

P16-0712

NCHRP-IDEA
Project 162

Low-Cost Scour-Preventing Streamlined Fairings for Bridges

www.noscour.com

Permanent Prevention of Bridge Scour: scAUR™ and VorGAUR™ Products



Designs for all types of piers and abutments

Designed and built to meet AASHTO and ACI Standards

scAUR™ Steel Concrete Forms for New Construction
The best time to install at a fraction of retrofit cost!

Wing-wall abutment concrete forms

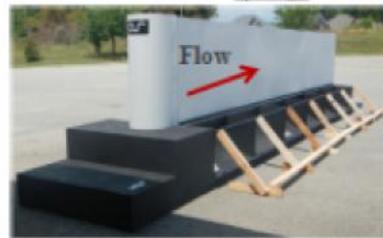
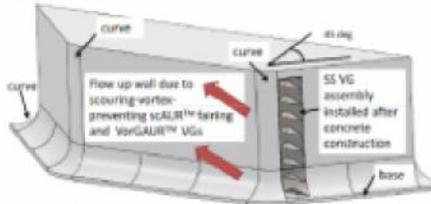


Completed new construction abutment

Spill-through abutment forms



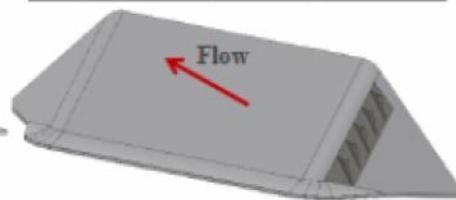
Completed new spill-through abutment



Forms for new Piers

Standard rebar methods for foundation construction

Modular interlocking forms permit quick and exact assembly and preserve the scAUR™ shape



Modular Stainless Steel (SS) Retrofits for Existing Bridges
Greatly extends bridge life! Modules quick and easy to install.

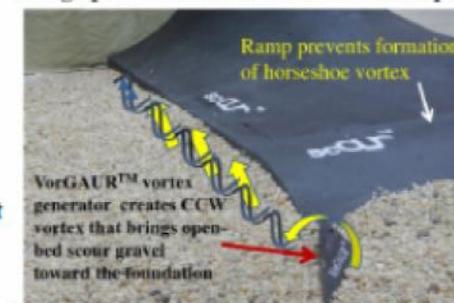


Example stainless steel scAUR™ retrofit (black) for a 45° wing-wall abutment. Note SS vortex generators.

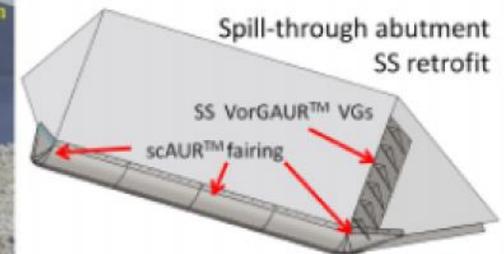


Example stainless steel scAUR™ retrofit (black) for a pier. VorGAUR™ vortex generators create CW vortices that bring low-speed flow up to prevent scour.

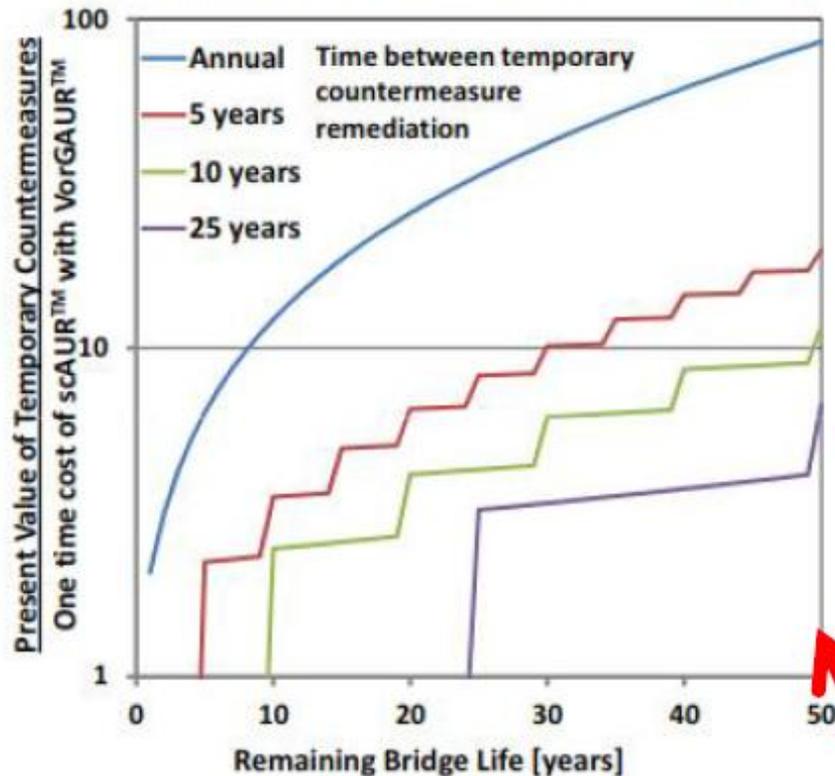
Ramp and VorGAUR™ vortex generator bring open-bed scour material toward a pier



Pre-fabricated interlocking modules permit quick and exact assembly and preserve the scAUR™ shape



Economics of Stainless Steel *scAUR*TM Retrofits

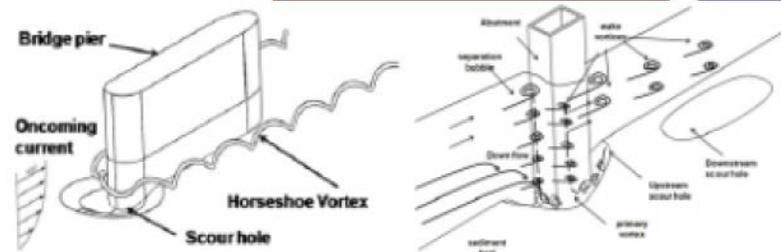


Costs of temporary countermeasures obtained from a number of published sources.
 Computed with 7% inflation and 5% tax exempt interest.
Example of a bridge with six piers and two abutments requiring protection.

*scAUR*TM Manufacturer AUR, Inc.
aur@aurinc.com
 Ph: 540-961-3005 Fax: 866.223.8673.

- Temporary scour countermeasures (TSC) carry compounding future costs (monitoring, inspections, engineering, remediation) with *real present value*.
- *scAUR*TM is a permanent sustainable scour prevention measure with a **one-time cost**. **Stainless steel costs ½ as much as concrete.**
- *scAUR*TM **prevents catastrophic failure risk and liability due to local scour and saves >90% of present value of TSC.**
- The methods of **HYRISK** used to compare *scAUR*TM to temporary countermeasures.
 - Risks from temporary countermeasures incur substantial costs and liabilities.
 - Failure probabilities yield the costs that are implicitly assumed by the bridge owner due to risk.

***scAUR*TM is the clear economic choice for bridges with or likely to have severe local scour.**



Bridge scour is produced by discrete vortices formed around unprotected piers (left above) and abutments (right). Many near catastrophes and loss of life have occurred, as shown in examples below.



Loon Mountain Bridge (Lincoln, N.H.) collapsed due to heavy scouring around the abutment after 11" of rain.



Scour caused Irish railway bridge pier collapse minutes after passage of a full Dublin-to-Belfast commuter train, 3 September 2009.

scAUR™ with VorGAUR™ products will prevent such failures.

Based on aero/hydrodynamic design concepts, scAUR™ and VorGAUR™ products prevent the discrete vortices that cause scour. Extensive computer modeling and model and full-scale testing have proven these products.

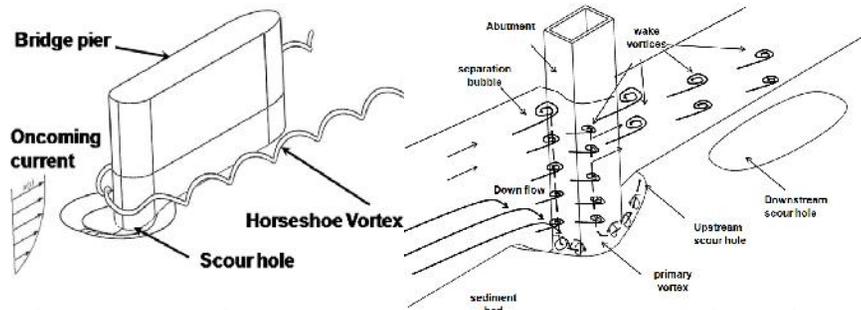


Example AUR flume test of pier with scAUR™ and VorGAUR™ - NO SCOUR! Note scour on cylinder.

