLATION
NO	NUMBER
L	LENGTH
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
MAX	MAXIMUM
MIN	MINIMUM
MISC	MISCELLANEOUS
MTL	METAL
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OH	OPPOSITE HAND
PAF	POWDER ACTUATED FASTENER
PL	PLATE
PMEJ	PREMOLDED EXPANSION JOINT
PROJ	PROJECTION
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY	QUANTITY
REF	REFERENCE OR REFER
REINF	REINFORCING
REQD	REQUIRED
RTU	ROOF TOP UNIT
SCHED	SCHEDULE
SDI	STEEL DECK INSTITUTE
SIM	SIMILAR
SJI	STEEL JOIST INSTITUTE
SPEC	SPECIFICATIONS
STL	STEEL
STRUC	STRUCTURAL
T&B	TOP AND BOTTOM
THK	THICKNESS
TO	TOP OF
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOGB	TOP OF GRADE BEAM
TOP	TOP OF PLATE
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WIDTH
WP	WORK POINT
WSP	WOOD STRUCTURAL PANEL

REBAR SCHEDULE											
DEVELOPMENT LENGTHS - Ld											
f'c = 3000 PSI						f'c = 4000 PSI					
BAR SIZE	STD Ld		CLASS B		BAR SIZE	STD Ld		CLASS B			
	TOP	TYP	TOP	TYP		TOP	TYP	TOP	TYP		
#4	15"	19"	20"	25"	#4	13"	17"	17"	23"		
#5	28"	36"	37"	47"	#5	24"	31"	32"	41"		
#6	33"	43"	43"	56"	#6	29"	37"	38"	48"		
#7	48"	63"	63"	82"	#7	42"	54"	55"	71"		
#8	55"	72"	72"	94"	#8	48"	62"	63"	81"		
#9	62"	81"	81"	106"	#9	54"	70"	71"	91"		
#10	69"	90"	90"	117"	#10	60"	78"	78"	102"		
#11	76"	98"	99"	128"	#11	66"	85"	86"	111"		
STANDARD HOOKS											
f'c = 3000 PSI						f'c = 4000 PSI					
BAR SIZE	Ldh	HOOK DIMENSIONS			BAR SIZE	Ldh	HOOK DIMENSIONS				
		"A"	"B"	"C"			"A"	"B"	"C"		
#4	6"	2½"	6"	2"	#4	6"	2½"	6"	2"		
#5	10"	2½"	7½"	2½"	#5	9"	2½"	7½"	2½"		
#6	12"	3"	9"	3"	#6	10"	3"	9"	3"		
#7	14"	3½"	10½"	3½"	#7	12"	3½"	10½"	3½"		
#8	16"	4"	12"	4"	#8	14"	4"	12"	4"		
#9	18"	4½"	13½"	5½"	#9	15"	4½"	13½"	5½"		
#10	20"	5"	15"	6½"	#10	17"	5"	15"	6½"		
#11	22"	5½"	16½"	6½"	#11	19"	5½"	16½"	6½"		
180 DEGREE HOOK						90 DEGREE HOOK					
USE THE ABOVE TABLE UNLESS NOTED OTHERWISE ON PLANS OR DETAILS											

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL TO BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
- MATERIALS:
 - WIDE FLANGE SHAPES: ASTM A992
 - CHANNELS, ANGLES, PLATES: ASTM A36
 - HOLLOW STRUCTURAL SHAPES: ASTM A500, GRADE B
 - HIGH STRENGTH BOLTS: ¾" DIAMETER (MINIMUM), A325-N.
 - ANCHOR BOLTS: ¾" DIAMETER (MINIMUM), A36 - F1554
 - ADHESIVE BOLTS: (THREADED ROD ASTM A36)
 - HILTI HIT-HY 70 ADHESIVE w/ SCREEN TUBES IN MASONRY
 - HILTI HIT-HY 200 (OR HIT-HY 250) ADHESIVE IN CONCRETE
 - EXPANSION BOLTS: HILTI KWIK BOLT III
- BOLTED CONNECTIONS:

ALL BOLTED CONNECTIONS SHALL BE SNUG-TIGHT IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" PUBLISHED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS.
- WELDED CONNECTIONS:

ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING SOCIETY CODE" (AWS D1.9) PUBLISHED BY THE AMERICAN WELDING SOCIETY. ELECTRODES FOR WELDING SHALL COMPLY WITH THE REQUIREMENTS OF TABLE 3.1 OF (AWS D1.1-96). ALL WELDING TO BE DONE BY QUALIFIED WELDERS CONFORMING TO THE AMERICAN WELDING SOCIETY STANDARDS.
- SPLICING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE, AND HOLES, SLOTS, CUTS, ETC., ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS THEY ARE DETAILED ON THE APPROVED SHOP DRAWINGS.
- NO FINAL BOLTING OR WELDING SHALL BE MADE UNTIL AS MUCH OF THE STRUCTURE AS WILL BE STIFFENED THEREBY HAS BEEN PROPERLY ALIGNED.
- FABRICATE ALL BEAMS WITH THE MILL CAMBER UP U.N.O.
- ALL COLUMN BASE PLATES SHALL BE SOLIDLY GROUTED WITH NON-SHRINK GROUT UNDER THE ENTIRE BASE PLATE AREA. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS.
- ALL HOLLOW STRUCTURAL SHAPE WELDED CONNECTIONS TO BE GROUND SMOOTH.
- THE FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PERFORMANCE OF ALL CONNECTIONS NOT FULLY DESIGNED OR DETAILED ON THE CONTRACT DOCUMENTS. FABRICATOR IS TO FURNISH ARCHITECT/ENGINEER WITH SHOP DRAWINGS FOR APPROVAL.
- ONCE THE FRAME IS ASSEMBLED, ALL STRUCTURAL STEEL SHALL BE COATED WITH A RUST INHIBITING PRIMER AND PAINTED. THE NEW AND EXISTING STEEL SHALL BE PROPERLY CLEANED AND PREPARED PRIOR TO PRIMING

PREFABRICATED WOOD TRUSSES

- TRUSSES TO BE DESIGNED AND ERECTED IN CONFORMANCE WITH TRUSS PLATE INSTITUTE SPECIFICATIONS AND RECOMMENDATIONS AND IN ACCORDANCE WITH LOCAL BUILDING CODES.
- TRUSSES TO BE BRACED DURING ERECTION. REFER TO BCSS: GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, CURRENT EDITION.
- TRUSSES TO BE DESIGNED FOR LOADS NOTED IN DESIGN INFORMATION.
- TRUSS MANUFACTURER IS TO SUBMIT LAYOUT PLANS AND CALCULATIONS FOR ALL TRUSSES. THE CALCULATIONS ARE TO BEAR A LICENSED PROFESSIONAL ENGINEER'S SEAL. CALCULATIONS AREA TO SHOW LOADINGS, SPACING, STRESSES, CONFIGURATION, CONNECTIONS, GRADE OF LUMBER, CAMBER, AND DEFLECTIONS.

ROUGH CARPENTRY

- CONTRACTOR IS RESPONSIBLE TO ADEQUATELY SHORE AND BRACE ALL FLOOR AND ROOF FRAMING AND WALLS DURING CONSTRUCTION.
- MATERIALS:
 - FRAMING LUMBER
 - JOIST, RAFTERS, HEADERS, BEAMS 2x AND LARGER, PLATES, SILLS a. NO. 2 DFL
 - STUDS, BEARING WALLS. SEE PLAN FOR SIZE USED: a. STUDS: SPRUCE-PINE-FIR "STUD" GRADE b. SILL AND TOP PLATES: NO. 2 SOUTHERN PINE
 - SPECIES AND GRADES SHOWN ARE THE MINIMUM ACCEPTABLE. BETTER GRADES MAY BE SUBSTITUTED.
 - PRESSURE TREATED WOOD TO BE USED AT THE FOLLOWING LOCATIONS: a. LUMBER EXPOSED TO WEATHER b. WOOD IN CONTACT WITH CONCRETE OR MASONRY
 - SHEATHING (WOOD STRUCTURAL PANEL)
 - ROOF SHEATHING: ¾" (NOMINAL) RATED 48/24.
 - FLOOR SHEATHING: ¾" (NOMINAL) RATED 48/24 FOR MAXIMUM JOIST SPACING, TONGUE-AND-GROOVE, GLUED TO SUPPORTS.
 - WALL SHEATHING: 1½" NOMINAL.
 - WOOD STRUCTURAL PANELS TO BE A.P.A. RATED AND EXPOSURE 1. PANELS TO BE MANUFACTURED PER U.S. DEPARTMENT OF COMMERCE PRODUCT STANDARDS PS1 OR PS2.
 - MICROCLAM:

