



70th Annual MRCA Conference & Expo
November 20-22, 2019
Overland Park, Kansas

**Update on current
roofing industry technical issues**

presented by

Mark S. Graham
Vice President, Technical Services
National Roofing Contractors Association (NRCA)



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Topics

- MRCA/NRCA-joint research:
 - Fastener pull-out testing
 - MB sheet testing
- Steel roof decks/seam-fastened systems
- Moisture in concrete roof decks
- ASTM and ICC developments
- Questions

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MRCA/NRCA-joint research:

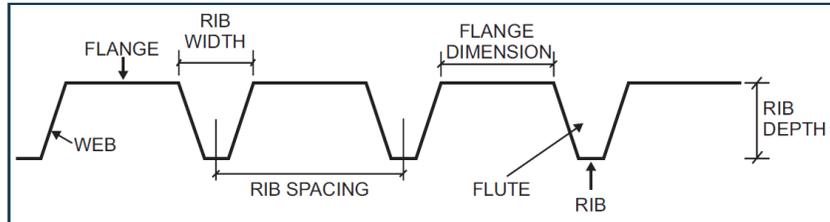
- Fastener pull-out testing
- MB sheet testing

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Fastener pull-out testing

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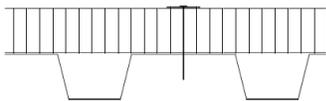
Terminology -- Steel roof decks



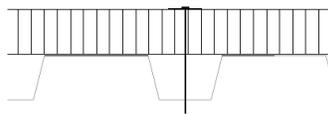
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Tested fastener locations

Fastener in flange



Fastener in rib



Fastener in web



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Other test parameters

Steel deck types:

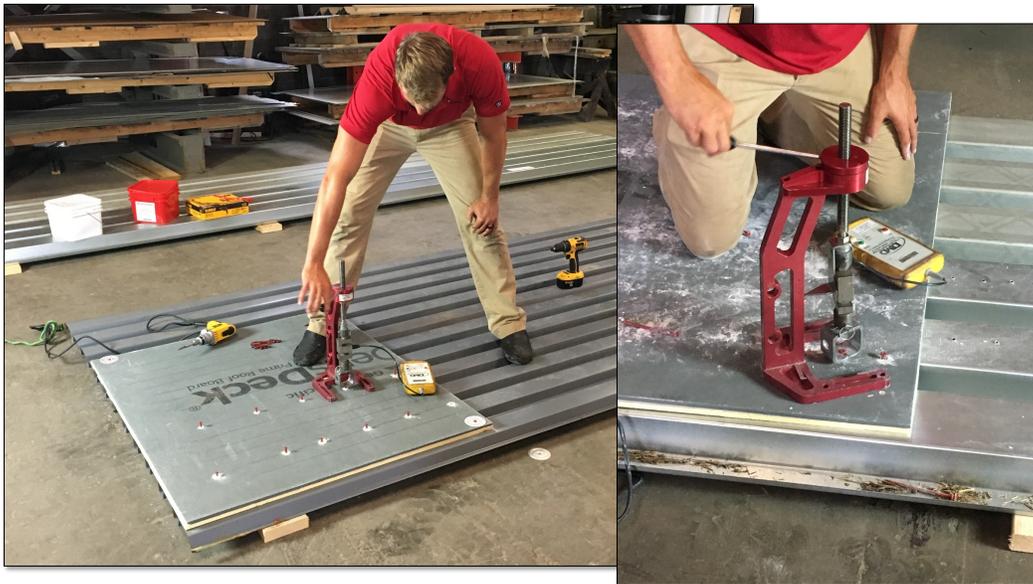
- 22 ga., 1½-in.-thick, Type B-deck
- 20 ga., 3-in.-thick, Type N-deck (Type 3DR)

Fastener types:

- All-purpose fastener (#14)
 - Published pull-out values:
 - 22 ga.: 315 lbf at 33 ksi and 480 lbf at 80 ksi
 - 20 ga.: 420 lbf at 33 ksi and 615 lbf at 80 ksi
- Heavy duty fastener (#15)
 - Published pull-out values:
 - 22 ga.: 595 lbf at 33 ksi and 650 lbf at 80 ksi