Other Features of scAUR™ and VorGAUR™

- 1. Much lower present value of present and future scour mitigation costs as compared to other approaches.**
- 2. Lower drag force, flow blockage, water level, and over-topping frequencies on bridges during flood conditions, any water level or inflow turbulence level.**
- 3. Debris accumulation prevention and pier and abutment protection from impact loads because of the streamlined flow without a horseshoe vortex, which deflects objects and debris away from the underwater structure.**
- 4. High quality proven-technology prefabricated stainless steel or cast concrete components for quality control and rapid installation.**
- 5. More stability for the soil and rocks surrounding the piers and abutments.**
- 6. 100 year or more lifetimes and longer bridge life.**

Bridge Scour is Prevented by the Use of scAUR™ and VorGAUR™ that Prevent Scouring Vortices



Bridge scour is produced by discrete vortices formed around unprotected piers (left) and abutments (right)

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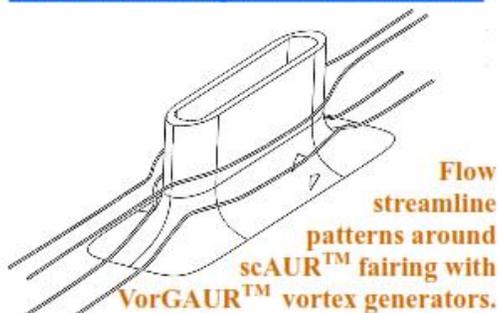
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Full-Scale Prototype Testing and Manufacturing and Installation Plans for New Bridge-Scour-Prevention scAUR™ and VorGAUR™ Products: NCHRP IDEA Project 162

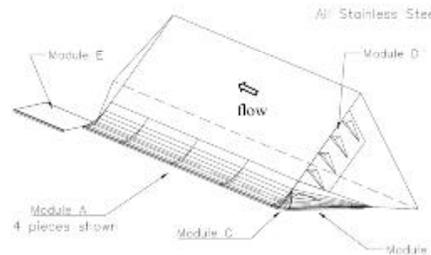


- About 60% of bridge failures in the U.S. are due to scour (1)
- Over 20,000 scour-critical U.S. bridges are in danger of failure, because of scour around the piers and abutments (1).
- **HEC 23: Rip-rap countermeasures for scour are unacceptable for new bridges; Use streamlined piers and abutments.**

Sustainable Solution : scAUR™ (2) streamlined fairings with VorGAUR™ (2) vortex generators prevent scouring vortices around piers and abutments



TYPICAL APPLICATION
Scour Preventing scAUR™ and VorGAUR™ on New or Retrofit 45 degree spill-through abutment.



- scAUR™ fairing and VorGAUR™ products **prevent scouring vortices at both model and full scale.**
- scAUR™ and VorGAUR™ products **prevent scouring vortices around piers & abutments downstream of river bends.**
- Tests show that they **prevent scour at incident flow angles of attack up to 45 degrees** with special design features.
- Tests prove that they **prevent local scour for a large range of bed materials.**
- **INCREASES FLOW RATE AROUND PIERS AND ABUTMENTS**

Prevents foundation scour when there is open-bed scour.

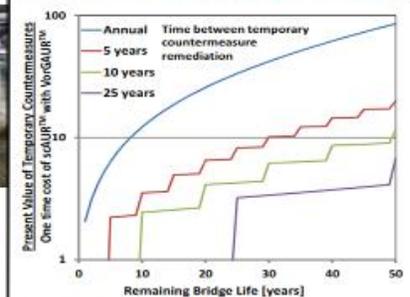
Open Bed Scour Case: scAUR™ full-scale model with VorGAUR™ VGs raised 3" above gravel bed with leading edge curved ramp whose stream-wise vortex brings bed material toward foundation. Looking downstream (middle photo) and looking upstream (right).



Manufacturing and Installation for Piers and Abutments

Retrofits - Prefabricated scAUR™ fairing with VorGAUR™ stainless steel units are the most cost effective, easy to install, and reliable for a long life compared to concrete.
New Construction - Steel scAUR™ fairing concrete forms replace foundation concrete forms. Low costs.

AUR, Inc.
www.noscour.com
The economics of stainless steel scAUR™ with VorGAUR™ units



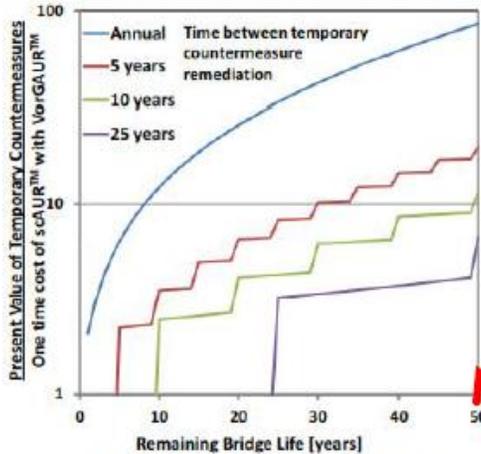
Cost of current temporary countermeasures ALWAYS MORE than the cost of scAUR™ after ONE scour mitigation.

Risk and liability of bridge failure is ALWAYS LESS WITH scAUR™.

Plans complete for installation on a scour-critical bridge: US 360 over the Appomattox River, VA

1. Beatrice E. Hunt (2009), Monitoring Scour Critical Bridges, NCHRP Synthesis 396.
2. scAUR™ in US Patent No. 8,348,553 and VorGAUR™ in US Patent No. 8,434,723.

Economics of Stainless Steel *scAUR*TM Retrofits



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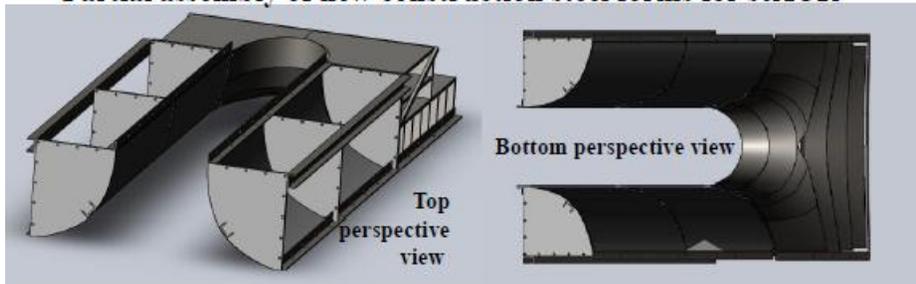
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Manufacturing and Installation Processes

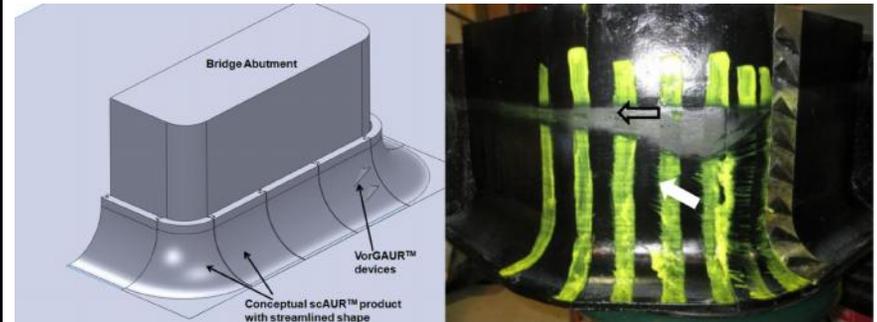
New construction – Cast-in-place Concrete – 1/3 Cost of Retrofit

- Only difference with current practice: use *scAUR*TM steel forms for concrete
- All standard current concrete construction methods and tools used.
- Bridge pier or abutment foundation or footer top surface width and length large enough for *scAUR*TM concrete fairing on top.
- Rebar for the *scAUR*TM concrete included in the foundation during construction.
- Stainless steel rebar for welding to stainless steel vortex generators mounting plates on the surface used for specific locations.