Fb = 2600 PSI, E = 1.9 x 10 E6 PSI
 - TIMBERSTRAND BAND BOARD:

Fb = 1700 PSI, E 1.3x10 E6 PSI
 - HARDWARE:
 - BOLTS AND THREADED ROD: ASTM A307 (MIN.)
 - PREFABRICATED CONNECTORS: "SIMPSON STRONG TIE"/"USP" OR EQUAL
 - NAILS: BOX WIRE NAILS. UNLESS COMMON NAILS REQ'D BY METAL CONNECTORS.
 - USE GALVANIZED HARDWARE FOR EXTERIOR FRAMING.
- NAILING: SHALL BE PER FASTENING SCHEDULE OF THE INTERNATIONAL BUILDING CODE. FOR PREFABRICATED CONNECTORS USE ALL FASTENERS AS PRESCRIBED BY THE MANUFACTURER.
- ALL POST AND JAMBS ARE TO BE BLOCKED SOLID WITH THE SAME NUMBER OF PIECES AS THE POST OF JAMB WITHIN THE FLOOR SPACE. BLOCKING IS TO ALIGN WITH POST OR JAMBS.

GENERAL

- STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
- THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- ALL THINGS WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS, BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. PLANS AND/OR SPECIFICATIONS WILL BE CORRECTED, OR WRITTEN INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE ARCHITECT/ENGINEER BEFORE THE AFFECTED WORK PROCEEDS.
- THE CONTRACTOR SHALL INFORM THE ARCHITECT/ENGINEER OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ARCHITECT/ENGINEER'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS HE HAS SPECIFICALLY INFORMED THE ARCHITECT/ENGINEER OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ARCHITECT/ENGINEER HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- OPENINGS IN SLAB OF 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
- DO NOT SCALE THESE DRAWINGS. USE DIMENSIONS.
- UNLESS OTHERWISE NOTED, FIREPROOFING METHODS AND MATERIALS FOR STRUCTURAL MEMBERS ARE NOT SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FIRE RATING REQUIREMENTS, FIRE PROOFING METHODS AND MATERIALS.

FOUNDATIONS

- UNLESS INDICATED ON THE PLANS OR IF SITE CONDITIONS REQUIRE OTHERWISE, FOUNDATION DESIGN SHALL BE BASED ON THE FOLLOWING ALLOWABLE BEARING PRESSURES:
 - CONTINUOUS FOOTINGS: 2000 PSF
 - ISOLATED FOOTINGS: 2000 PSF
- ALL FOUNDATIONS TO BEAR ON ORIGINAL, UNDISTURBED SOIL OR FILL COMPACTED AND TESTED TO CONFORM TO THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER.
- ALL FOOTING EXCAVATIONS ARE TO BE APPROVED BY A GEOTECHNICAL ENGINEER BEFORE PLACING CONCRETE.
- FOOTINGS SHALL EXTEND BELOW THE FROST LINE: MINIMUM DEPTH OF 30" BELOW GRADE.
- UNLESS OTHERWISE NOTED ON THE PLANS OR IF SITE CONDITIONS REQUIRE OTHERWISE, FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP WITH (2) #4 BARS CONTINUOUS.
- COLUMN PADS SHALL BE A MINIMUM OF 24" x 24" x 12" WITH (3) #4 BARS EACH WAY UNLESS OTHERWISE NOTED.
- CONCRETE FLOOR SLAB ON GRADE SHALL BE A MINIMUM OF 4" THICK OVER A MINIMUM 4" BASE OF ¾" CLEAN GRAVEL.