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Test set-up and equipment



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Test data

22 ga., 1½-in.-thick, Type B deck
All-purpose Fastener (#14)
Average value 10 pull-out tests

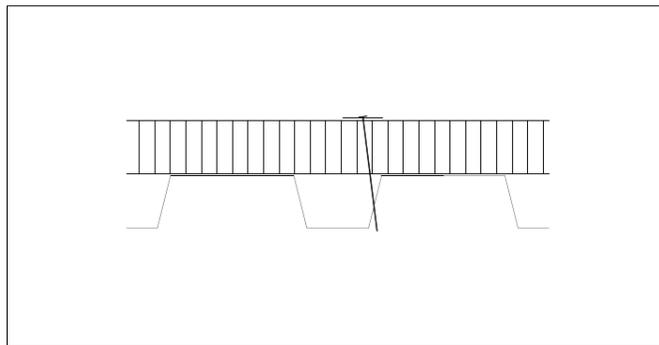
Fastener in flange	Fastener in rib	Fastener in web
637.4 lbf	561.1 lbf	556.2 lbf

Published pull-out value is 315-480 lbf

Tested fastener in rib value is 88 % of fastener in flange value
Tested fastener in web value is 87% of fastener in flange value

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Fastener in flute



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Test data
 22 ga., 1½-in.-thick, Type B deck
 Heavy Duty Fastener (#15)
 Average value 10 pull-out tests

Fastener in flange	Fastener in rib	Fastener in web
761 lbf	680.9 lbf	674.8 lbf

Published pull-out value is 595-650 lbf

Tested fastener in rib value is 89 % of fastener in flange value
Tested fastener in web value is 89% of fastener in flange value

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Test data
 20 ga., 3-in.-thick, Type3DR deck
 All-purpose Fastener (#14)
 Average value 10 pull-out tests

Fastener in flange	Fastener in rib	Fastener in web
848.8 lbf	732.8 lbf	733.0 lbf

Published pull-out value is 420-615 lbf

Tested fastener in rib value is 86% of fastener in flange value
Tested fastener in web value is 86% of fastener in flange value

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Test data

20 ga., 3-in.-thick, Type3DR deck
Heavy Duty Fastener (#15)
Average value 10 pull-out tests

Fastener in flange	Fastener in rib	Fastener in web
1,044 lbf	1,037 lbf	978.2 lbf

No published pull-out value

Tested fastener in rib value is 99% of fastener in flange value
Tested fastener in web value is 94% of fastener in flange value

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Conclusions

Fastener pull-out testing

- Tested pull-out values are greater than published values
- “Fastener in web” or “Fastener in rib” placement results in a less than 15% reduction in pull-out load versus “Fastener in flange” placement
- Actual deck gauge, deck yield strength and fastener selection have larger impacts on fastener pull-out values
- A safety factor is typically applied to fastener pull-out loads which more than covers this reduction
- This test data applies to insulation fasteners’ performances, not necessarily membrane fasteners’ (e.g, fastener “rocking” due to membrane fluttering)

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Polymer-modified bitumen sheet testing

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Polymer-modified bitumen test results			
Sample (manufacturers and product)	Low-temperature flexibility (F)		Granule embedment as received (grams)
	As received	Heat aged (90 days at 158 F)	
SBS products			
1-A	-25	-25	0.9
2-A	-20	-15	1.6
2-B	0	15	0.7
2-C	-35	-15	1.3
3-A	10	20	1.8
4-A	-30	-30	1.1
4-B	-15	-5	0.8
5-A	-5	0	0.6
5-B	10	10	0.7
6-A	-20	-15	1.1
9-A	-30	-15	0.6
ASTM International's maximum allowable values	0	0	2
APP products			
3-B	20	20	0.7
8-A	20	35	3.4
ASTM International's maximum allowable values	32	32	2

Professional Roofing
February 2016

Nine of 13 products tested complied...