Partial assembly of new construction steel forms for *scAUR*TM



ALL Designs of Piers and Abutments are Permanently Protected from Scour by Vortex-preventing *scAUR*TM and *VorGAUR*TM



Vertical abutment

Wing-wall abutment

Vortex-preventing *scAUR*TM with *VorGAUR*TM cause near-river-bottom water to move up abutment and piers

Spill-through abutment



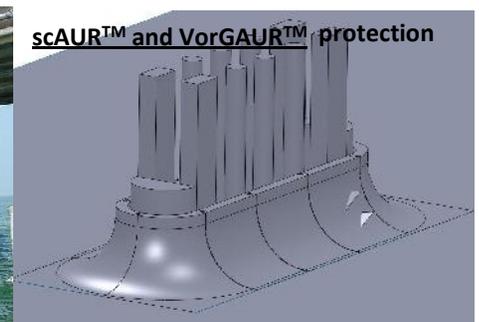
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Multiple pier arrangements



Protects coastal structures for 100 years

Some Recent Bridge Failures due to Scour

Abutments

ALL of these failures would have been prevented and the piers and abutments permanently protected from scour by vortex-preventing scAUR™ with VorGAUR™



The Loon Mountain Bridge in Lincoln, N.H., collapsed due to heavy scouring of the East Branch of the Pemigewasset River following Tropical Storm Irene.

The Loon Mountain Bridge in Lincoln, N.H. collapsed due to heavy scouring around the abutment after 11 inches of rain. (From Structural Engineer, p. 32, August 2013)

AUR, Inc. www.noscour.com



USGS photo
file:///F:/Abutments/File%20Abutment_scour2.jpg.htm



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Piers



Scour revealed as cause of Irish bridge collapse

3 September 2009 | By Diarmaid Fleming
Scour undermining a Victorian masonry bridge pier has been identified as the likely cause of a near-disastrous collapse of a section of railway viaduct on the Dublin-to-Belfast main line.

<http://www.nce.co.uk/news/structures/scour-revealed-as-cause-of-irish-bridge-collapse/5207460.article>

AUR, Inc. www.noscour.com

Schoharie Creek, NY State Thruway, April 5th, 1987; 5 vehicles fell into the river, and 10 occupants died. The direct cause of the collapse was excessive scour under bridge pier (Storey and Delatte, 2003). The indirect human cause of the collapse was the failure to maintain the bridge riprap (Storey and Delatte, 2003). The lawsuits against the New York State Thruway Authority were settled for a total cost of about \$4.5 million with 50% inflation (Wattson, 2007)

Bridge scour: Potentially catastrophic

- Case: Schoharie Creek, NY State Thruway, April 5th , 1987;
- Five vehicles fell into the river, and ten occupants died;
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- The lawsuits against the New York State Thruway Authority were settled for a total cost of about \$4.5 million with 50% inflation (Wattson, 2007)



photo credit: U.S. Department of the Interior, U.S. Geological Survey

Reference

1. Chris Storey and Norbert Delatte, 2003, Lessons from the Collapse of the Schoharie Creek Bridge, ASCE, Forensic Engineering, pp. 158-167
2. Peter S. Wattson, 2007, "Compensating Victims of Bridge Collapses Outside Minnesota"

1 Paper 16-0712, Transportation Research Board 95th Annual Meeting
2 Walter E. Washington Convention Center, Washington, DC
3 January 10-14, 2016.
4

5 **Title: Implementation of NCHRP IDEA 162: Low-Cost**
6 **Scour-Preventing Streamlined Fairings for Bridges**
7

8 4621 words, 11 figures
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Abstract

Cost-effective optimized robust scour preventing three-dimensional convex-concave hydrodynamic fairings with attached vortex generators have been designed, developed, extensively tested, and are now available for practical use. These were tested for bridge piers and abutments during a National Cooperative Highway Research Program (NCHRP-IDEA) project. Their particular shape prevents creation of scouring vortices that cause the local scour problem for any river level, speed, and angles of attack up to 20 degrees, unlike a fairing shape used by FHWA that does not prevent scour. This device exceeds requirements for HEC-23. Cost-effective versions are of stainless-steel or conventionally cast concrete that are attached to an existing or cast as part of the base of a new hydraulic structure above the footing, respectively. The vortex generators energize the decelerating near-wall flow with higher-momentum flow, resulting in a more steady, compact downstream separation and wake and substantially mitigated scour inducing vortical flow. Experimental test results confirm that scAUR™ scouring-vortex-preventing fairings prevent foundation local scour for smaller sediments, wing-wall and spill-through abutments, and full-scale piers, as well as preventing the effects of open-bed scour on foundations.

49

50 Other advantages of this robust device over other current
51 approaches are: (1) much lower costs for scour prevention and
52 bridge maintenance; (2) much lower probability of bridge
53 failure;(3) lower river levels due to lower drag and lower flow
54 blockage around the pier or abutment; (4) much lower possibility
55 for debris and ice buildup; and (5) greater protection of piers
56 and abutments against impact loads.

57

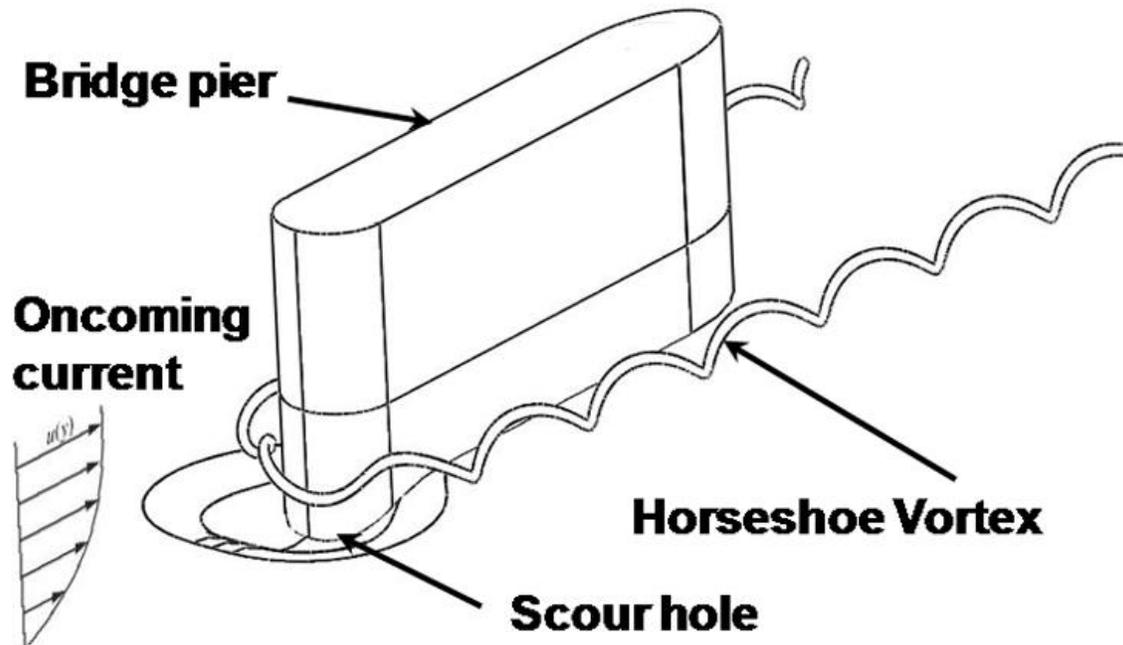
58 **Introduction- Background of Bridge Pier and Abutment Scour**

59 Removal of river bed substrate around bridge pier and abutment
60 footings, also known as scour, presents a significant cost and
61 risk in the maintenance of many bridges throughout the world and
62 is one of the most common causes of highway bridge failures (1).
63 It has been estimated that 60% of all bridge failures result
64 from scour and other hydraulic-related causes (2). This has
65 motivated research on the causes of scour at bridge piers and
66 abutments (3) and led bridge engineers to develop numerous
67 countermeasures that attempt to reduce the risk of catastrophe.
68 Unfortunately, all currently used countermeasures are temporary
69 responses that require many recurring costs and do not prevent
70 the formation of scouring vortices, which is the root cause of
71 the local scour (4,5). Consequently, sediment such as sand and
72 rocks around the foundations of bridge abutments and piers is

73 loosened and carried away by the flow during floods, which may
74 compromise the integrity of the structure. Even designing bridge
75 piers or abutments with the expectation of some scour is highly
76 uncertain, since a recently released study (4) showed huge
77 uncertainties in scour data from hundreds of experiments. None
78 of the conservative current bridge pier and abutment footing or
79 foundation designs prevent scouring vortices, which are created
80 when the flow interacts with underwater structures, so the
81 probability of scour during high water or floods is present in
82 all current designs.