CONCRETE

- ALL CONCRETE CONSTRUCTION TO CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318-(MOST RECENT EDITION) AND ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" ACI 301. UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL TO BE DETAILED IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES."
- CONCRETE MIX TO CONFORM TO THE FOLLOWING:
 - FOR FOOTINGS, FOUNDATION WALLS:
 - 3,500 PSI AT 28 DAYS
 - W/C RATIO = 0.50 (MAXIMUM)
 - SUMP LIMIT = 4" (+/- 1")
 - AIR-ENTRAINED = 5%-7%
 - FOR INTERIOR SLABS:
 - 4,000 PSI AT 28 DAYS
 - W/C RATIO = 0.45 (MAXIMUM)
 - SUMP LIMIT = 4" (+/- 1")
 - AIR-ENTRAINED = MAX 3%
- ALL REINFORCED STEEL TO BE ASTM A 615, DEFORMED.
 - GRADE 40, #4 BAR & <
 - GRADE 60, #5 BAR & >
- WELDED WIRE FABRIC TO BE ASTM A 185. LAP FABRIC 6" MINIMUM. FILL SLABS AND SLABS ON GRADE SHALL BE REINFORCED WITH 6x6-W1.4 x W1.4 W.W.F. UNLESS NOTED OTHERWISE.
- REINFORCING SHALL BE CONTINUOUS WHEREVER POSSIBLE. SPLICES AND LAPS TO CONFORM TO ACI 318 REQUIREMENTS. REF SPLICE TABLE ON S1.0.
- CONTRACTOR TO HIRE A TESTING LABORATORY TO TAKE CONCRETE CYLINDER TEST. FREQUENCY OF TESTING IS TO BE IN ACCORDANCE WITH ACI 318 REQUIREMENTS. TAKE FIVE (5) CYLINDERS EACH SAMPLE, TEST TWO (2) AT SEVEN (7) DAYS, AND TWO (2) AT 28, AND HOLD ONE IN RESERVE UNTIL PROJECT IS COMPLETED. TESTING LABORATORY IS TO FURNISH ARCHITECT/ENGINEER WITH TEST RESULTS PROMPTLY.
 - ONCE EACH DAY A GIVEN CLASS IS PLACED, NOR LESS THAN
 - ONCE FOR EACH 150 YD³ OF EACH CLASS PLACED EACH DAY. NOR LESS THAN
 - ONCE FOR EACH 5000 SQ/FT OF SLAB WALL OR SURFACE AREA PLACED EACH DAY.
- DOWELS IN FOOTING, WALLS, AND DRILLED PIERS MUST BE IN POSITION BEFORE PLACING CONCRETE WHENEVER POSSIBLE.
- UNLESS OTHERWISE SHOWN IN THE ARCHITECTURAL DRAWINGS, PROVIDE ¾" CHAMFERS AT THE EDGES THAT ARE EXPOSED TO VIEW IN THE FINISHED STRUCTURE.
- REFER TO ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, DRIP SLOTS, REGLETS, MASONRY, ANCHORS, BRICK LEDGE ELEVATIONS AND FOR MISCELLANEOUS EMBEDDED PLATES, BOLTS, ANCHORS, ANGLES, ETC.
- REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES. WHERE FINISH IS NOT SPECIFIED, CONFORM TO REQUIREMENTS OF ACI 301.
- REFER TO MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR DRAINS, SLEEVES, OUTLET BOXES, CONDUIT, ANCHORS, ETC. THE VARIOUS TRADES ARE RESPONSIBLE FOR PLACING THEIR ITEMS.
- REFER TO MECHANICAL DRAWINGS FOR HOUSEKEEPING PADS AND INERTIA BASES AT MECHANICAL EQUIPMENT.