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2011 testing

Only six of the 16 products tested complied....

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2019 MB testing

- ASTM D5147 -- Low-temperature flexibility (as received)
- ASTM D4977 -- Granule embedment (as received)

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Products tested

2019 MB testing

- 18 products tested:
 - 7 APP
 - 9 SBS
 - 15 products with granules
 - 3 products without granules (granule embedment doesn't apply)
- Manufacturers:
 - 10 (CertainTeed, Derbigum, Firestone, GAF, Garland, JM, Polyglass, Siplast, Soprema and Tremco)

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Results – SBS products

2019 testing

Sample ID	Modifier	ASTM designation	Low-temp. flex. (F)	Granule loss (g)
1-A	SBS	ASTM D6164, Type I, Grade G	-13	0.56
3-A	SBS	ASTM D6164, Type I, Grade S	-27	NA
3-B	SBS	ASTM D6164, Type II, Grade G	-15	0.48
4-A	SBS	ASTM D6164, Type II, Grade G	-16	1.13
5-A	SBS	ASTM D6162, Type III, Grade G	-15	2.05
6-A	SBS	ASTM D6164, Type I, Grade G	-13	0.34
6-B	SBS	ASTM D6164, Type II, Grade G	-13	0.53
6-C	SBS	ASTM G6164, Type I, Grade G	-9	0.55
8-A	SBS	ASTM D6163, Type I, Grade G	-20	0.09
9-A	SBS	ASTM D6164, Type I, Grade G	-8	0.53
10-A	SBS	ASTM D6163, Type III, Grade G	Less than -40	1.16
ASTM spec.			0 (max.)	2.0 (max)

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Results – SBS products

2019 testing

Sample ID	Modifier	ASTM designation	Low-temp. flex. (F)	Granule loss (g)
1-A	SBS	ASTM D6164, Type I, Grade G	-13	0.56
3-A	SBS	ASTM D6164, Type I, Grade S	-27	NA
3-B	SBS	ASTM D6164, Type II, Grade G	-15	0.48
4-A	SBS	ASTM D6164, Type II, Grade G	-16	1.13
5-A	SBS	ASTM D6162, Type III, Grade G	-15	2.05
6-A	SBS	ASTM D6164, Type I, Grade G	-13	0.34
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10-A	SBS	ASTM D6163, Type III, Grade G	Less than -40	1.16
ASTM spec.			0 (max.)	2.0 (max)

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Results – APP products

2019 testing

Sample ID	Modifier	ASTM designation	Low-temp. flex. (F)	Granule loss (g)
2-A	APP	ASTM D6223, Type I, Grade G	21	0.95
2-B	APP	ASTM D6223, Type I, Grade S	10	NA
2-C	APP	D6223, Grade G	14	0.60
2-D	APP	ASTM D6222, Type II, Grade G	10	0.65
2-E	APP	D6223, Grade G	9	NA
7-A	APP	D6222, Grade G	Greater than 41	0.10
7-B	APP	D6222, Type I, Grade G	Greater than 41	0.88
ASTM spec.			32 (max.)	2.0 (max)

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Results – APP products

2019 testing

Sample ID	Modifier	ASTM designation	Low-temp. flex. (F)	Granule loss (g)
2-A	APP	ASTM D6223, Type I, Grade G	21	0.95
2-B	APP	ASTM D6223, Type I, Grade S	10	NA
2-C	APP	D6223, Grade G	14	0.60
2-D	APP	ASTM D6222, Type II, Grade G	10	0.65
2-E	APP	D6223, Grade G	9	NA
7-A	APP	D6222, Grade G	Greater than 41	0.10
7-B	APP	D6222, Type I, Grade G	Greater than 41	0.88
		ASTM spec.	32 (max.)	2.0 (max)

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Summary of results

- 15 of the 18 products tested comply
- Results notably are better than 2015 and 2011
- Still some reason(s) for concern