83

84 The bridge foundations in a water current, such as piers and
85 abutments, change the local hydraulics drastically because of
86 the appearance of large-scale unsteadiness and shedding of
87 coherent vortices, such as horseshoe vortices. Figure 1 is a
88 sketch of the horseshoe vortex formed around the base of a pier
89 by a separating boundary layer. The horseshoe vortex produces
90 high bed shear stress, triggers the onset of sediment scour, and
91 forms a scour hole.



92

93 **Figure 1. The formation of a horseshoe vortex around the bottom**
 94 **of a bridge pier with no scouring-vortex prevention.**

95

96 The flowfield around an abutment is also highly three-
 97 dimensional and involves strong separated vortex flow (6). A
 98 separation bubble is formed at the upstream corner of the
 99 abutment. Unsteady shed wake vortices are created due to the
 100 separation of the flow at the abutment corners. These wake
 101 vortices are very unsteady, are oriented approximately parallel
 102 to the abutment edge and have low pressure at the vortex cores.
 103 These vortices act like small tornadoes, lifting up sediment and
 104 creating a large scour hole behind the abutment. The downflow at
 105 the front of the abutment is produced by the large stagnation
 106 pressure gradient of the approaching flow. The down flow rolls

107 up and forms the primary vortex, which is similar to the
108 formation of the horseshoe vortex around a single bridge pier.

109
110 It should be noted that rip rap countermeasures are not
111 acceptable design elements for new bridges (1). To avoid
112 liability risk to engineers and bridge owners, new bridges must
113 be over-designed to withstand 500-year superfoods, assuming
114 that all sediment is removed from the 'scour prism' at that flow
115 rate (1). Unlike temporary scour countermeasures, the
116 streamlined control Against Underwater Rampage fairing sCAUR™
117 (pronounced like 'scour') designs avoid liability risk by
118 preventing or drastically diminishing the scour prism and
119 reducing the cost of new bridge engineering and construction.
120 This greatly reduces the probability of failure, by the tenets
121 of catastrophic risk theory (7).

122

123 **Features of sCAUR™ that Prevent Scouring Vortices**

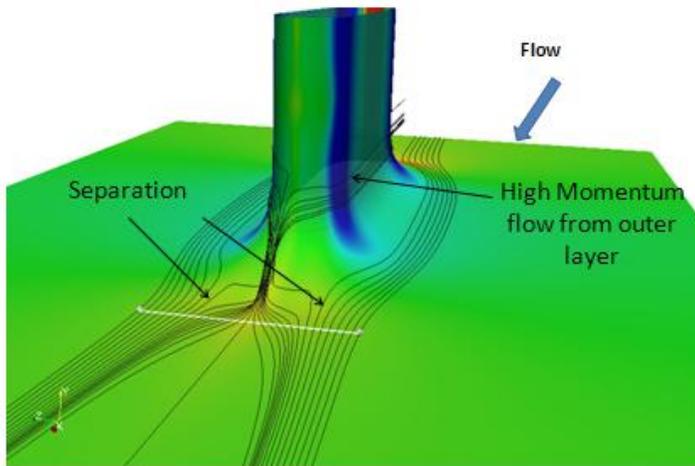
124 Using the knowledge of how to prevent the formation of discrete
125 vortices and separation for junction flows (8,9,10), prior to
126 the NCHRP-IDEA-162 project, AUR developed, proved using model-
127 scale tests, and patented new local-scouring-vortex-prevention
128 sCAUR™ products. The sCAUR™ design fundamentally alters the way
129 the river flows around a pier or abutment. The sCAUR™ scouring-
130 vortex preventing fairing, US Patent No. 8,348,553, and VorGAUR™

131 tetrahedral vortex generators, US Patent No. 8,434,723, are
132 practical long-term permanent solutions. A hydraulically optimum
133 pier or abutment fairing prevents the formation of highly
134 coherent vortices around the bridge pier or abutment and reduces
135 3D separation downstream of the bridge pier or abutment with the
136 help of the VorGAUR™ vortical flow separation control (Figure
137 2). This is in contrast to a fairing shape used in an
138 unpublished FHWA study which did not prevent scour for flows at
139 angles of attack.

140
141 Recent NCHRP research using hundreds of sets of scour data (4)
142 shows that model-scale bridge scour experiments produce much
143 more severe scour depth to pier size ratios than the scour depth
144 to pier size ratios observed for full-scale cases due to scale
145 or size effects. Thus, the scAUR™ fairing will work just as well
146 in preventing the scouring vortices and any scour at full scale
147 as at the proven model scale.

148

Low Reynolds Number Case - Near wall streamlines pass through
 $X/t = 7.24$ and $Y/t = 0.013$



149

150 **Figure 2 Low Reynolds number case CFD calculated flow streamline**
151 **patterns around a scAUR™ streamlined bridge pier fairing. Flow**
152 **indicates no discrete vortex formation on nose and sides.**

153

154 Recent NCHRP-IDEA-162 Project

155 This project focused on providing more evidence that the scAUR™
156 and VorGAUR™ concepts and products work at full scale in
157 preventing scour-producing vortices and for a wider range of
158 geometries and conditions. Task I, which is not discussed
159 further here, dealt with selecting a scour-critical bridge in
160 Virginia for prototype installation (7). Further computational
161 work on the effect of pier size or scale (Task II) and model
162 flume tests for other sediments (Task III), other abutment
163 designs (Task IV.A), and for open bed scour conditions (Task
164 IV.B) were done to expand confidence in these concepts and

165 designs. Constructed full-scale prototypes (Task V, not
166 discussed here) were tested (Task VI). Cost-effective
167 manufacturing and installation of scAUR™ and VorGAUR™ products
168 were further developed (Task VII).

169

170 **TASK II - Computational Fluid Dynamic (CFD) Calculations for a**
171 **Full-scale Pier compared to low Reynolds Number Model-scale CFD**

172 While much previous AUR computational and experimental work at
173 model size ($Re_t = 1.34 \times 10^5$, pier width $t = 0.076\text{m}$) was done to
174 prove these designs, Reynolds number and bridge pier size
175 effects were examined using computations to confirm the
176 applicability of these products at full scale ($Re_t = 2.19 \times 10^6$, t
177 $= 0.624\text{m}$). Since the V2F Reynolds-averaged Navier-Stokes (RANS)
178 model in the Open Foam code is proven to accurately compute 3D
179 flows and the presence of any separation or discrete vortices
180 (7,8,9,10,11,12), then the behavior of mean streamlines, the
181 local non-dimensional surface pressure coefficient C_p , and the
182 local surface skin friction coefficient C_f are sufficient to
183 determine if any separation or discrete vortices are present(7).

184

185 Figure 2 shows a perspective view from downstream of near-wall
186 streamlines that pass through $X/t = 7.24$ at $Y/t = 0.013$, where t
187 is the pier width. No vortices or separation are observed
188 upstream of the stern or tail of the pier and there are similar

189 streamline features for both Reynolds numbers. An important
190 feature in the C_p and the C_f results is the lack of any abrupt
191 changes in the slope of C_p or C_f over a short distance, which
192 means that there is no discrete vortex formation and separation.
193 The non-dimensional drag on the pier is clearly lower for the
194 higher Reynolds number case because C_f is always lower and the
195 overall drag is an integral of the surface shearing stress over
196 the pier surface area. In addition, these results show lower
197 flow blockage than without the scAURTM and VorGAURTM products
198 because low velocity swirling high flow blockage vortices are
199 absent. As a result, water moves around a pier or abutment
200 faster near the river surface, producing a lower water level at
201 the bridge and lower over-topping frequencies on bridges during
202 flood conditions for any water level when no discrete vortices
203 are present.