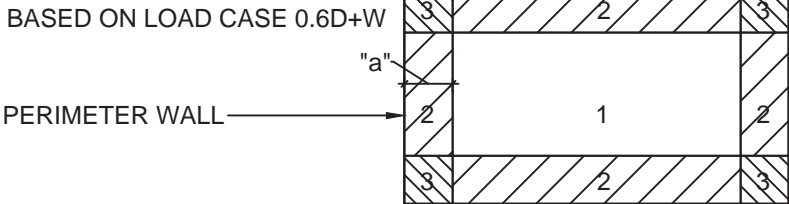
DESIGN INFORMATION

- BUILDING CODE: 2012 INTERNATIONAL BUILDING CODE AS ADOPTED AND/OR AMENDED BY LOCAL BUILDING CODES
- WIND DESIGN INFORMATION:

ULTIMATE WIND SPEED (3 SECOND GUST) V _{ult} =	115 MPH
WIND EXPOSURE CATEGORY	B
RISK CATEGORY	II
INTERNAL PRESSURE COEFFICIENT	GC _{pi} = ±0.18
DIRECTIONALITY FACTOR	K _d = 0.85
VELOCITY PRESSURE COEFFICIENT	K _h = 0.7
TOPOGRAPHICAL FACTOR	K _{zt} = 1.0
EDGE REGION	a = 8' - 6"
VELOCITY PRESSURE (WALL)	q _z = 15 PSF
WIND NET UPLIFT LOADS:	
ZONE 1	14 PSF
ZONE 2	14 PSF
ZONE 3	15 PSF

WIND UPLIFT DIAGRAM

1. NET UPLIFT LOADS ARE BASED ON LOAD CASE 0.6D+W



- SEISMIC DESIGN INFORMATION:

SEISMIC ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE
SPECTRAL RESPONSE COEFFICIENTS	S _s = 0.2908 S ₁ = 0.0801
SITE CLASS (ASSUMED)	D
SEISMIC RESISTING SYSTEM	LIGHT FRAMED WALLS
DESIGN SPECTRAL RESPONSE ACCELERATIONS	S _{ds} = 0.304 S _{d1} = 0.128
RESPONSE MODIFICATION	R = 6.50
DEFLECTION AMPLIFICATION FACTOR	C _d = 4.00
OVERSTRENGTH FACTOR	Q = 3.00
SEISMIC RESPONSE COEFFICIENT	C _s = 0.0467
SEISMIC DESIGN CATEGORY	B
SEISMIC BASE SHEAR	V = C _s * W
- ROOF LOADING INFORMATION:

A. TOP CHORD	
1) DEAD LOAD	10 PSF
2) LIVE LOAD	20 PSF
3) SNOW LOAD	15 PSF
B. BOTTOM CHORD	
1) DEAD LOAD	10 PSF
2) LIVE LOAD	5 PSF
- FLOOR LIVE LOADING INFORMATION:

FLOOR	100 PSF
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SPECIAL INSPECTIONS

- SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE AND THE SPECIFICATIONS OF THIS PROJECT PER SPECIAL INSPECTION TABLE THIS SHEET.

INSPECTION TASK	FREQUENCY		CODE REFERENCE
	CONT	PERIODIC	
A. SOILS			
1.) VERIFICATION OF SOILS, EXCAVATION		X	IBC TABLE 1705.6
2.) PLACEMENT OF COMPACTED FILL	X		IBC TABLE 1705.6
B. REINFORCED CONCRETE			
1.) PLACEMENT OF REINF STEEL		X	IBC TABLE 1705.3-1
2.) ANCHORS CAST IN PLACE		X	IBC TABLE 1705.3-3
3.) POST INSTALLED ANCHORS	X		IBC TABLE 1705.3-4
4.) PLACEMENT OF CONCRETE	X		IBC TABLE 1705.3-7
C. STRUCTURAL STEEL			
1.) FIELD WELDING OF STR STEEL	AS REQD PER AISC		AISC 14th ED CHAPTER N
2.) FIELD BOLTING OF STR STEEL	AS REQD PER AISC		AISC 14th ED CHAPTER N
3.) STEEL FRAME INSPECTION	AS REQD PER AISC		AISC 14th ED CHAPTER N
D. WOOD CONSTRUCTION	AS REQ'D PER IBC		IBC 2009 1706.2

SHEET INDEX

S1.0	GENERAL NOTES & SPECIFICATIONS
S1.1	FOUNDATION PLAN
S1.2	FRAMING PLAN
S2.0	FOUNDATION DETAILS
S3.0	FRAMING DETAILS
S4.0	BRACED WALL DETAILS



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GENERAL NOTES

DATE
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SHEET NO.

S1.0



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