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Recommendations

2019 MB testing

- Select MB products carefully
- Consider seeking out products with third-party verification of compliance:
 - UL product certification
 - PRI Product Validation
 - Dade County Approval
- As always, call NRCA Technical Services if you see anything unusual

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Steel roof decks/seam-fastened systems

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SDI bulletin

2009

STEEL DECK INSTITUTE
Position Statement

ATTACHMENT OF ROOFING MEMBRANES TO STEEL DECK

This document has been published by the Steel Deck Institute (SDI) as a position paper in response to discussions taking place in the roofing community about the screw attachment of roofing membranes to steel deck following line patterns with large spacing. The impetus for this paper is in response to testing carried out by the Special Interest Group for Dynamic Evaluation of Roofing Systems (SIGEDERS) at the Institute for Research in Construction, National Research Council of Canada. The mandate of the SIGEDERS joint research program is to carry out generic, pre-competitive research on the performance of flat roofing systems subjected to dynamic wind loading. The objective is to develop improved roofing systems and design methods.

The SIGEDERS research is looking at roofing systems that incorporate wide membrane sheets attached to the steel deck following line patterns spaced at up to 12 ft (3.65 m). While the membrane itself has the performance characteristics to accommodate this type of loading, the existing design methods for steel deck under wind uplift are typically based on the uniform application of the wind suction to the deck. The large majority of the steel roof deck used for commercial buildings in North America is profiled with 1 1/4" (38 mm) flutes, with the structural supports usually spaced between 5'-0" (1.52 m) and 6'-0" (1.83 m). Under right conditions, the attachment of the roofing membrane along lines with large spacing could produce localized loads that can exceed the capacity of the deck, whereas those same loads, applied uniformly on the surface of the deck would be acceptable.

The strength of screwed connection between the membrane and the steel deck, as well as the strength of screwed, nailed or welded attachment of the steel deck to the structural supports can be computed according to the North American Specification for the Design of Cold-Formed Steel Structural Members. These design values are based on the specified minimum mechanical properties (i.e. have steel thickness and yield strength) specified for the steel sheet roof deck, and should be lower than the strength determined by field testing. The use of field test results for properties such as the pull-out strength of a screw into a steel deck needs to recognize that the properties of the steel deck can be higher than the minimum limits required by the steel specifications. Therefore, field testing results must be adjusted accordingly to account for the difference between the actual properties of the deck and the minimum properties of the steel according to the material specification used in design.

The screw fastening of wide roofing membranes (up to 12 ft) and the corresponding spacing of the lines of screws holding the membrane on the deck, will have a very different effect on the deck and structural supports than a membrane that is adhered over its entire surface. The screws will produce a line load along the deck instead of a uniform load of the entire deck surface. The line loads can be perpendicular or parallel to the deck flutes depending on the orientation of the membrane; each condition can have different implications of the loading that is applied to the deck.

If the roofing membrane seam is perpendicular to the flutes of the deck, as illustrated in Figure 1, there are two special conditions that need to be considered:

1. if the membrane seam occurs at the mid-span of the steel deck; and
2. if the membrane seam occurs at the structural support (joist).

1

- Decks designed for joist spacing between 5' and 6' 8" o.c.
- Deck designed for uniform loading
- Seam-fastened single-ply membranes are a concern