204
205 Based on the past published work on scour and experience of AUR
206 (8, 9, 10), more physical evidence and insights support the idea
207 that these scour vortex preventing devices will work better at
208 full scale than model scale. Scouring forces on river bed
209 materials are produced by pressure gradients and turbulent
210 shearing stresses, which are instantaneously unsteady. At higher
211 Reynolds numbers and sizes, pressure gradients and turbulent
212 fluctuation stresses are lower than at model scale, so scour at

213 the same flow speed is lower. **Work by others (3,4,13) supports**
214 **the conclusion that scour predictive equations, developed**
215 **largely from laboratory data, overpredict scour on full-scale**
216 **underwater structures. Thus, the scaUR™ and VorGAUR™ work as**
217 **well or better in preventing the scouring vortices and any scour**
218 **at full scale as at the proven model scale.** Other CFD by AUR,
219 not reported here, shows that scaUR™ and VorGAUR™ products also
220 prevent scouring vortices around bridge piers downstream of
221 bending rivers.

222

223 **TASK III Flume Tests with Several Smaller Size Sediments at**
224 **Model Scale**

225 Data on the performance of the scaUR™ fairing and VorGAUR™ VGs
226 were obtained using several smaller size sediments at model
227 scale in the AUR flume to prove the applicability of the designs
228 for fine sediments (7). All tests were at a flow speed of
229 0.66mps when incipient open bed scour of the pea gravel (3.2mm
230 to 6.3mm) was first observed. Melville (14) states that the
231 greatest equilibrium scour depth occurs around a circular pier
232 (width = t) when it is surrounded by uniform sediment at times
233 when the flow velocity equals the critical value, i.e.,
234 incipient conditions for open bed scour. Also, live bed scour
235 depth is never larger than incipient scour depth. Melville
236 states: "Recent data by Sheppard et al. (13)

237 demonstrate significant scour depth reductions for increasing
238 t/d50 when t/d50 > 50. Thus, local scour depths at field scale
239 may be significantly reduced from those observed in the
240 laboratory." The "t/d50" term is the ratio of pier width to
241 median grain diameter. A value of t/d50=50 was used, with a
242 range of sediments from 38.1 to 64.6.

243
244 Three sieved sand or gravel sizes were used to encompass this
245 range for previously reported flow conditions where scour will
246 be the greatest for the AUR t = 76.2mm wide model pier: Gravel
247 A: 1.18 to 1.4 mm; Gravel B: 1.4 to 1.7mm; Gravel C: 1.7 to
248 2mm. Usually smaller sediment scours before larger pea gravel.
249 No scour around the sCAUR™ model occurred for any of these black
250 slag gravel at speeds when the open bed pea gravel began to
251 scour (7) within the $y/t = +/- 0.004$ measurement uncertainty.

252
253 Task IV.A - Flume Tests of SCAUR™ and VorGAUR™ Concepts for a
254 Larger Class of Abutments

255 The performance of sCAUR™ and VorGAUR™ concepts for wing-wall
256 and spill-through abutments was examined by model scale flume
257 tests at incipient open bed scour flow speeds of 0.66mps (7) and
258 show that sCAUR™ and VorGAUR™ prevent the formation of scouring
259 vortices and scour.

260

261 Figure 3 shows surface oilflow results for a sCAUR™ modified
262 wing-wall abutment with VorGAUR™ vortex generators (VGs)(7). The
263 mixture of yellow artist oil paint and mineral oil flows with
264 the skin friction lines. Yellow streaks are first painted about
265 perpendicular to the flow direction on a black painted surface.
266 The flow causes some oil to be carried downstream in a local
267 flow direction, which can be observed against the black painted
268 surface. **Figure 3 clearly shows that the effects of the sCAUR™**
269 **with VorGAUR™ are to bring lower velocity flow up from the flume**
270 **bottom and prevent the scour around the bottom of the abutment.**

271
272 **With a sCAUR™ modified wing-wall abutment with VGs, there is not**
273 **only no scour around the model base (Figure 4), but there is no**
274 **open bed scour hole farther downstream of the model around $x/L =$**
275 **2.** This is because the VGs generate counter-rotating vortices
276 which diffuse and reduce the strength of the free-surface
277 generated vortex, which caused the scour hole farther downstream
278 of the model for the untreated case.

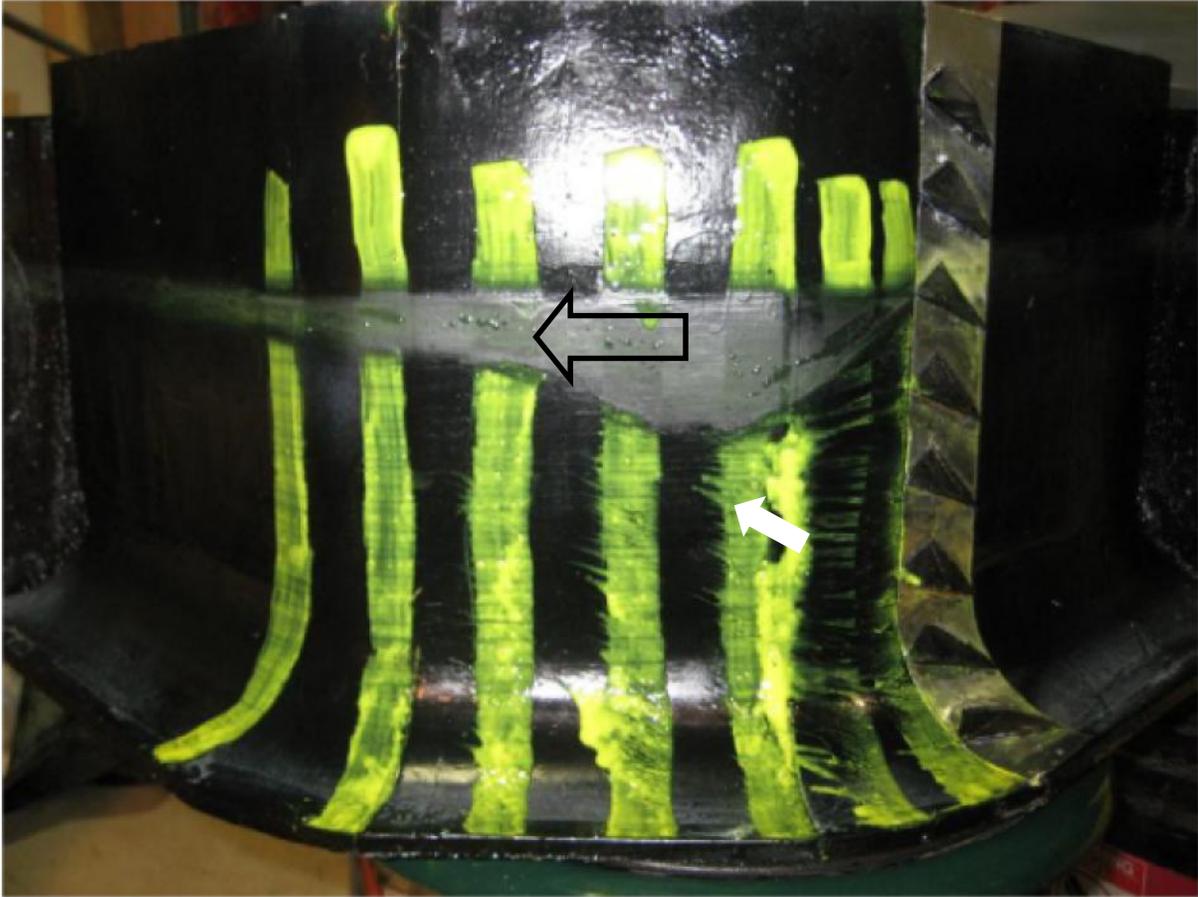


Figure 3. Surface oilflow results for the modified wing-wall abutment model with VGs. Flow from right to left. The upward streaks show that sCAUR™ and VorGAUR™ products cause the flow to move up the abutment. The gray region is produced by a mixture of the oilflow material and waterborne substances at the free surface.

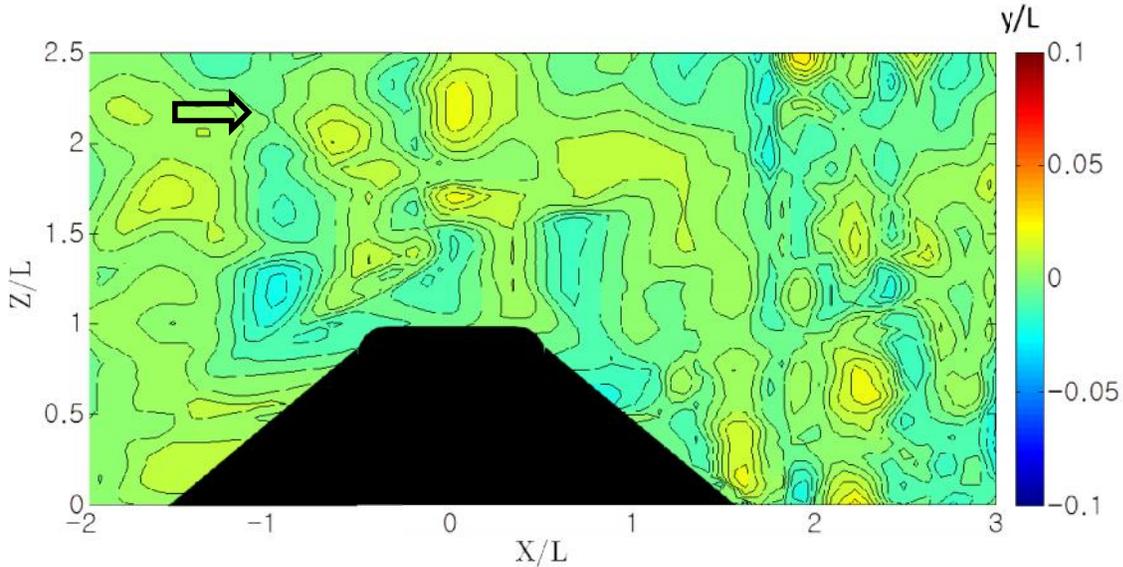
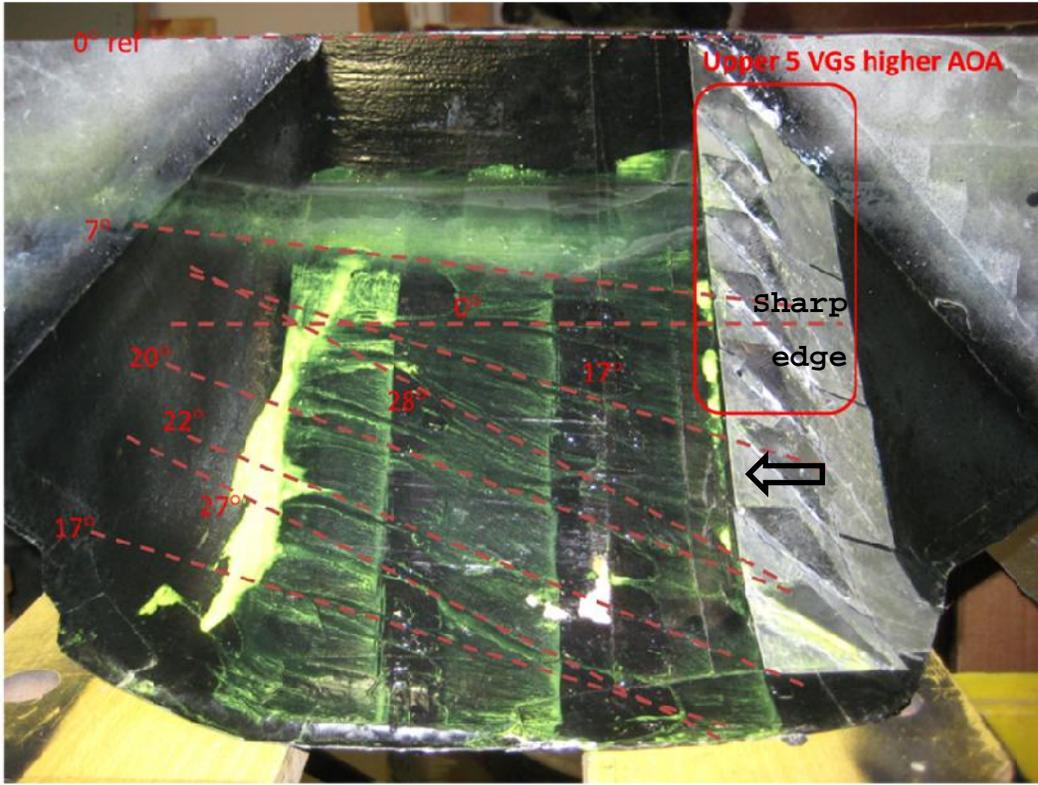


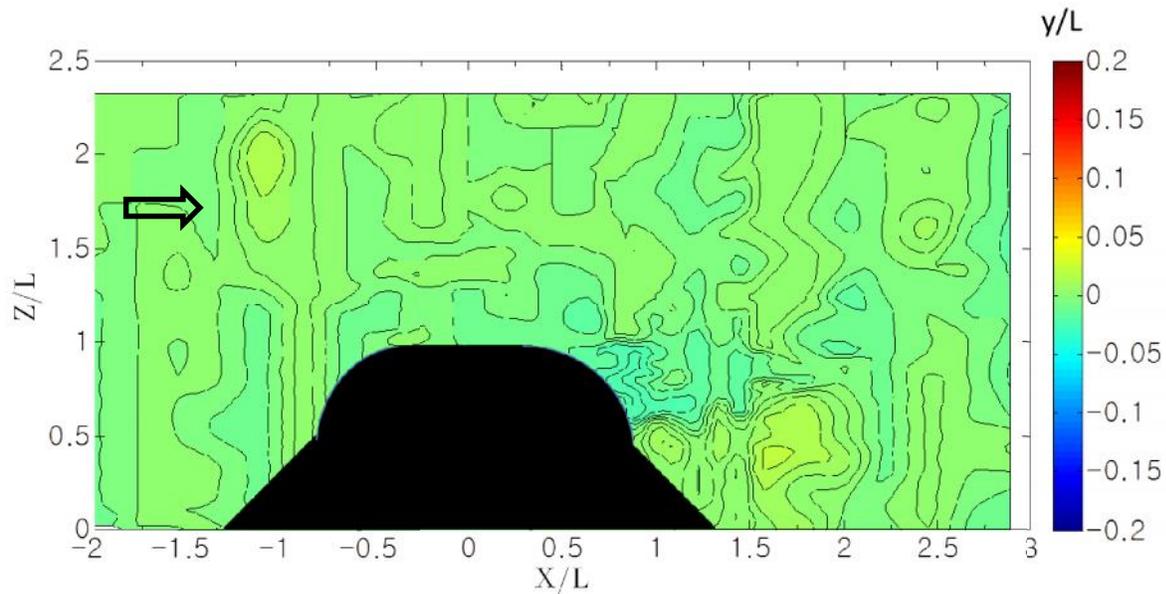
Figure 4. Bed level change contours after and before flow around the sCAUR™ modified wing-wall model with VorGAUR™ VGs. L is the abutment length into the flow. No scour observed at any location (7).

Flow and scour depth results are given for flume tests without and with sCAUR™ modified spill-through abutment with VorGAUR™ VGs under the same 0.66mps flow (7). The surface oilflow (Figure 5) clearly shows that the sCAUR™ and VorGAUR™ products bring lower velocity flow up from the flume bottom and prevent scour around the bottom of the abutment. Deep scour holes occur around the foundation for the untreated spill-through abutment (7). Figure 6 shows no scour around the upstream contraction and near the base of the modified spill-through abutment due to the fairing. Although there is still a very minor scour at the downstream of the model, its max depth (-0.02L) is much lower

than that for an untreated abutment. The open bed scour due to the free surface vortex has been prevented.



1
2 Figure 5. Surface oilflow results for modified sharp-edge spill-
3 through abutment model with 8 VGs. Note that sCAUR™ and VorGAUR™
4 cause the flow to move up the abutment as it moves downstream,
5 bringing low speed fluid from the bottom of the river and
6 preventing scour. The gray region is produced by a mixture of
7 the oilflow material and waterborne substances at the free
8 surface (7).