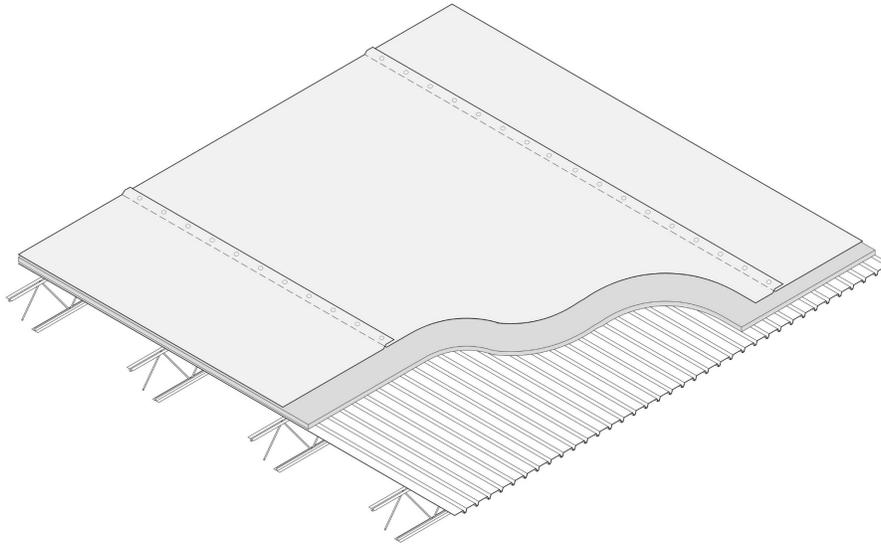
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Membrane seams across deck flutes

SDI: 3.8 X moment (deck); 2 X load (joists)

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Membrane seams in deck flute direction



SDI: 12 X bending moment and shear (deck)

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SDI bulletin – Conclusion

2009 bulletin

“...SDI does not recommend the use of roofing membranes attached to the steel deck using line patterns with large spacing unless a structural engineer has reviewed the adequacy of the steel deck and the structural supports to resist to wind uplift loads transmitted along the lines of attachment. Those lines of attachment shall only be perpendicular to the flutes of the deck.”

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FM Global's Loss Prevention Data Sheet 1-29

April 2016

FM Global
Property Loss Prevention Data Sheets 1-29
January 2016
Revised Previous April 2015
Page 1 of 48

ROOF DECK SECUREMENT AND ABOVE-DECK ROOF COMPONENTS

Note to Members of Factory Mutual Insurance Company: Contact the local FM Global office before beginning any roofing work.

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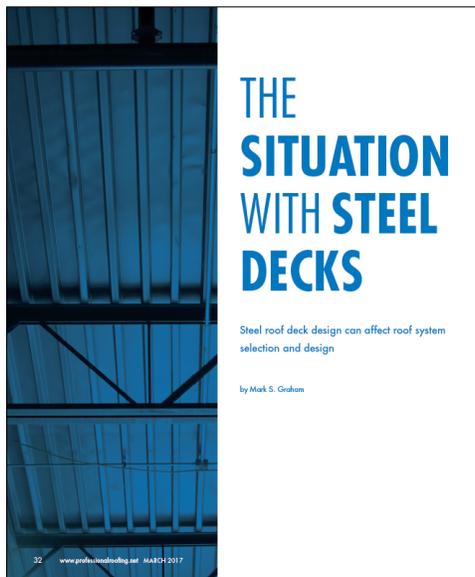
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Revised/new criteria:

- Steel roof decks:
 - Uniformly-distributed loading
 - Concentrated loading
- Lightweight structural concrete

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Professional Roofing
March 2017
www.professionalroofing.net

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Fastener pull-out tests...

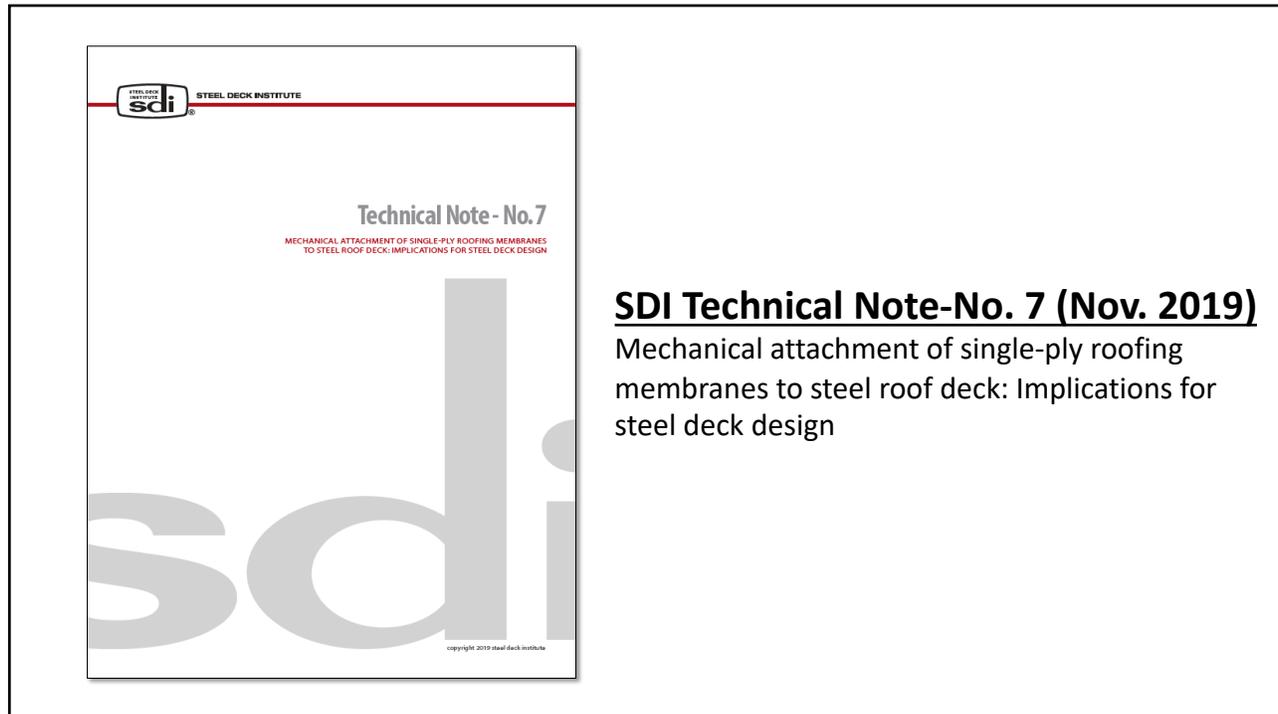
There is little correlation between fastener pull-out resistance and a steel roof deck's yield strength and uplift (bending) strength

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Although roofing contractors sometimes are given the responsibility of inspecting and accepting steel roof decks to receive a new roof system, determining a roof deck's design adequacy is beyond the expertise of most roofing contractors.

This determination is best made during a project's design phase.

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SDI Technical Note-No. 7 (Nov. 2019)

Mechanical attachment of single-ply roofing membranes to steel roof deck: Implications for steel deck design

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The image shows page 6 of the technical note. At the top, the SDI logo and 'Technical Note - No. 7' are visible. The main heading is 'Analysis of Steel Deck and Supports for Mechanically Attached Membrane Roofs'. Below this, a paragraph states: 'For both new construction, and recovering or reroofing, the following guidelines reflect generally accepted industry practice:'. A numbered list of four guidelines follows. At the bottom, a section titled 'SDI Recommendations' contains four specific points regarding fastener spacing, structural review, attachment orientation, and pre-construction submittals.

Analysis of Steel Deck and Supports for Mechanically Attached Membrane Roofs

For both new construction, and recovering or reroofing, the following guidelines reflect generally accepted industry practice:

1. Analyze the deck as a continuous 3-span beam, unless shorter spans are used.
2. Utilize all load combinations required by the applicable building code.
3. For the design spacing of fastener lines, place the first uplift line load at the midspan of the first deck span, then continue to add line loads as applicable. Repeat as necessary to determine the maximum positive and negative bending moments.
4. To determine maximum uplift on deck securement fasteners and support framing, place a line load atop a support.

SDI Recommendations

1. The SDI does not recommend the use of roofing membranes attached to the steel deck using line patterns with large spacing (spacing greater than 1/2 of the deck span) unless a structural engineer has reviewed the adequacy of the steel deck and the structural supports to resist wind uplift loads transmitted along the lines of attachment.
2. When existing buildings with steel roof deck are recovered or reroofed with a mechanically attached membrane, a competent structural engineer should be engaged to determine the limitations imposed by the existing steel deck.
3. The lines of attachment for mechanically attached membranes shall only be perpendicular to the ribs of the deck. Membranes should not be attached with lines of fasteners parallel to the deck ribs.
4. Designers should require pre-construction submittals of membrane layouts to ensure that the lines of fasteners (direction and spacing) comply with structural design assumptions. Determination of membrane layouts should not be left to the option of field crews.