9

10 **Figure 6. Bed level change contours after and before flow around**
 11 **the sCAUR™ modified sharp-edge spill-through model with VorGAUR™**
 12 **VGs (L = 229mm). No scour at any location (7).**

13

14 **TASK IV.B - Flume Tests of Foundations Exposed by Open Bed**

15 **Scour**

16 Aspects of the sCAUR™ and VorGAUR™ design features have been
 17 expanded for use around the foundation (AUR Provisional Patent)
 18 to protect the foundation from the effects of contraction scour,
 19 long term degradation scour, settlement and differential
 20 settlement of footers, undermining of the concrete sCAUR™
 21 segments, and effects of variable surrounding bed levels.

22 As all AUR flume studies have shown (7), under these conditions
 23 scour of the open bed material occurs at a lower river speed
 24 before scour of the material around the base of the sCAUR™

25 fairing occurs.

26

27 This means that scour of the river bed away from the sCAUR™
28 protected pier or abutment occurs first and that the river bed
29 level will be lower away from the pier or abutment. If a pier or
30 abutment foundation is exposed, it will still have a higher
31 immediate surrounding river bed level than farther away. Even
32 so, one would like to further arrest scour around the foundation
33 to prevent high speed open bed scour from encroaching on the
34 river bed material next to the foundation.

35

36 Second, if the front of the foundation of a pier or abutment is
37 exposed to approach flows, then a foundation horseshoe or
38 scouring vortex is formed at the front which will cause local
39 scour around the pier or abutment. This suggests that a curved-
40 top ramp be mounted in front of the foundation that prevents the
41 formation of this foundation horseshoe vortex.

42

43 Based on these facts, flume tests were conducted with 3
44 foundation leading edge ramp configurations: (1) an exposed
45 rectangular foundation with no front ramp protection, (2) an
46 upstream curved-top foundation ramp with trapezoidal span-wise
47 edges to produce a stream-wise vortex to bring open bed
48 materials toward the foundation, and (3) a curved-top upstream

49 foundation ramp with straight span-wise edges. Gravel A was
50 used around the foundation since it was the smallest gravel
51 tested in this project in Task III. **In summary, all of these**
52 **foundation tests show that a leading edge straight-sided curved**
53 **top ramp prevents scour around a foundation when there is open**
54 **bed scour, as shown in Figure 7.**



55
56 **Figure 7. Gravel level after flume test for 12.7mm high**
57 **elevation with a 12.7mm high straight-sided curved leading edge**
58 **ramp. No scour is observed (7).**

59
60 **TASK VI. Tests of Full-Scale sCAUR™ and VorGAUR™ Prototype in**
61 **the University Of Iowa Institute of Hydraulic Research (IIHR)**
62 **Flume.**

63 Full-scale pier model scour tests were conducted during 2013 in
64 the high flow quality University of Iowa Institute of Hydraulic
65 Research (IIHR) 3.05m wide Environmental Flow Facility, which is
66 described at the website:

67 [http://www.iihr.uiowa.edu/research/instrumentation-and-](http://www.iihr.uiowa.edu/research/instrumentation-and-technology/environmental-flow-facility/)
68 [technology/environmental-flow-facility/](http://www.iihr.uiowa.edu/research/instrumentation-and-technology/environmental-flow-facility/).

69 Two test gravel sediment sizes (specific gravity = 3) were used
70 during each test. With only a trace amount below 3.2mm, by
71 weight about 63% of the smaller sediment gravel was between
72 3.2mm and 6.3mm and 37% was between 6.3mm and 9.5mm. The larger
73 test gravel, which filled most of the flume bed, was between
74 9.5mm and 16mm. A 88.9mm outside diameter vertical circular
75 cylinder model was located downstream of the sCAURTM model about
76 0.46m from a flume side wall and 0.46m from the end of the
77 gravel bed and tested with the larger gravel at the same time as
78 each of the several configurations of the sCAURTM full-scale
79 model to show that the flow conditions cause scour with the
80 cylinder. Test runs continued until after the cylinder scour
81 reached equilibrium conditions with no further observed scour.
82 With the larger gravel, the equilibrium scour hole was 76mm deep
83 in front of the cylinder and extended 89mm upstream with a span-
84 wise width of 0.28m.

85

86 Measurements were obtained for the scour depth around the base
87 of the model after the flume was drained using photos of laser
88 sheet surface locations (5), surface oilflows over the model to
89 determine the local surface flow direction, and some pitot tube
90 flow velocity data in front of and around the model. Five full-

91 scale model configurations were tested with the larger and
92 smaller gravel on opposite sides of the model (7). Configuration
93 A, a full-scale 10.16m long 1.42m wide sCAUR™ model with 6
94 VorGAUR™ vortex generators with three 2.44m side sections on
95 each side, as shown in Figure 8, flush with the gravel bed top;
96 Configuration B, same as Configuration A, but with 8 VorGAUR™
97 vortex generators; Configuration C, same as B, but with the
98 straight-sided leading edge curved-top ramp like in Figure 7
99 above and the model 76mm above the surrounding gravel bed;
100 Configuration D, full-scale sCAUR™ with 8 VorGAUR™ vortex
101 generators with only one side section on each side and flush
102 with the gravel bed; Configuration E, full-scale sCAUR™ nose and
103 tail sections with 4 nose section VorGAUR™ vortex generators
104 with no side sections.

105



106

107 **Figure 8. Photo from upstream of the AUR full-scale 10.16m long**
108 **1.42m wide sCAUR™ with VorGAUR™ vortex generators model in the**
109 **IIHR Environmental Flume Facility with three 2.44m side sections**
110 **on each side for Configurations A and B. Small and large gravel**
111 **on opposite sides are flush with the edge of the model.**

112

113 In summary, the full-scale model tests confirmed that there was
114 no scour around the front and sides for each Configuration with
115 either the smaller or larger gravel, as was also observed at
116 model scale. Only a small amount of scour of the smaller gravel
117 was observed downstream, which was due to full-scale model width
118 to flume width (0.15 to 1/3) flow blockage effects, which were

119 comparable to flow blockage results for the 1/7 size models in
120 the AUR flume (7).

121

122 **TASK VII. Cost-effective Manufacturing and Installation of**
123 **sCAUR™ and VorGAUR™ Products**

124

125 Before this project, AUR performed a cost benefit analysis of
126 sCAUR™ with VorGAUR™ as compared to current scour
127 countermeasures (7). Published information shows that current
128 expenses are required for scour monitoring, evaluation, and
129 anti-scour mitigation design and construction, usually with rip-
130 rap. For a bridge closed due to scour, the cost to motorists
131 due to traffic detours is estimated to be as great as all other
132 costs combined, but were not included in the analysis (7).

133

134 There is no situation where sCAUR™ and VorGAUR™ products cost
135 more than current countermeasures. There is no situation where
136 any type of scour is worse with the use of the sCAUR™ and
137 VorGAUR™ products than without them. The more frequent that
138 scouring floods occur, the more cost effective are sCAUR™ and
139 VorGAUR™. Clearly, sCAUR™ and VorGAUR™ products are practical
140 and cost-effective for US highway bridges (7).