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Technical Note - No. 7

The steel deck bending and shear strength (resistance) and strength (resistance) of the fasteners attaching the deck to the supports are calculated using the *North American Specification for the Design of Cold-Formed Steel Structural Members (ANSI S100-16)* and the *Standard for Steel Roof Deck (ANSI/SDI RD-2017)*. These design strengths are dependent on the specified minimum mechanical properties (i.e. base steel thickness, yield and ultimate strength) for the roof deck, and should be lower than the strength determined by field-testing. Results of field-tests utilized to determine strengths which are dependent on the mechanical properties of the steel deck, such as pull-out or pull-over of a screw fastened through deck, must recognize the properties of the delivered steel may exceed the minimum limits required by the steel specification. Therefore, field-test results must be adjusted.

SDI Recommendations

1. The SDI does not recommend the use of roofing membranes attached to the steel deck using line patterns with large spacing (spacing greater than 1/2 of the deck span) unless a structural engineer has reviewed the adequacy of the steel deck and the structural supports to resist wind uplift loads transmitted along the lines of attachment.
2. When existing buildings with steel roof deck are recovered or reroofed with a mechanically attached membrane, a competent structural engineer should be engaged to determine the limitations imposed by the existing steel deck.
3. The lines of attachment for mechanically attached membranes shall only be perpendicular to the ribs of the deck. Membranes should not be attached with lines of fasteners parallel to the deck ribs.
4. Designers should require pre-construction submittals of membrane layouts to ensure that the lines of fasteners (direction and spacing) comply with structural design assumptions. Determination of membrane layouts should not be left to the option of field crews.

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*Expect additional scrutiny of seam-fastened,
mechanically-attached, single-ply membrane roof systems*

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Moisture in concrete roof decks

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The quest for dryness
by Matt Dupuis, Ph.D., P.E.
Professional Roofing, June 2017

RESEARCH+TECH
Are admixtures the answer?
Moisture in concrete roof decks continues to be problematic
by Mark S. Graham
Professional Roofing, December 2018

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What's next...

- January 2020: Results of MVRA testing
- February 2020: Updated recommendations:
 - Use a “very good” vapor retarder. What is very good?
 - Avoid roof system components with organic content
 - Deck dryness: 75% RH or less using ASTM F2178
 - Limit deck acceptance to visible conditions (e.g, surface moisture, broom clean)
 - Contract language limiting liability

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The screenshot shows the NRLRC website with a navigation bar and a news article. The article title is "Contract provision addresses installation of roof system over concrete deck". The article text discusses the risks of installing a roof over a wet concrete deck and provides a contract provision to address this issue.

NRLRC News

Contract provision addresses inadequate drainage design

Contract provision states reroofing contractor not responsible for removing existing water and ice-dam protection membrane

[More news]

Contract provision addresses installation of roof system over concrete deck

Installing a roof over a structural concrete deck that is not sufficiently dry can cause an array of serious problems. A “wet” concrete deck can cause inadequate adhesion or detachment of roofing materials, putting the roof at risk of blow-off or falling wind-uplift testing. Over time, there is an increased risk that moisture in the concrete deck will migrate into the roof system. This problem is particularly acute with unvented lightweight structural concrete roof decks but is not limited to lightweight structural concrete. A general contractor faced with a compressed project timeline, delays and pressure to meet schedule may push a roofing contractor to proceed with roof installation before the concrete deck has had enough time to dry. Rewetting also is a major concern. In the event a project involves installation of a roof system over a structural concrete roof deck, it is important a roofing contractor include a provision such as the one above. Subcontract agreements roofing contractors are requested to sign commonly include a

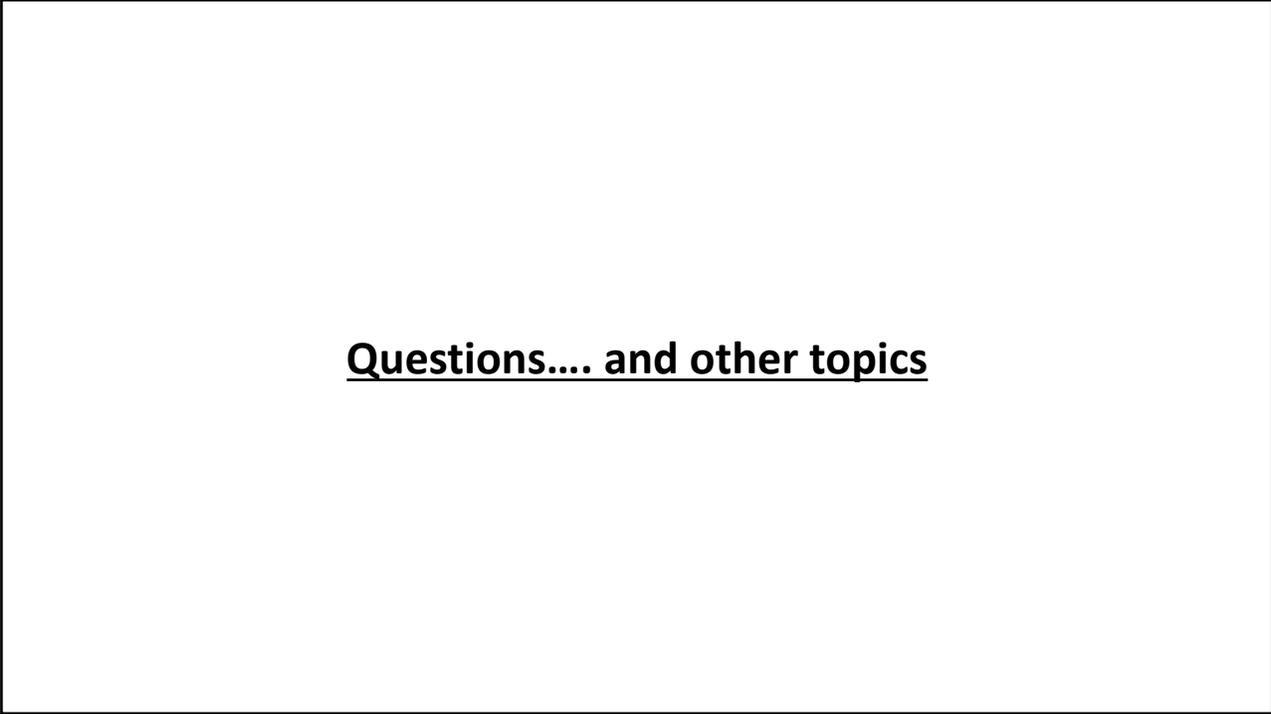
Assessing moisture content in roof deck: Roofing Contractor is not responsible for the effects of moisture migration originating within the roof deck or substrate, including concrete decks, or due to moisture vapor drive from within the building. Residual moisture within the roof deck, particularly structural concrete decks, can adversely affect the properties and performance of roofing materials, regardless of additives or concrete admixtures that may be included in the concrete mix. Roofing Contractor's commencement of roof installation indicates only that the Roofing Contractor has visibly inspected the surface of the deck for visible defects prior to commencement of roofing and the surface of the deck appeared dry. The 28-day concrete curing period does not signify the deck is sufficiently dry.

Roofing Contractor is not responsible to test or assess the moisture content of the deck or evaluate the likelihood of condensation from moisture drive within the building. Roofing contractor recommends that roofing not commence until probes in concrete decks show moisture content is no greater than 75% relative humidity when there is no organic content within the roofing materials. Wood fiberboard, perlite and organic paper facers on polyisocyanurate insulation will generate mold with relative humidity as low as about 65-70%.

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