141

142 In order to further reduce costs and increase the versatility of

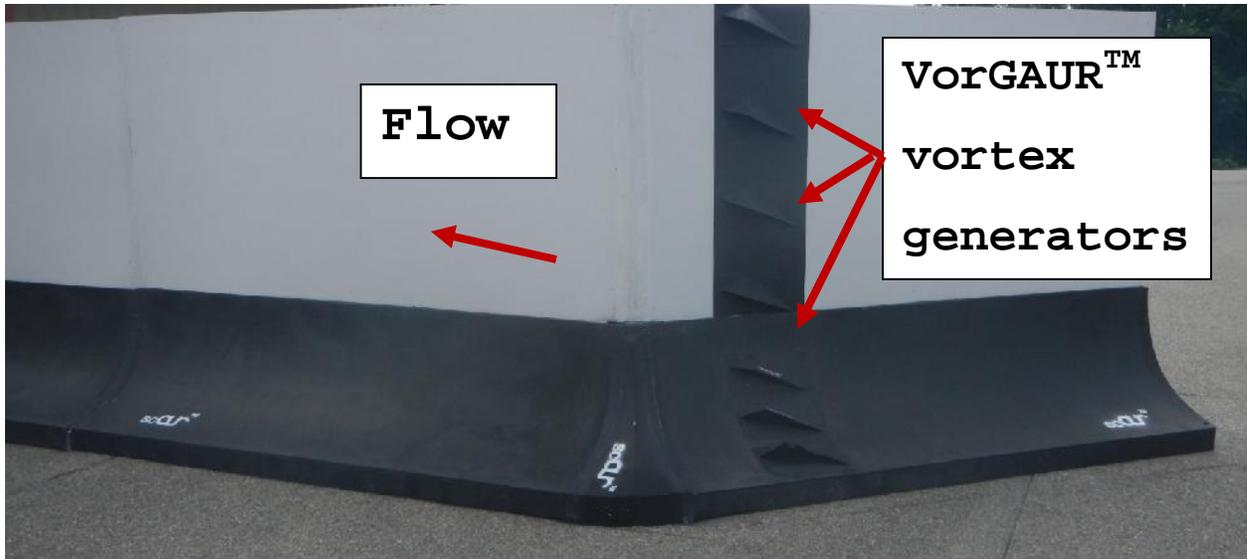
143 the scAUR™ and VorGAUR™ products, multiple manufacturing
144 alternatives were considered. The required labor, materials,
145 time, logistics, and practical issues were examined and used to
146 evaluate manufacturing alternatives (7). Since the NCHRP-IDEA-
147 162 project, detailed full-scale cost-effective versions have
148 been developed for installation.

149

150 **Retrofit to an Existing Bridge**

151 An installed welded stainless steel (SS) scAUR™ retrofit bridge
152 fairing is cost-effective, being about half of all costs for
153 precast or cast-in-place concrete manufacturing and installation
154 (7). Its corrosion resistance gives it a lifetime of 100 years
155 even in seawater environments, using a proper thickness,
156 construction methods, and type of SS. It is an effective way to
157 reduce weight and the cost associated with casting custom
158 reinforced concrete structures. Another benefit is that the SS
159 VorGAUR™ vortex generators can be welded directly onto the side
160 sections instead of having to be integrated into the rebar cage
161 of the reinforced concrete structure. Figure 9 is an example of
162 a retrofitted wing-wall abutment. **Even for bridges with little**
163 **life left, current temporary countermeasures are much more**
164 **expensive when the present value of future expenses is**
165 **considered (7).**

166



167

168 **Figure 9.** Photo of an example stainless steel sCAUR™ retrofit
169 (black) for a 45° wing-wall abutment. Note stainless steel
170 VorGAUR™ vortex generators.

171

172 **New construction**

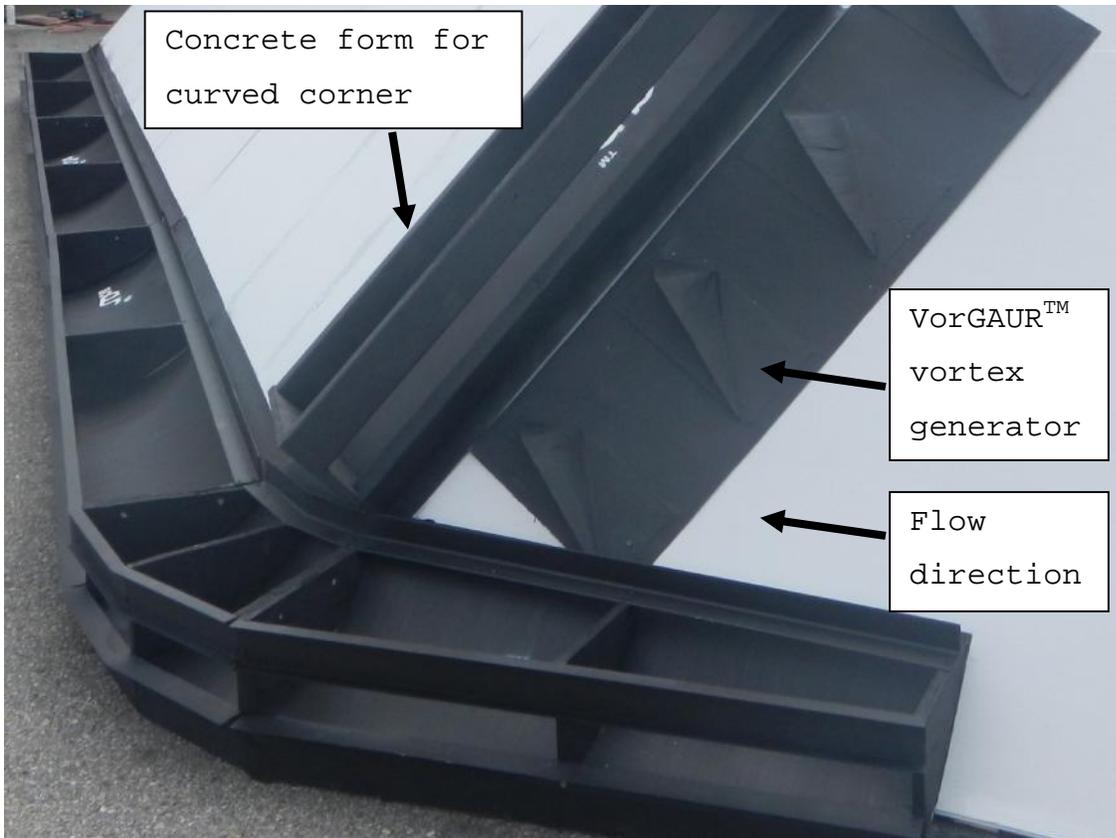
173 In the case with new construction, essentially the difference
174 between the way cast-in-place bridge piers and abutments are
175 constructed currently without the sCAUR™ products and in the
176 future with the sCAUR™ products is that sCAUR™ steel forms for
177 the concrete are used (7). All standard currently used concrete
178 construction methods and tools can be used. During the bridge
179 design phases, the bridge pier or abutment foundation or footer
180 top surface width and length would need to be large enough to
181 accommodate the location of the sCAUR™ concrete fairing on top.
182 Rebar needed for the sCAUR™ would be included in the foundation

183 during its construction. Stainless steel rebar for welding to
184 the stainless steel vortex generators mounting plates on the
185 surface needs to be used for specific locations. Figure 10 shows
186 example sCAUR™ new construction concrete forms for a pier while
187 Figure 11 shows example sCAUR™ new construction concrete forms
188 for a 45° spill-through abutment. **Clearly, since the new**
189 **construction cost is about 1/3 of retrofit costs, the best time**
190 **to include the sCAUR™ fairing on piers and abutments is during**
191 **new construction (7).**



192
193 **Figure 10. Photo of example sCAUR™ new construction concrete**
194 **forms (black) for a pier.**

195



196

197 **Figure 11. Photo of example sCAUR™ new construction concrete**
198 **forms (black) for a 45° spill-through abutment. Note stainless**
199 **steel VorGAUR™ vortex generators mounted after concrete**
200 **construction.**

201 **CONCLUSIONS**

202 Local scour of bridge piers and abutments is a common cause of
203 highway bridge failures. All currently used countermeasures are
204 temporary and do not prevent the root cause of local scour -
205 discrete large-scaled vortices formed by separations on
206 underwater structures. Using the knowledge of how to prevent the
207 formation of discrete vortices, prior to the NCHRP-IDEA-162
208 project, AUR developed, proved using model-scale tests, and

209 patented new local-scouring-vortex-prevention products that are
210 practical cost-effective long-term permanent solutions to the
211 bridge pier and abutment local scour problem. In the NCHRP
212 Project and later work, work on the effect of pier size or scale
213 and model flume tests for other sediments, other abutment
214 designs, and for open bed scour conditions showed that the
215 products prevent scouring vortices and scour. Full-scale
216 prototypes were successfully tested and cost-effective
217 manufacturing and installation plans were developed. **The present**
218 **value cost of these products over the life of a bridge are an**
219 **order of magnitude cheaper than current scour countermeasures.**
220 Concrete forms for new bridges and stainless steel retrofit
221 versions for existing bridges are now available. Plans for
222 installation these products on scour-critical bridges are
223 underway.

224

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231

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